

Bryant Development and Review Committee Meeting

Boswell Municipal Complex - City Hall Conference Room

210 SW 3rd Street

Date: October 03, 2024 - Time: 9:00 AM

Call to Order

Old Business

New Business

1. 2714 Lavern St - Conditional Use Permit for Duplex

Vanessa Guerra - Requesting recommendation for approval of CUP

• <u>0908-APP-01.pdf</u>

2. Window World - 511 Boone Road - New Driveway

Jodie Cerrato - Requesting Approval for New Additional Driveway

• <u>0905-PLN-01.pdf</u>

3. 21814 I-30 - Site Plan Addition

Requesting Approval for Addition of Two Carports on Site

- <u>0914-PLN-03.pdf</u>
- <u>0914-PLN-02.pdf</u>
- <u>0914-PLN-01.png</u>
- 0914-DRW-01.pdf

4. Hillcrest Addition - Preliminary plat

Tim Lemons - Requesting Recommendation for Preliminary Plat Approval

- <u>0890-PLN-03.pdf</u>
- <u>0890-RSP-02.pdf</u>
- <u>0890-DRN-04.pdf</u>

5. First Southern Baptist Church - 604 S Reynolds Rd - Site Plan

Hope Consulting - Requesting Recommendation for Approval of Site Plan

- <u>0912-PLN-02.pdf</u>
- <u>0912-RSP-01.pdf</u>
- <u>0912-DRN-02.pdf</u>
- <u>0912-SRVY-01.pdf</u>
- <u>0912-LTR-01.pdf</u>

6. Skye Blue Duplexes Subdivision - Preliminary Plat and Conditional Use Permits

Hope Consulting - Requesting Recommendation for Approval of Preliminary Plat and Four Conditional Use Permits for Duplexes

- <u>0889-DRN-02.pdf</u>
- <u>0889-PLN-03.pdf</u>
- <u>0889-LTR-01.pdf</u>

7. Midtown Ph. 3 - Final Plat

Hope Consulting - Requesting Recommendation for Approval of Final Plat

- <u>0917-BNDLTR-01.pdf</u>
- <u>0917-PLN-01.pdf</u>
- <u>0917-LTR-01.pdf</u>

Staff Approved

8. Rookh - 22000 I-30 - Sign Permit

Seiz Sign Company - Requesting Sign Permit Approval - Staff Approved

- <u>92829-SGNAPP-02.pdf</u>
- 92829-SGNAPP-01.jpg

9. 7 Brew Coffee - 2006 N Reynolds Road - Sign Permit

Springfield Sign - Requesting Sign Permit Approval - Staff Approved

- <u>92828-SGNAPP-02.pdf</u>
- 92828-SGNAPP-01.pdf

Permit Report

Adjournments



City of Bryant, Arkansas Community Development 210 SW 3rd Street Bryant, AR 72022 501-943-0943

Conditional Use Permit Application

Applicants are advised to read the Conditional Use Permit section of Bryant Zoning Code prior to completing and signing this form. The Zoning Code is available at www.cityofbryant.com under the Planning and Community Development tab.

Date: 08/22/2027

Applicant or Designee:	Project Location:
Name Ablia Vanessa George	ARTZIC Bryant, AR.
Address #029 Kiewellr. NLR	AR7211C Bryant, AR.
Phone 501-283-4058	Parcel Number
Email Address: deligguerra - Egnal,	Zoning Classification $\underline{R} - \mathcal{M}$
Property Owner (If different from Applicant)	
Name	
Phone	
Address	
Email Address	
Additional Information: Legal Description (Attach description if necessar	γ)
Description of Conditional Use Request (Attach Convert portion Proposed/Current Use of Property <u>for</u>	of home to deplex

Application Checklist

Requirements for Submission

- □ Letter stating request of Conditional Use and reasoning for request
- □ Completed Conditional Use Permit Application
- □ Submit Conditional Use Permit Application Fee (\$125)
- □ Submit Copy of completed Public Notice
- Publication: Public Notice shall be published at least one (1) time fifteen (15) days prior to the public hearing at which the variance will be heard. Once published please provide a proof of publication to the Community Development office.
- Posting of Property: The city shall provide a sign to post on the property involved for the fifteen (15) consecutive days leading up to Public hearing. One (1) sign is required for every two hundred (200) feet of street frontage.
- □ Submit eight (8) Copies of the Development Plan (Site Plan) showing:
 - Location, size, and use of buildings/signs/land or improvements
 - Location, size, and arrangement of driveways and parking. Ingress/Egress
 - Existing topography and proposed grading
 - Proposed and existing lighting
 - Proposed landscaping and screening
 - Use of adjacent properties
 - Scale, North Arrow, Vicinity Map
 - Additional information that may be requested by the administrative official due to unique conditions of the site.

Once the application is received, the material will be reviewed to make sure all the required information is provided. The applicant will be notified if additional information is required. The application will then go before the Development and Review Committee (DRC) for a recommendation to the Planning Commission. A public hearing will be held at this meeting for comments on the Conditional Use. After the public hearing, the Planning Commission will make a decision on the use.

Note: that this is not an exhaustive guideline regarding the Conditional Use Permit Process. Additional information is available in the Bryant Zoning Ordinance.

READ CAREFULLY BEFORE SIGNING

A ______, do hereby certify that all information contained within this application is true and correct. I further certify that the owner of the property authorizes this proposed application. I understand that I must comply with all City Codes and that it is my responsibility to obtain all necessary permits required.

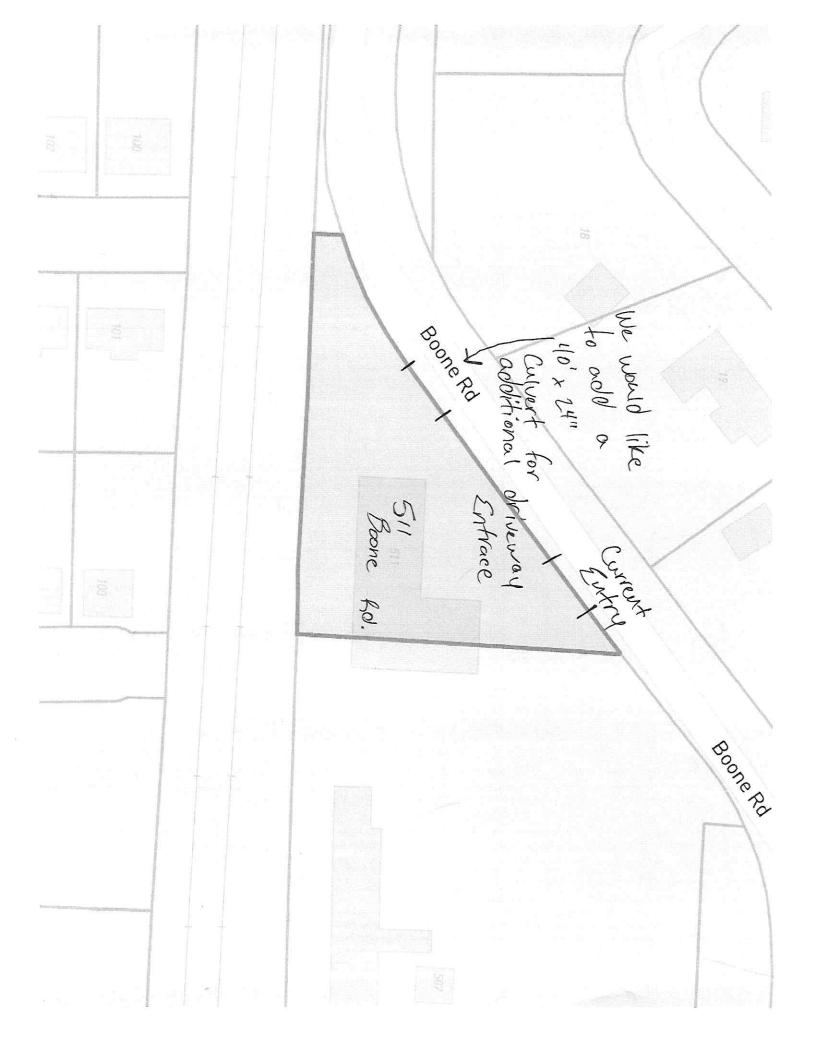
NOTICE OF PUBLIC HEARING

A public hearing will be held on Monday, <u>Oct. 14, 2024</u> at 6:00 P.M. at the Bryant City Office Complex, 210 Southwest 3rd Street, City of Bryant, Saline County, for the purpose of public comment on a conditional use request at the site of <u>2714 Lavern 54</u>. Bryant, AR 72022 (address).

A legal description of this property can be obtained by contacting the Bryant Department of Community Development.

> Lance Penfield Chairman of Planning Commission City of Bryant

This notice is to be run in the legal notices section of the Saline Courier no less than 15 days prior to the public hearing.



	R	EGULAR	2 / A-F	RAN	TE				457 N. Broadway, Joshua, TX 76058 1-866-730-9865
		24'-(115
DESIGN NOTES		ORT ST DESIGN CRI1		BUILI	DINGS) DRAWING	NDEX		A&A ENGINEERING CIVIL • STRUCTURAL 6036 Renaissance Place, Toledo, OH 43623 Tel. 419-292-1983 • Fax. 419-292.0955 WWW.aa-engineers.com
 ALL CONSTRUCTION SHALL BE PROVIDED IN ACCO WITH IBC 2012, OSHA, AISC 360, AISI 100, ASCE 7-1 CODES AND ALL APPLICABLE LOCAL REQUIREME BASE CONNECTIONS SHALL BE PROVIDED AS SH FOUNDATION DETAILS SHEET. ALL MATERIALS IDENTIFIED BY MANUFACTURER MAY BE SUBSTITUTED WITH MATERIAL EQUAL OR E ORIGINAL. ALL SHOP CONNECTIONS SHALL BE WELDED CON ALL FIELD CONNECTIONS SHALL BE #12X1" SDS (I OR EQ). 	O, AWSD NTS. OWN ON NAME XCEEDING NECTIONS.	3. SNOW LO, GROUND IMPORTA THERMAL EXPOSUR	U I I I I I I I I I I I I I I I I I I I	J = 4 PSF (Lr) r = 20 - 6 AS PER S BEE TABLE $g = 20 - 5s = 0.8Ct = 1.2Cc = 1.0$	61 PSF 6NOW LOAD E 4)	SPACING SC & ENCLOS	& MEMBER - TIONS & DETAILS HEDULES - SURE NOTES RT SCHEDULES DPTIONS RAMING GS	1 2 3-A, 3-B 4 5 6 7-A, 7-B	DRAWING INFORMATION PROJECT: 24'-O" WIDE BUILDINGS LOCATION: STATE OF ARKANSAS PROJECT NO.: 033-23-0101 SHEET TITLE: COVER SHEET SHEET NO.: 1 / 11 DRAWN BY: A.W. DATE: 1/22/21 CHECKED BY: OAA DATE: 1/22/21 LEGAL INFORMATION
 STEEL SHEATHING SHALL BE 29GA. CORRUGATE OR PAINTED STEEL - MAIN RIB HT. 3/4" (FY=80K5I) ALL STRUCTURAL LIGHT GAUGE TUBING AND CH. SHALL BE GRADE 50 STEEL. STRUCTURAL TUBE TS2 1/2"X2 1/2" - 14GA. IS EQ TO TS2 1/4"X2 1/4" - 12GA AND EITHER ONE MAY BE LIEU OF THE OTHER. ALL DESIGN CRITERIA MUST BE INCREASED TO HIGHER INCREMENT BASED ON THE TABLES ON PAGE INTERPOLATION IS ALLOWED. 	OR EQ. ANNELS UIVALENT USED IN THE NEXT	EXPOSUR 5. SEISMIC DESIGN C IMPORTA LOAD COMBINATIO 1. D + (Lr OR 2. D + (0.6W 3. D + 0.75 (E C OAD (E) ATEGORY D NCE FACTOR 10 DNS:	e = 1.00	- 180 MPH Lr OR 5)	& OPENING CORNER BRA	GS CING DETAILS AN-TO ADDITION	8-A, 8-B 9 10 11-A TO 11-D	- ANY DUPLICATION OF THIS DRAWING IN WHOLE OR PART IS STRICTLY PORBIDDEN. ANYONE DOING SO WILL BE PROSECUTED UNDER THE FULL EXTENT OF THE LAW. - DRAWINGS VALID UP TO DATE OF EXPIRATION. SEAL: A R K A N S A S * * * * L I C E N S E D P R O F E S S I O N A L E N G I N E E R * * * No. 14606
CUSTOMER INFORMATION OWNER: ADDRESS:	DES GROUND SNO ROOF LIVE L BASIC WIND	.OAD:	BI <u>WIDTH:</u> LENGTH: HEIGHT:	UILDING	INFORMATIC FRAME TYPE: ENCLOSURE TYPE:	A-FRAME REGULAR FULL PARTIAL OPEN	CERTIFICATION NOTICE DATE OF PLANS EXPIRATION: 01-18 CERTIFICATION ON THESE VALID FOR ONE YEAR FROM	8-2024 E DRAWINGS IS	STAMP EXPIRY: 12-31-2024 DATE SIGNED: 01-18-2023

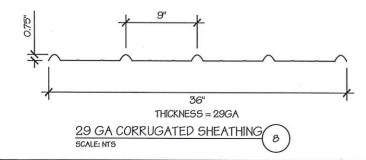
	TABLE 2.1:	MEMBER PROPERTIES	
NO.	LABEL	PROPERTY	DETAIL NO.
1	COLUMN POST	2.5" X 2.5" X 14GA TUBE	1
2	ROOF BEAM	2.5" X 2.5" X 14GA TUBE	1
3	BASE RAIL	2.5" X 2.5" X 14GA TUBE	1
4	PEAK BRACE	2.5" X 2.5" 14GA CHANNEL	4
5	KNEE BRACES	2.5" X 1.5" 14GA CHANNEL	4
6	CONNECTOR SLEEVE	2.25" X 2.25" X 12GA TUBE	2
7	BASE ANGLE	2" X 2" X 3" LG. 3/16" ANGLE	10
8	PURLIN	4.25" X 1.5" X 14GA / 18GA HAT CHANNEL	5
9	GIRT	4.25" X 1.5" X 14GA / 18GA HAT CHANNEL	5
9A	OPT. END WALL GIRT	2.5" X 1.5" 14GA CHANNEL	1
10	SHEATHING	29 GA CORRUGATED SHEET	8
11	END WALL POST	2.5" X 2.5" X 14GA TUBE	1
12	DOOR POST	2.5" X 2.5" X 14GA TUBE	1
13	SINGLE HEADER	2.5" X 2.5" X 14GA TUBE	1
14	DOUBLE HEADER	DBL. 2.5" X 2.5" X 14GA TUBE	1
15	SERVICE DOOR / WINDOW FRAMING	2.5" X 2.5" X 14GA TUBE	1
16	ANGLE BRACKET	2" X 2" X 2" LG. 14GA ANGLE	7
17	STRAIGHT BRACKET	2" X 2" X 4" LG. 14GA PLATE	6
18	PB SUPPORT	2.5" X 2.5" X 14GA TUBE	1
19	DIAGONAL BRACE	2" X 2" X 14 GA TUBE	3
20	GABLE BRACE	2" X 2" X 14 GA TUBE	3
21	DB BRACKET	2.25" X 2.25" X 6" LG. 14GA ANGLE	9
22	TRUSS SPACER	2.5" X 2.5" X 14GA TUBE	1
23	ALL FASTENERS	#12 X 1" SELF-DRILL SCREWS (ESR-2196 OR EQ) W/ NEOPRENE/STEEL WASHER	

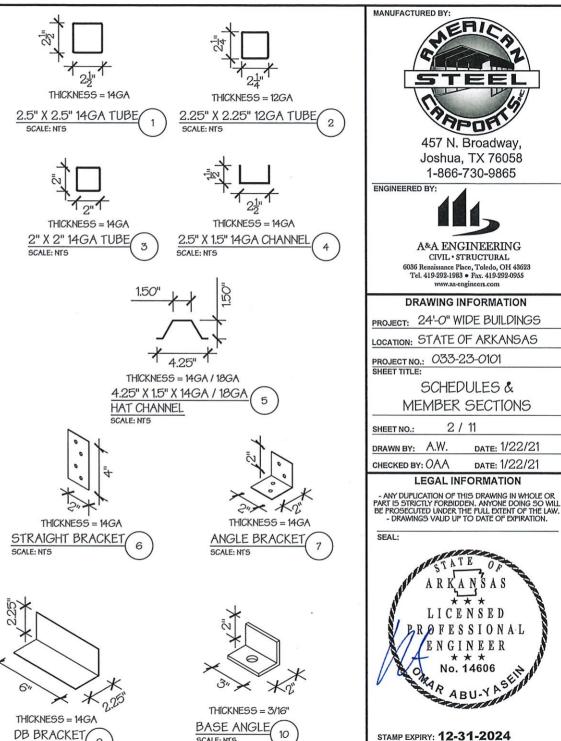
TABLE 2.2: SHEATHING FASTENER SCHEDULE

LOCATION	CORNER PANELS	SIDE LAPS	EDGE LAPS	ELSEWHERE
SPACING	9" C/C	MIN. 1	4½" C/C	9" C/C

FASTENER TYPE: #12X1" SELF-DRILL SCREWS (ESR-2196 OR EQ) W/ NEOPRENE/STEEL WASHER

*SEE TYP. SHEATHING FASTENER SCHEDULE DIAGRAM ON PAGE 6.

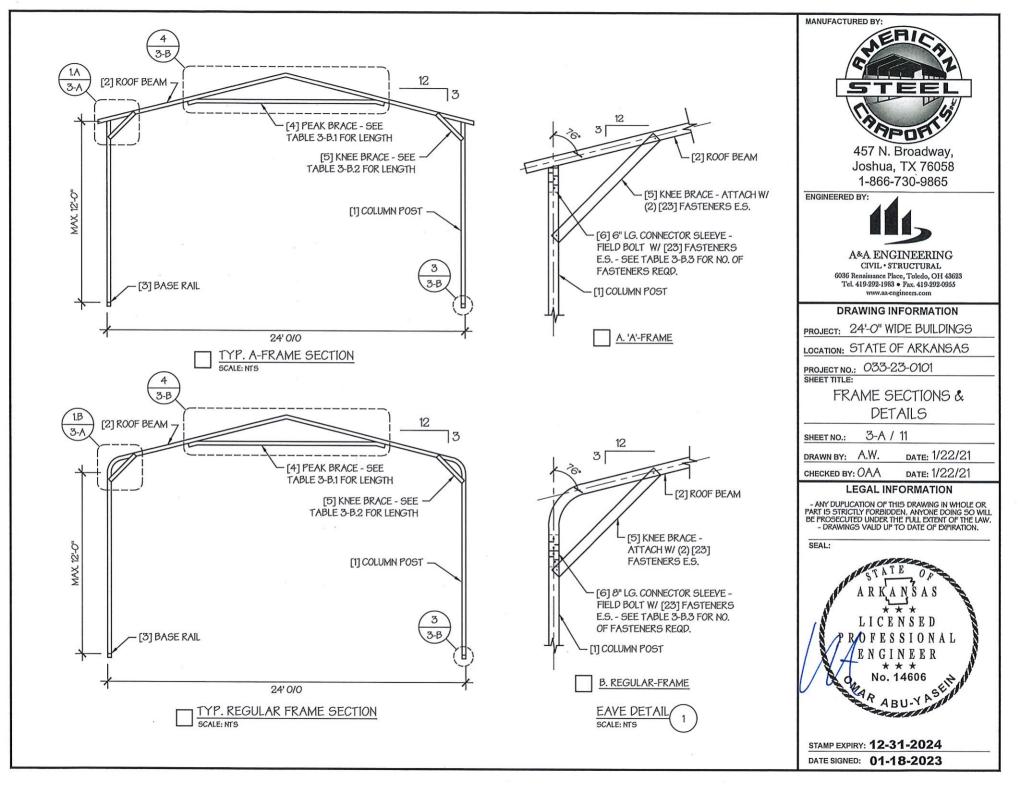




SCALE: NTS

DATE SIGNED: 01-18-2023

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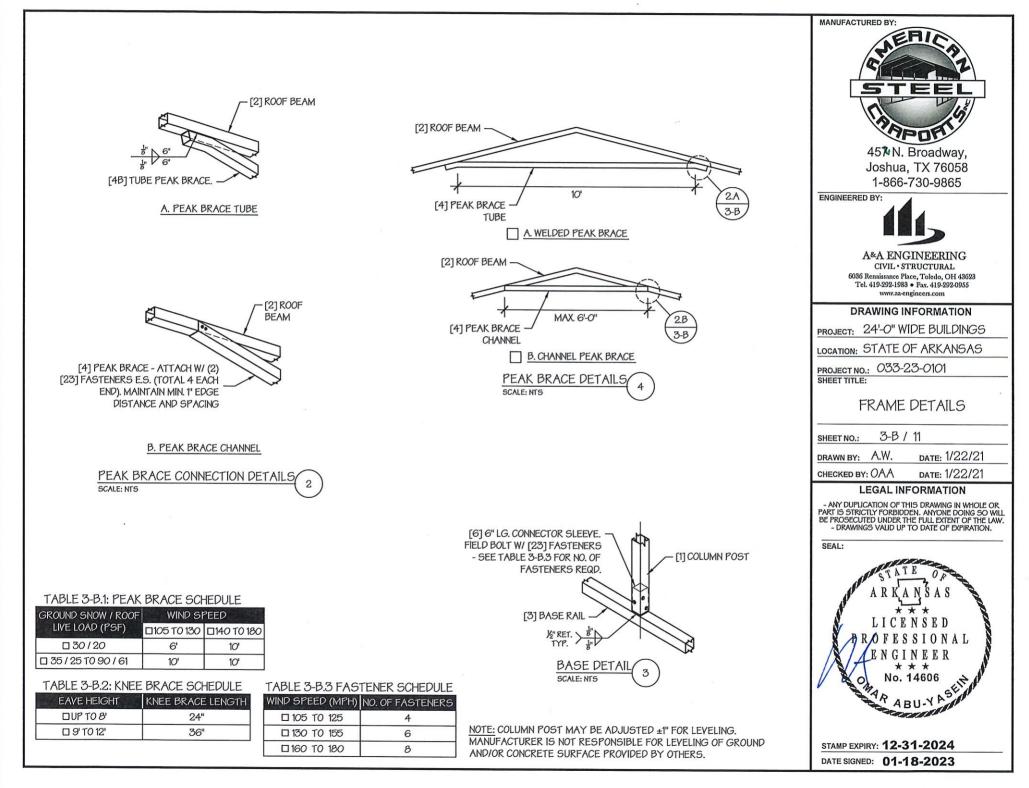


	TABLE 4:	FRAME	SPACIN	G CHART	/ SCHE	DULE											
	GROUND SNOW /			ENCLC	SED BUIL	DINGS			OPEN BUILDINGS								
	ROOF LIVE		WIND SPEED (MPH)							WIND SPEED (MPH)							
	LOAD (PSF)	□105	0 115	□130	□14 0	155	□165	180	0105	115	□130	□140	155	□165	 □180		
	□30/20	60	60	54/60	54	42	42	36	48	48	48	42	36	30	24		
₽ <u></u> 9	40/27	48/60	48/60	42/60	42/54	42	42	36	42	42	42	42	36	30	24		
HEIGHT = 10 12'-0"	□50/34	40/48	40/48	40/48	40/48	40/42	40/42	36	30	30	30	30	30	30	24		
뽀	60/41	36	36	36	36	36	36	36	30	30	30	30	30	30	24		
EAVE 10'-0"	0 70 / 47	30	30	30	30	30	30	30	24	24	24	24	24	24	24		
	080/54	24	24	24	24	24	24	24	24	24	18	18	18	18	18		
	90/61																
	30/20	60	60	54/60	54	48	42/48	42	54	54	48/54	42/54	36/48	36	30		
HEIGHT = T0 9'-0"	040/27	48/60	48/60	42/60	42/54	42/48	42/48	42	42	42	42	42	36/42	36	30		
50	□50/34	40/48	40/48	40/48	40/48	40/48	40/48	40/42	36	36	36	36	36	36	30		
用 つ	60/41	36	36	36	36	36	36	36	30	30	30	30	30	30	30		
EAVE 7:-0"	0 70 / 47	30	30	30	30	30	30	30	24	24	24	24	24	24	24		
	080/54	24	24	24	24	24	24	24	24	24	24	24	24	24	24		
	0 90 / 61																
	□30/20	60	60	54/60	54	48	42/48	42	60	54/60	48/60	42/54	36/48	36/42	36		
ي = 1	040/27	48/60	48/60	42/60	42/54	42/48	42/48	42	48	48	42/48	42/48	36/48	36/42	36		
50	□50/34	40/48	40/48	40/48	40/48	40/48	40/48	40/42	40/42	40/42	40/42	40/42	36/42	36	36		
: HEICHT TO 6'-0"	060/41	36	36	36	36	36	36	36	36	36	36	36	36	36	12.52.57		
EAVE UP T	070/47	30	30	30	30	30	30	30	30	30	30	30	30	30	30		
L L	□80/54	24	24	24	24	24	24	24	24	24	24	24	24	24	30		
	90/61								/					24	24		
							-			and the second second	and the second second	COLOR DE LA CAL					

NOTES:

1. FRAME SPACINGS ARE IN UNITS OF INCHES (IN).

2. WHERE TWO VALUES ARE SHOWN, THE HIGHER VALUE CAN ONLY BE USED FOR VERTICAL SHEATHING.

3. SNOW LOADS AND ROOF LIVE LOADS ARE IN POUNDS PER SQUARE FOOT (PSF). WIND SPEED IS 3 SEC. GUST IN MILES PER HOUR (MPH).

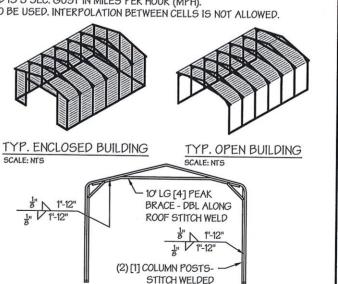
4. FOR VALUES THAT LIE BETWEEN TWO CELLS, THE HIGHER (MORE STRINGENT) VALUE HAS TO BE USED. INTERPOLATION BETWEEN CELLS IS NOT ALLOWED.

ENCLOSURE CLASSIFICATION:

- 1. <u>ENCLOSED BUILDING</u> = ALL 4 WALLS FULLY ENCLOSED WITH DOORS/WINDOWS = USE ENCLOSED BUILDING SPACING CHART.
- 2. OPEN BUILDING = ALL 4 WALLS FULLY OPEN = USE OPEN BUILDING SPACING CHART.
- 3. <u>3FT PARTIALLY ENCLOSED</u> = BOTH END-WALLS FULLY OPEN, WITH BOTH SIDE-WALLS ONLY 3FT ENCLOSED = USE OPEN BUILDING SPACING CHART.
- 4. <u>PARTIALLY ENCLOSED</u> = BOTH END-WALLS FULLY OPEN, WITH BOTH SIDE-WALLS ENCLOSED MORE THAN 3FT = START WITH OPEN BUILDING SPACING CHART AND THEN REDUCE SPACING BY 6".
- <u>3 SIDED ENCLOSED</u> = ALL WALLS ARE ENCLOSED EXCEPT FOR 1 END-WALL = START WITH ENCLOSED BUILDING SPACING + THE OPEN END FRAME MUST HAVE EITHER A GABLED END OR HAVE DOUBLED WELDED LEGS & ROOF.
- 6. FOR ALL SHEATHING ENCLOSURES NOT LISTED ABOVE, REFER TO SHEET 5 FOR SPACING AND DESIGN REQUIREMENTS.

GENERAL NOTES:

- <u>THE MAX. BUILDING LENGTH FOR ENCLOSED BUILDINGS IS 50'-0"</u>. THIS CAN BE INCREASED BY ADDING A DOUBLE FRAME AT THE CENTER TO BREAK THE LENGTH OF THE BUILDING.
- 2. BUILDINGS WITH <u>PARTIALLY ENCLOSED END WALLS</u> NEED TO HAVE SIDE WALL BRACING TO SUPPORT THE PARTIALLY ENCLOSED END WALL. (SEE FIGURE A ON SHEET 5).
- 3. ALL BUILDINGS WITH AN OPEN END WALL MUST HAVE A 10'-O" TUBE PEAK BRACE.



TYP. OPEN END WALL ON 3

SIDE ENCLOSED BUILDING

MANUFACTURED BY: STEEL 457 N. Broadway, Joshua, TX 76058 1-866-730-9865 ENGINEERED BY:
A&A ENGINEERING CIVIL • STRUCTURAL 6036 Renaissance Place, Toledo, OH 43623 Tel. 419-292-1983 • Fax. 419-292-0955 www.aa-engineers.com
DRAWING INFORMATION
PROJECT: 24'-O" WIDE BUILDINGS
LOCATION: STATE OF ARKANSAS
PROJECT NO.: 033-23-0101 SHEET TITLE:
SPACING SCHEDULES
& ENCLOSURE NOTES
Sheet no.: 4 / 11
DRAWN BY: A.W. DATE: 1/22/21
CHECKED BY: OAA DATE: 1/22/21
LEGAL INFORMATION
- ANY DUPLICATION OF THIS DRAWING IN WHOLE OR PART IS STRICTLY FORBIDDEN. ANYONE DOING SO WILL BE PROSECUTED UNDER THE FULL EXTENT OF THE LAW. - DRAWINGS VALID UP TO DATE OF EXPIRATION. SEAL:
A R KA N S A S A R KA N S A S X * * L I C E N S E D P R O F E S S I O N A L E N G I N E E R X * * No. 14606 MARY A SU-Y A SERVICE
STAMP EXPIRY: 12-31-2024 DATE SIGNED: 01-18-2023

TABLE 5.1: PURLIN SPACING SCHEDULE

	INDLL J.I.	TUK	LINE	IN AC	ING	SCHI	EDUL	E
	GROUND SNOW /		14GA	. HAT	CHA	NNEL	PURI	LIN
	ROOF LIVE		۷	VIND S	PEED	(MPH	-1)	
	LOAD (PSF)	105	115	130	140	155	165	180
(iii	0 30 / 20	54	48	42	36	30	24	24
ONI	0 40 / 27	42	42	42	36	30	24	24
FRAME SPACING:	050/34	40	40	40	36	30	24	24
E SF	0 60 / 41	36	36	36	36	30	24	24
E ∎	070/47	32	32	32	32	30	24	24
RA	080/54	30	30	30	30	30	24	24
ш <u> </u>	0 90 / 61	24	24	24	24	24	24	24
iii	030/20	54	48	42	42	36	30	30
ONIC	040/27	42	42	42	42	36	30	30
AC	050/34	40	40	40	40	36	30	30
FRAME SPACING: ■ 4'-6"	0 60 / 41	36	36	36	36	36	30	30
ME	070/47	32	32	32	32	32	30	30
KA	080/54	32	32	32	32	32	30	30
ш	0 90 / 61	30	30	30	30	30	30	30
i	030/20	54	48	42	42	36	36	30
DNI	040/27	42	42	42	42	36	36	30
FRAME SPACING: 4'-0"	050/34	40	40	40	40	36	36	30
П 5P	0 60 / 41	36	36	36	36	36	36	30
E E	0 70 / 47	32	32	32	32	32	32	30
KA	080/54	32	32	32	32	32	32	30
ш	0 90 / 61	30	30	30	30	30	30	30
iii	030/20	54	48	42	42	36	36	30
DNI	0 40/27	42	42	42	42	36	36	30
AC "	050/34	40	40	40	40	36	36	30
E 5P/ 3-6"	0 60 / 41	36	36	36	36	36	36	30
FRAME SPACING: ■ 3'-6"	0 70 / 47	32	32	32	32	32	32	30
KA	080/54	32	32	32	32	32	32	30
<u> </u>	0 90 / 61	30	30	30	30	30	30	30
(1)	030/20	54	48	42	42	36	36	30
ANC AN	0 40 / 27	42	42	42	42	36	36	30
ACING	050/34	40	40	40	40	36	36	30
SF L	0 60 / 41	36	36	36	36	36	36	30
FRAME SPACING: 1 3'-O" OR LOWER	0 70 / 47	32	32	32	32	32	32	30
S-S-S	080/54	32	32	32	32	32	32	30
" □	0 90 / 61	30	30	30	30	30	30	30

WIND SPEED (MPH) FRAME SPACING 180 105 130 115 140 155 165 П5'-O" 48 36 30 60 24 24 18 TEE 60 04-6" 60 48 42 36 30 24 04-0" 60 60 54 54 42 36 30 03-6" 60 60 54 54 48 42 42 60 60 02'-0' TO 3'-0" 54 54 48 42 42 457 N. Broadway, NOTES: Joshua, TX 76058 1 GIRT SPACING UNITS ARE IN INCHES. 2. 1-866-730-9865 THIS SCHEDULE IS TO BE USED FOR BOTH 14GA ENGINEERED BY: FRAME SPACING NEEDS TO BE DETERMINED 3. FROM TABLE 4. A&A ENGINEERING CIVIL · STRUCTURAL 6036 Renaissance Place, Toledo, OH 43623 Tel. 419-292-1983 • Fax. 419-292-0955 www.aa-engineers.com DRAWING INFORMATION OPEN OR PARTIAL PROJECT: 24'-O" WIDE BUILDINGS SIDE WALLS (1) ENCLOSED END WALL DIAGONAL BRACING LOCATION: STATE OF ARKANSAS FIGURE A AT ENCLOSED END WALL PROJECT NO .: 033-23-0101 SHEET TITLE: PURLIN & GIRT SPACING SCHEDULES 5/11 SHEET NO .: ENCLOSED SIDE WALLS DRAWN BY: A.W. PARTIAL END WALLS DIAGONAL BRACING CHECKED BY: OAA FIGURE B AT PARTIAL END WALL LEGAL INFORMATION - ANY DUPLICATION OF THIS DRAWING IN WHOLE OR PART 19 STRICTLY FORBIDDEN, ANYONE DOING SO WILL BE PROSECUTED UNDER THE FULL EXTENT OF THE LAW, - DRAWINGS VALID UP TO DATE OF EXPIRATION. SEAL: (1) ENCLOSED SIDE WALL PARTIAL DIAGONAL BRACING END WALL AT PARTIAL END WALL FIGURE C ICENSED * * * PARTIAL No. 14606 END WALL PARTIALLY ENCLOSED ABU-Y SIDE WALLS and the FIGURE D

DIAGONAL BRACING

AT PARTIAL END WALL

TABLE 5.2: GIRT SPACING SCHEDULF

NOTES:

PURLIN SPACING UNITS ARE IN INCHES. 1.

FRAME SPACING NEEDS TO BE DETERMINED FROM TABLE 4. 2.

IRREGULAR BUILDING NOTES:

FIGURES A, B, C & D ON THE RIGHT INDICATE EXAMPLES OF IRREGULAR BUILDINGS.

FOR IRREGULAR BUILDINGS, FRAME SPACING MUST BE REDUCED BY 6" FROM OPEN BUILDING SPACING TABLE. SEE SHEET 2. 4 FOR OPEN BUILDING TABLE.

3. SITE SPECIFICS MAY ALLOW FOR ALTERNATIVE SPACING.

IRREGULAR BUILDING & BUILDINGS W/ MORE THAN 2 SIDE OPENINGS MUST HAVE A 10' TUBE PEAK BRACE ON ALL FRAMES. 4.

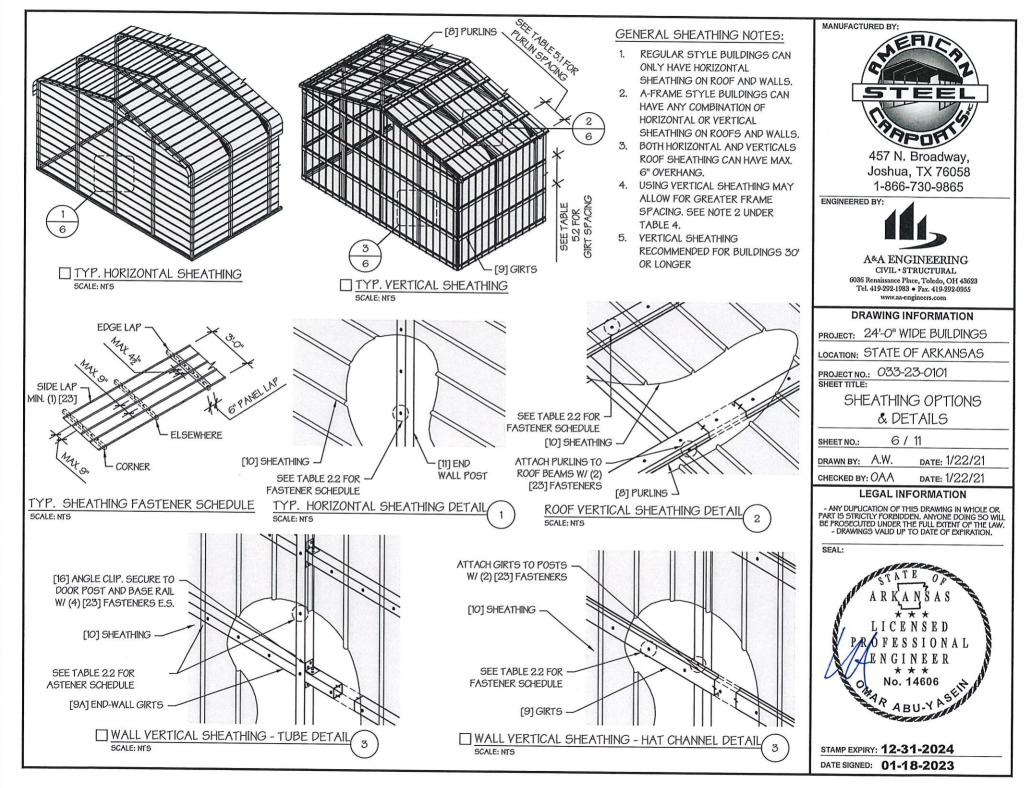
STAMP EXPIRY: 12-31-2024 DATE SIGNED: 01-18-2023

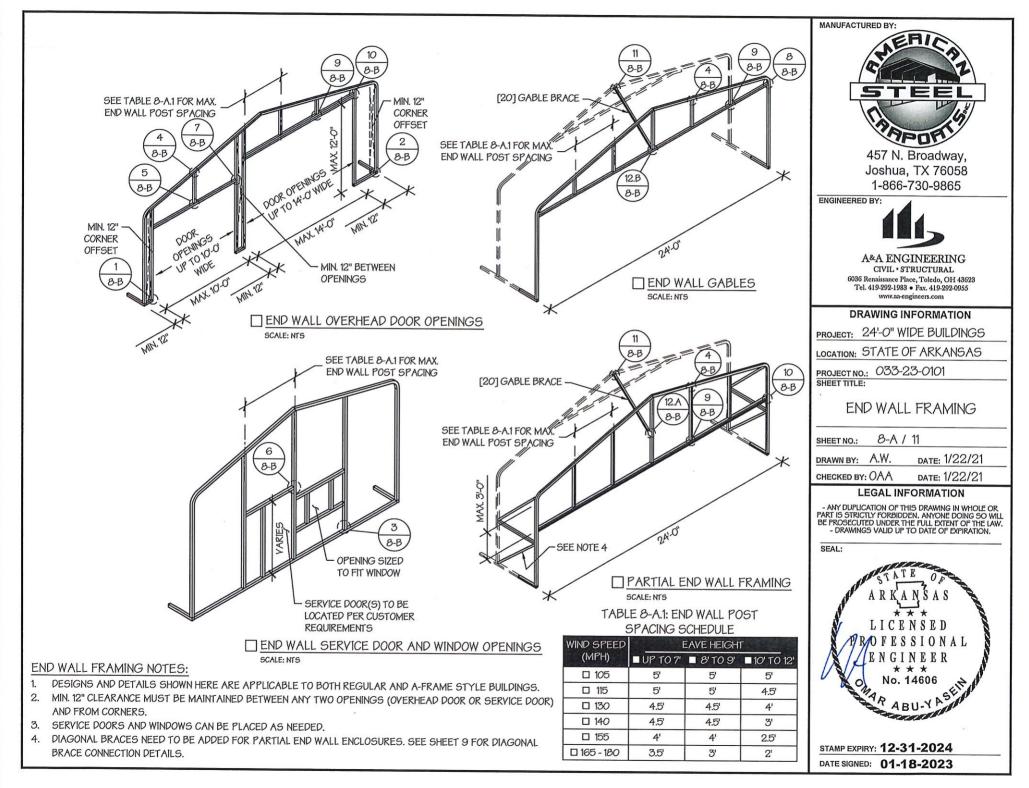
DATE: 1/22/21

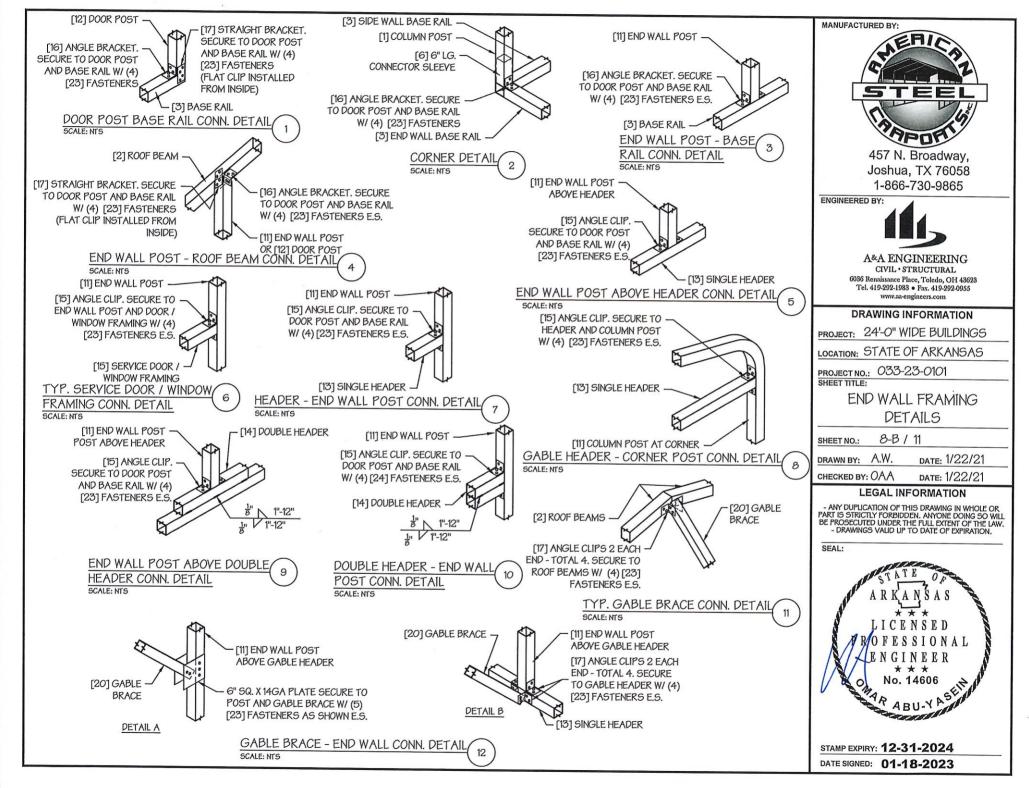
DATE: 1/22/21

SAS

SIONAL INEER

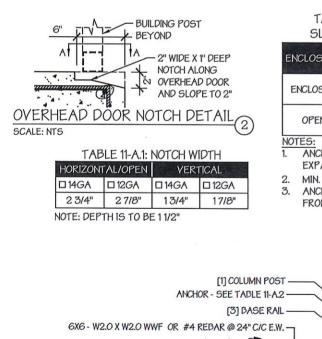








- 1 DESIGNS SHOWN ON THIS SHEET ARE FOR CONCRETE SLAB FOUNDATION. ANY OF THE FOUNDATIONS SHOWN ON SHEETS 11-A THRU C CAN BE USED.
- 2. CONCRETE ANCHORS SHALL BE LOCATED NEXT TO EVERY POST AND ON EITHER SIDE OF OPENINGS, TWO ANCHORS SHALL BE INSTALLED AT CORNERS OF ENCLOSED BUILDINGS WITH END WALLS - ONE ON EACH BASE RAIL. IN LOCATIONS REQUIRING TWO ANCHORS DUE TO WIND, ONE ANCHOR IS TO BE ON EACH SIDE OF THE COLUMN POST.
- 3. ANCHORS IN CLOSE PROXIMITY TO EACH OTHER MUST HAVE A MIN. 4" SPACING. ,
- 4. MIN. NUMBER OF CONCRETE ANCHORS PER POST SHALL BE AS SHOWN IN TABLE 11-A.2.
- 5. THE SIZE OF THE SLAB SHALL BE THE SIZE (WIDTH AND LENGTH) OF THE BUILDING PLUS 5%" FOR 14GA MATERIAL AND 5%" FOR 12GA MATERIAL.
- 6. DEPTH OF SLAB TURN DOWN FOOTING SHALL BE GREATER THAN FROST DEPTH SPECIFIED PER LOCAL CODE.
- 7. CONTROL JOINTS SHALL BE PLACED SO AS TO LIMIT MAX. SLAB SPANS TO 20' IN EACH DIRECTION.
- 8. ASSUMED SOIL BEARING CAPACITY IS TO BE A MIN. OF 1500 PSF.
- 9. CONCRETE STRENGTH TO BE A MIN OF 2500 PSI @ 28 DAYS.



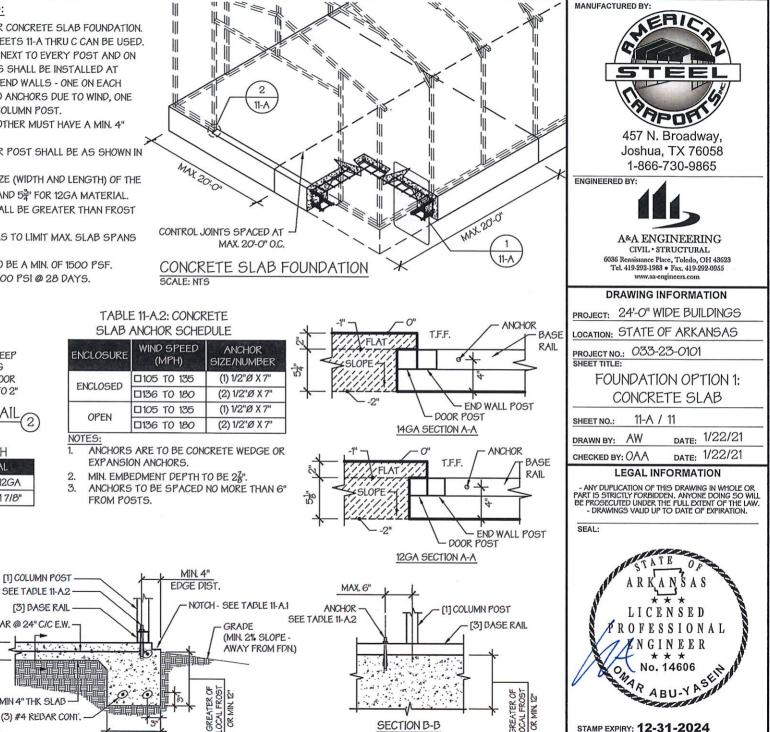
NOTCH EDGE DETAIL

EDGE OFFSET DETAIL

SCALE: NTS

MIN 4" THK SLAD

12"



DATE SIGNED: 01-18-2023

	R	EGULAR	2 / A-F	RAN	AF				457 N. Broadway, Joshua, TX 76058 1-866-730-9865
		24'-(-		115
DESIGN NOTES		ORT ST DESIGN CRI1		BUILI	DINGS) DRAWING	NDEX		A&A ENGINEERING CIVIL • STRUCTURAL 6036 Renaissance Place, Toledo, OH 43623 Tel. 419-292-1983 • Fax. 419-292.0955 WWW.aa-engineers.com
 ALL CONSTRUCTION SHALL BE PROVIDED IN ACCO WITH IBC 2012, OSHA, AISC 360, AISI 100, ASCE 7-1 CODES AND ALL APPLICABLE LOCAL REQUIREME BASE CONNECTIONS SHALL BE PROVIDED AS SH FOUNDATION DETAILS SHEET. ALL MATERIALS IDENTIFIED BY MANUFACTURER MAY BE SUBSTITUTED WITH MATERIAL EQUAL OR E ORIGINAL. ALL SHOP CONNECTIONS SHALL BE WELDED CON ALL FIELD CONNECTIONS SHALL BE #12X1" SDS (I OR EQ). 	O, AWSD NTS. OWN ON NAME XCEEDING NECTIONS.	3. SNOW LO, GROUND IMPORTA THERMAL EXPOSUR	U I I I I I I I I I I I I I I I I I I I	J (CARPOR) $Q = 4 PSF$ (Lr) $r = 20 - 6$ $AS PER S$ $SEE TABLE$ $Q = 20 - 5$ $S = 0.8$ $Ct = 1.2$ $Cc = 1.0$	61 PSF 6NOW LOAD E 4)	SPACING SC & ENCLOS	& MEMBER - TIONS & DETAILS HEDULES - SURE NOTES RT SCHEDULES DPTIONS RAMING GS	1 2 3-A, 3-B 4 5 6 7-A, 7-B	DRAWING INFORMATION PROJECT: 24'-O" WIDE BUILDINGS LOCATION: STATE OF ARKANSAS PROJECT NO.: 033-23-0101 SHEET TITLE: COVER SHEET SHEET NO.: 1 / 11 DRAWN BY: A.W. DATE: 1/22/21 CHECKED BY: OAA DATE: 1/22/21 LEGAL INFORMATION
 STEEL SHEATHING SHALL BE 29GA. CORRUGATE OR PAINTED STEEL - MAIN RIB HT. 3/4" (FY=80K5I) ALL STRUCTURAL LIGHT GAUGE TUBING AND CH. SHALL BE GRADE 50 STEEL. STRUCTURAL TUBE TS2 1/2"X2 1/2" - 14GA. IS EQ TO TS2 1/4"X2 1/4" - 12GA AND EITHER ONE MAY BE LIEU OF THE OTHER. ALL DESIGN CRITERIA MUST BE INCREASED TO HIGHER INCREMENT BASED ON THE TABLES ON PAGE INTERPOLATION IS ALLOWED. 	OR EQ. ANNELS UIVALENT USED IN THE NEXT	EXPOSUR 5. SEISMIC DESIGN C IMPORTA LOAD COMBINATIO 1. D + (Lr OR 2. D + (0.6W 3. D + 0.75 (E C OAD (E) ATEGORY D NCE FACTOR 10 DNS:	e = 1.00	- 180 MPH Lr OR 5)	& OPENING CORNER BRA	GS CING DETAILS AN-TO ADDITION	8-A, 8-B 9 10 11-A TO 11-D	- ANY DUPLICATION OF THIS DRAWING IN WHOLE OR PART IS STRICTLY PORBIDDEN. ANYONE DOING SO WILL BE PROSECUTED UNDER THE FULL EXTENT OF THE LAW. - DRAWINGS VALID UP TO DATE OF EXPIRATION. SEAL: A R K A N S A S * * * * L I C E N S E D P R O F E S S I O N A L E N G I N E E R * * * No. 14606
CUSTOMER INFORMATION OWNER: ADDRESS:	DES GROUND SNO ROOF LIVE L BASIC WIND	.OAD:	BI <u>WIDTH:</u> LENGTH: HEIGHT:	UILDING	INFORMATIC FRAME TYPE: ENCLOSURE TYPE:	A-FRAME REGULAR FULL PARTIAL OPEN	CERTIFICATION NOTICE DATE OF PLANS EXPIRATION: 01-18 CERTIFICATION ON THESE VALID FOR ONE YEAR FROM	8-2024 E DRAWINGS IS	STAMP EXPIRY: 12-31-2024 DATE SIGNED: 01-18-2023

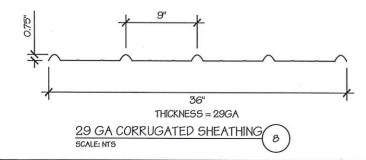
	TABLE 2.1:	MEMBER PROPERTIES	
NO.	LABEL	PROPERTY	DETAIL NO.
1	COLUMN POST	2.5" X 2.5" X 14GA TUBE	1
2	ROOF BEAM	2.5" X 2.5" X 14GA TUBE	1
3	BASE RAIL	2.5" X 2.5" X 14GA TUBE	1
4	PEAK BRACE	2.5" X 2.5" 14GA CHANNEL	4
5	KNEE BRACES	2.5" X 1.5" 14GA CHANNEL	4
6	CONNECTOR SLEEVE	2.25" X 2.25" X 12GA TUBE	2
7	BASE ANGLE	2" X 2" X 3" LG. 3/16" ANGLE	10
8	PURLIN	4.25" X 1.5" X 14GA / 18GA HAT CHANNEL	5
9	GIRT	4.25" X 1.5" X 14GA / 18GA HAT CHANNEL	5
9A	OPT. END WALL GIRT	2.5" X 1.5" 14GA CHANNEL	1
10	SHEATHING	29 GA CORRUGATED SHEET	8
11	END WALL POST	2.5" X 2.5" X 14GA TUBE	1
12	DOOR POST	2.5" X 2.5" X 14GA TUBE	1
13	SINGLE HEADER	2.5" X 2.5" X 14GA TUBE	1
14	DOUBLE HEADER	DBL. 2.5" X 2.5" X 14GA TUBE	1
15	SERVICE DOOR / WINDOW FRAMING	2.5" X 2.5" X 14GA TUBE	1
16	ANGLE BRACKET	2" X 2" X 2" LG. 14GA ANGLE	7
17	STRAIGHT BRACKET	2" X 2" X 4" LG. 14GA PLATE	6
18	PB SUPPORT	2.5" X 2.5" X 14GA TUBE	1
19	DIAGONAL BRACE	2" X 2" X 14 GA TUBE	3
20	GABLE BRACE	2" X 2" X 14 GA TUBE	3
21	DB BRACKET	2.25" X 2.25" X 6" LG. 14GA ANGLE	9
22	TRUSS SPACER	2.5" X 2.5" X 14GA TUBE	1
23	ALL FASTENERS	#12 X 1" SELF-DRILL SCREWS (ESR-2196 OR EQ) W/ NEOPRENE/STEEL WASHER	

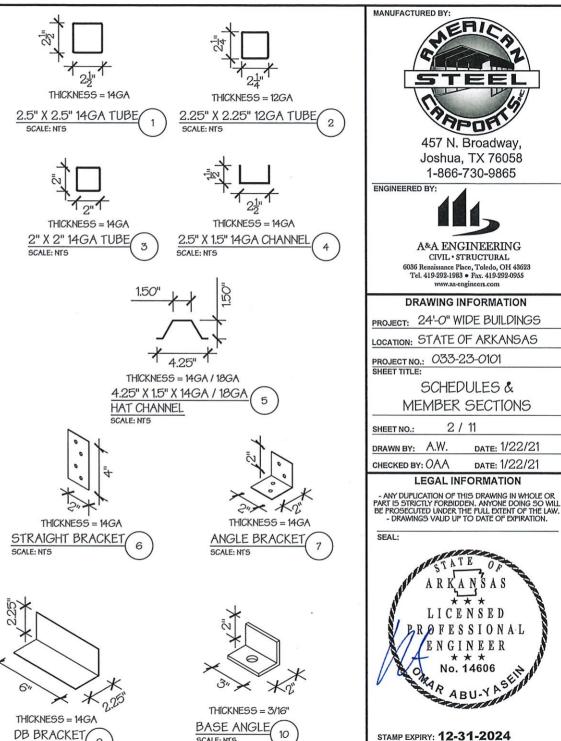
TABLE 2.2: SHEATHING FASTENER SCHEDULE

LOCATION	CORNER PANELS	SIDE LAPS	EDGE LAPS	ELSEWHERE
SPACING	9" C/C	MIN. 1	4½" C/C	9" C/C

FASTENER TYPE: #12X1" SELF-DRILL SCREWS (ESR-2196 OR EQ) W/ NEOPRENE/STEEL WASHER

*SEE TYP. SHEATHING FASTENER SCHEDULE DIAGRAM ON PAGE 6.

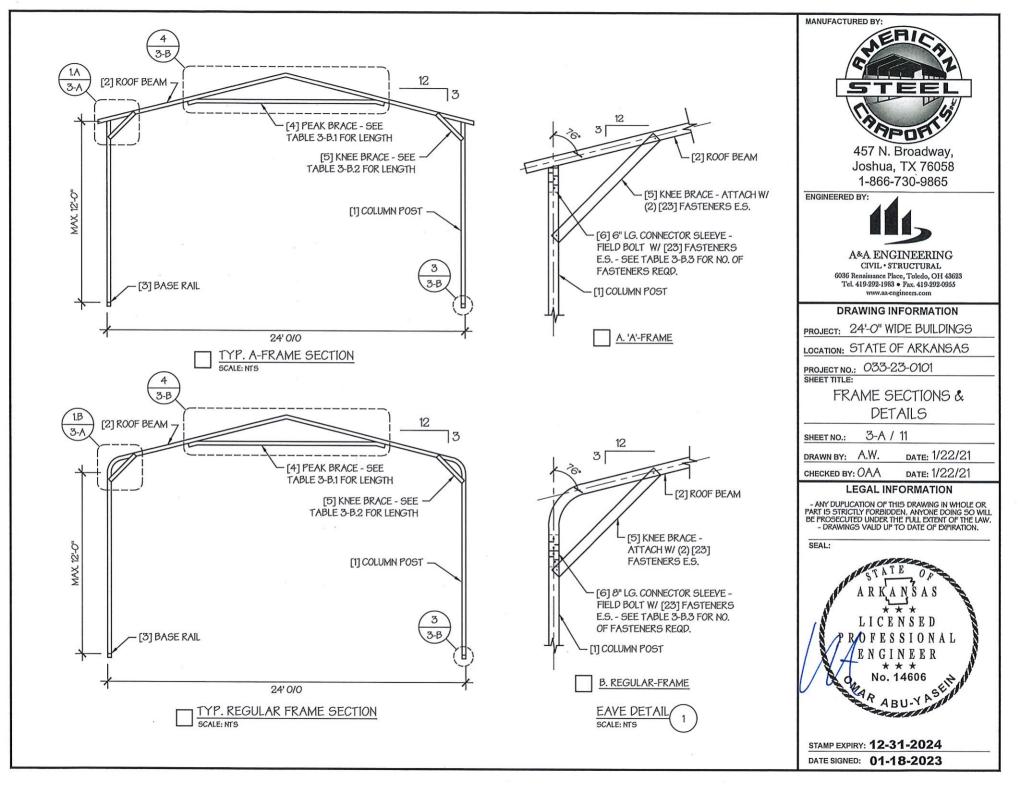




SCALE: NTS

DATE SIGNED: 01-18-2023

9



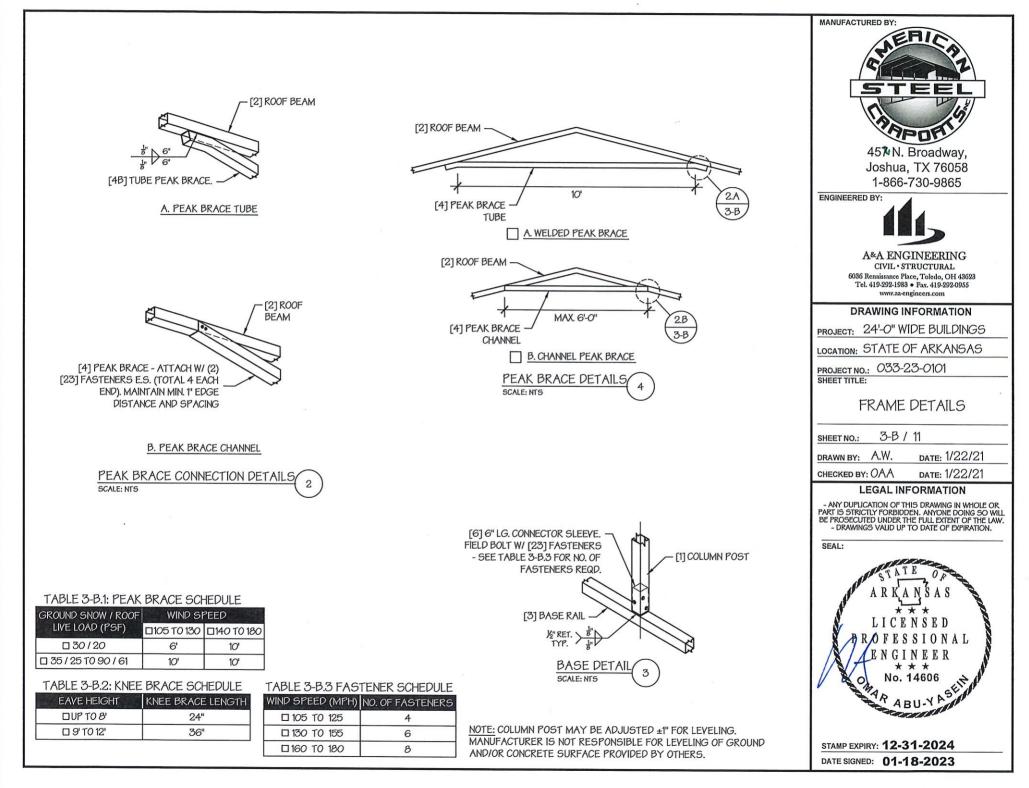


	TABLE 4:	FRAME	SPACIN	G CHART	/ SCHE	DULE											
	GROUND SNOW /			ENCLC	SED BUIL	DINGS			OPEN BUILDINGS								
	ROOF LIVE		WIND SPEED (MPH)							WIND SPEED (MPH)							
	LOAD (PSF)	□105	0 115	□130	□14 0	155	□165	180	0105	115	□130	□140	155	□165	 □180		
	□30/20	60	60	54/60	54	42	42	36	48	48	48	42	36	30	24		
₽ <u></u> 9	40/27	48/60	48/60	42/60	42/54	42	42	36	42	42	42	42	36	30	24		
HEIGHT = 10 12'-0"	□50/34	40/48	40/48	40/48	40/48	40/42	40/42	36	30	30	30	30	30	30	24		
뽀	60/41	36	36	36	36	36	36	36	30	30	30	30	30	30	24		
EAVE 10'-0"	0 70 / 47	30	30	30	30	30	30	30	24	24	24	24	24	24	24		
	080/54	24	24	24	24	24	24	24	24	24	18	18	18	18	18		
	90/61																
	30/20	60	60	54/60	54	48	42/48	42	54	54	48/54	42/54	36/48	36	30		
HEIGHT = T0 9'-0"	040/27	48/60	48/60	42/60	42/54	42/48	42/48	42	42	42	42	42	36/42	36	30		
50	□50/34	40/48	40/48	40/48	40/48	40/48	40/48	40/42	36	36	36	36	36	36	30		
用 て	60/41	36	36	36	36	36	36	36	30	30	30	30	30	30	30		
EAVE 7:-0"	0 70 / 47	30	30	30	30	30	30	30	24	24	24	24	24	24	24		
	080/54	24	24	24	24	24	24	24	24	24	24	24	24	24	24		
	0 90 / 61																
	□30/20	60	60	54/60	54	48	42/48	42	60	54/60	48/60	42/54	36/48	36/42	36		
ي = 1	040/27	48/60	48/60	42/60	42/54	42/48	42/48	42	48	48	42/48	42/48	36/48	36/42	36		
50	□50/34	40/48	40/48	40/48	40/48	40/48	40/48	40/42	40/42	40/42	40/42	40/42	36/42	36	36		
: HEICHT TO 6'-0"	060/41	36	36	36	36	36	36	36	36	36	36	36	36	36	12.52.57		
EAVE UP T	070/47	30	30	30	30	30	30	30	30	30	30	30	30	30	30		
L L	□80/54	24	24	24	24	24	24	24	24	24	24	24	24	24	30		
	90/61								/					24	24		
							-			and the second second	and the second second	COLOR DE LA CAL					

NOTES:

1. FRAME SPACINGS ARE IN UNITS OF INCHES (IN).

2. WHERE TWO VALUES ARE SHOWN, THE HIGHER VALUE CAN ONLY BE USED FOR VERTICAL SHEATHING.

3. SNOW LOADS AND ROOF LIVE LOADS ARE IN POUNDS PER SQUARE FOOT (PSF). WIND SPEED IS 3 SEC. GUST IN MILES PER HOUR (MPH).

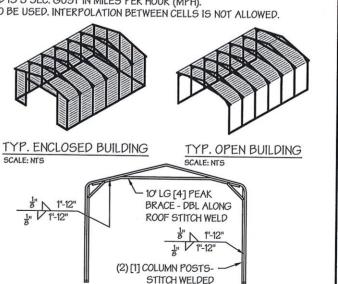
4. FOR VALUES THAT LIE BETWEEN TWO CELLS, THE HIGHER (MORE STRINGENT) VALUE HAS TO BE USED. INTERPOLATION BETWEEN CELLS IS NOT ALLOWED.

ENCLOSURE CLASSIFICATION:

- 1. <u>ENCLOSED BUILDING</u> = ALL 4 WALLS FULLY ENCLOSED WITH DOORS/WINDOWS = USE ENCLOSED BUILDING SPACING CHART.
- 2. OPEN BUILDING = ALL 4 WALLS FULLY OPEN = USE OPEN BUILDING SPACING CHART.
- 3. <u>3FT PARTIALLY ENCLOSED</u> = BOTH END-WALLS FULLY OPEN, WITH BOTH SIDE-WALLS ONLY 3FT ENCLOSED = USE OPEN BUILDING SPACING CHART.
- 4. <u>PARTIALLY ENCLOSED</u> = BOTH END-WALLS FULLY OPEN, WITH BOTH SIDE-WALLS ENCLOSED MORE THAN 3FT = START WITH OPEN BUILDING SPACING CHART AND THEN REDUCE SPACING BY 6".
- <u>3 SIDED ENCLOSED</u> = ALL WALLS ARE ENCLOSED EXCEPT FOR 1 END-WALL = START WITH ENCLOSED BUILDING SPACING + THE OPEN END FRAME MUST HAVE EITHER A GABLED END OR HAVE DOUBLED WELDED LEGS & ROOF.
- 6. FOR ALL SHEATHING ENCLOSURES NOT LISTED ABOVE, REFER TO SHEET 5 FOR SPACING AND DESIGN REQUIREMENTS.

GENERAL NOTES:

- <u>THE MAX. BUILDING LENGTH FOR ENCLOSED BUILDINGS IS 50'-0"</u>. THIS CAN BE INCREASED BY ADDING A DOUBLE FRAME AT THE CENTER TO BREAK THE LENGTH OF THE BUILDING.
- 2. BUILDINGS WITH <u>PARTIALLY ENCLOSED END WALLS</u> NEED TO HAVE SIDE WALL BRACING TO SUPPORT THE PARTIALLY ENCLOSED END WALL. (SEE FIGURE A ON SHEET 5).
- 3. ALL BUILDINGS WITH AN OPEN END WALL MUST HAVE A 10'-O" TUBE PEAK BRACE.



TYP. OPEN END WALL ON 3

SIDE ENCLOSED BUILDING

MANUFACTURED BY: STEEL 457 N. Broadway, Joshua, TX 76058 1-866-730-9865 ENGINEERED BY:
A&A ENGINEERING CIVIL • STRUCTURAL 6036 Renaissance Place, Toledo, OH 43623 Tel. 419-292-1983 • Fax. 419-292-0955 www.aa-engineers.com
DRAWING INFORMATION
PROJECT: 24'-O" WIDE BUILDINGS
LOCATION: STATE OF ARKANSAS
PROJECT NO.: 033-23-0101 SHEET TITLE:
SPACING SCHEDULES
& ENCLOSURE NOTES
Sheet no.: 4 / 11
DRAWN BY: A.W. DATE: 1/22/21
CHECKED BY: OAA DATE: 1/22/21
LEGAL INFORMATION
- ANY DUPLICATION OF THIS DRAWING IN WHOLE OR PART IS STRICTLY FORBIDDEN. ANYONE DOING SO WILL BE PROSECUTED UNDER THE FULL EXTENT OF THE LAW. - DRAWINGS VALID UP TO DATE OF EXPIRATION. SEAL:
A R KA N S A S A R KA N S A S X * * L I C E N S E D P R O F E S S I O N A L E N G I N E E R X * * No. 14606 MARY A SU-Y A SERVICE
STAMP EXPIRY: 12-31-2024 DATE SIGNED: 01-18-2023

TABLE 5.1: PURLIN SPACING SCHEDULE

	INDLL J.I.	TORLIN SPACING SCHEDULE										
	GROUND SNOW /		14GA	. HAT	CHA	NNEL	PURI	LIN				
	ROOF LIVE		۷	VIND S	SPEED	(MPH	1)					
	LOAD (PSF)	105	115	130	140	155	165	180				
(iii	0 30 / 20	54	48	42	36	30	24	24				
ONI	0 40 / 27	42	42	42	36	30	24	24				
FRAME SPACING:	050/34	40	40	40	36	30	24	24				
E SF	0 60 / 41	36	36	36	36	30	24	24				
E ∎	070/47	32	32	32	32	30	24	24				
RA	080/54	30	30	30	30	30	24	24				
ш <u> </u>	0 90 / 61	24	24	24	24	24	24	24				
iii	030/20	54	48	42	42	36	30	30				
ONIC	040/27	42	42	42	42	36	30	30				
AC	050/34	40	40	40	40	36	30	30				
FRAME SPACING: ■ 4'-6"	0 60 / 41	36	36	36	36	36	30	30				
ME	070/47	32	32	32	32	32	30	30				
KA	080/54	32	32	32	32	32	30	30				
ш	0 90 / 61	30	30	30	30	30	30	30				
iii	030/20	54	48	42	42	36	36	30				
FRAME SPACING: 4'-0"	040/27	42	42	42	42	36	36	30				
AC	050/34	40	40	40	40	36	36	30				
П 5P	0 60 / 41	36	36	36	36	36	36	30				
E E	0 70 / 47	32	32	32	32	32	32	30				
KA	080/54	32	32	32	32	32	32	30				
ш	0 90 / 61	30	30	30	30	30	30	30				
iii	030/20	54	48	42	42	36	36	30				
DNI	0 40/27	42	42	42	42	36	36	30				
AC "	050/34	40	40	40	40	36	36	30				
E 5P/ 3-6"	0 60 / 41	36	36	36	36	36	36	30				
FRAME SPACING: ■ 3'-6"	0 70 / 47	32	32	32	32	32	32	30				
KA	080/54	32	32	32	32	32	32	30				
<u> </u>	0 90 / 61	30	30	30	30	30	30	30				
(1)	030/20	54	48	42	42	36	36	30				
ANC AN	0 40 / 27	42	42	42	42	36	36	30				
ACING	050/34	40	40	40	40	36	36	30				
SF L	0 60 / 41	36	36	36	36	36	36	30				
FRAME SPACING: 1 3'-O" OR LOWER	0 70 / 47	32	32	32	32	32	32	30				
S-S-S	080/54	32	32	32	32	32	32	30				
" □	0 90 / 61	30	30	30	30	30	30	30				

WIND SPEED (MPH) FRAME SPACING 180 105 130 115 140 155 165 П5'-O" 48 36 30 60 24 24 18 TEE 60 04-6" 60 48 42 36 30 24 04-0" 60 60 54 54 42 36 30 03-6" 60 60 54 54 48 42 42 60 60 02'-0' TO 3'-0" 54 54 48 42 42 457 N. Broadway, NOTES: Joshua, TX 76058 1 GIRT SPACING UNITS ARE IN INCHES. 2. 1-866-730-9865 THIS SCHEDULE IS TO BE USED FOR BOTH 14GA ENGINEERED BY: FRAME SPACING NEEDS TO BE DETERMINED 3. FROM TABLE 4. A&A ENGINEERING CIVIL · STRUCTURAL 6036 Renaissance Place, Toledo, OH 43623 Tel. 419-292-1983 • Fax. 419-292-0955 www.aa-engineers.com DRAWING INFORMATION OPEN OR PARTIAL PROJECT: 24'-O" WIDE BUILDINGS SIDE WALLS (1) ENCLOSED END WALL DIAGONAL BRACING LOCATION: STATE OF ARKANSAS FIGURE A AT ENCLOSED END WALL PROJECT NO .: 033-23-0101 SHEET TITLE: PURLIN & GIRT SPACING SCHEDULES 5/11 SHEET NO .: ENCLOSED SIDE WALLS DRAWN BY: A.W. PARTIAL END WALLS DIAGONAL BRACING CHECKED BY: OAA FIGURE B AT PARTIAL END WALL LEGAL INFORMATION - ANY DUPLICATION OF THIS DRAWING IN WHOLE OR PART 19 STRICTLY FORBIDDEN, ANYONE DOING SO WILL BE PROSECUTED UNDER THE FULL EXTENT OF THE LAW, - DRAWINGS VALID UP TO DATE OF EXPIRATION. SEAL: (1) ENCLOSED SIDE WALL PARTIAL DIAGONAL BRACING END WALL AT PARTIAL END WALL FIGURE C ICENSED * * * PARTIAL No. 14606 END WALL PARTIALLY ENCLOSED ABU-Y SIDE WALLS and the FIGURE D

DIAGONAL BRACING

AT PARTIAL END WALL

TABLE 5.2: GIRT SPACING SCHEDULF

NOTES:

PURLIN SPACING UNITS ARE IN INCHES. 1.

FRAME SPACING NEEDS TO BE DETERMINED FROM TABLE 4. 2.

IRREGULAR BUILDING NOTES:

FIGURES A, B, C & D ON THE RIGHT INDICATE EXAMPLES OF IRREGULAR BUILDINGS.

FOR IRREGULAR BUILDINGS, FRAME SPACING MUST BE REDUCED BY 6" FROM OPEN BUILDING SPACING TABLE. SEE SHEET 2. 4 FOR OPEN BUILDING TABLE.

3. SITE SPECIFICS MAY ALLOW FOR ALTERNATIVE SPACING.

IRREGULAR BUILDING & BUILDINGS W/ MORE THAN 2 SIDE OPENINGS MUST HAVE A 10' TUBE PEAK BRACE ON ALL FRAMES. 4.

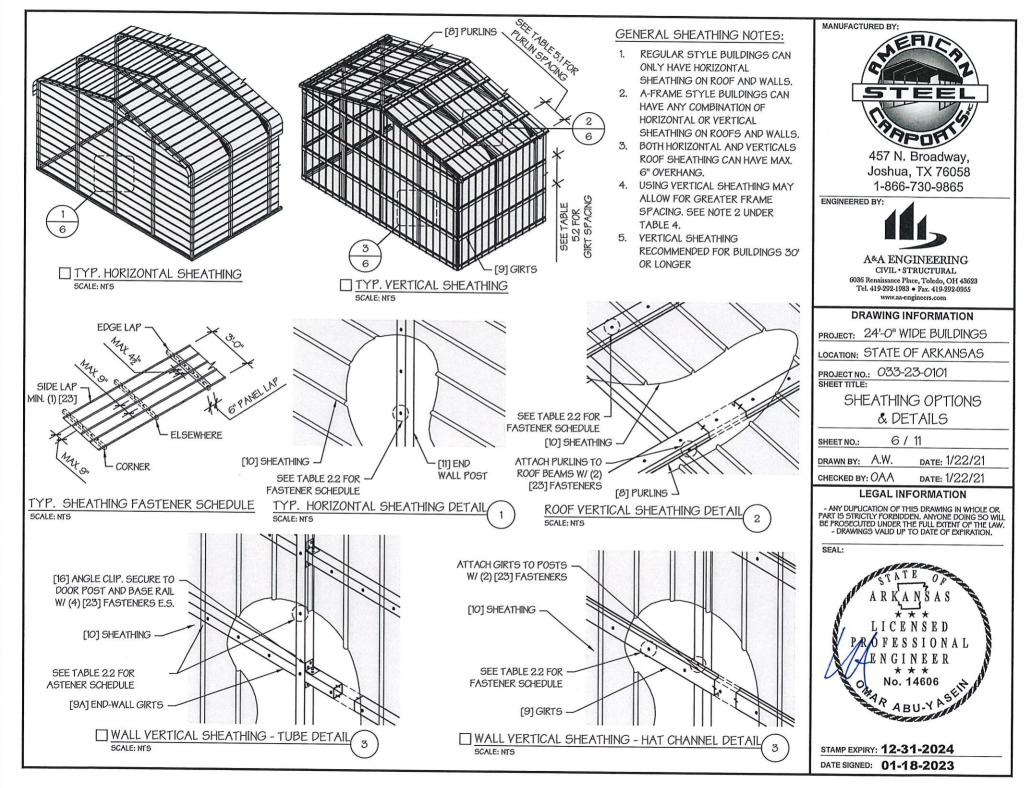
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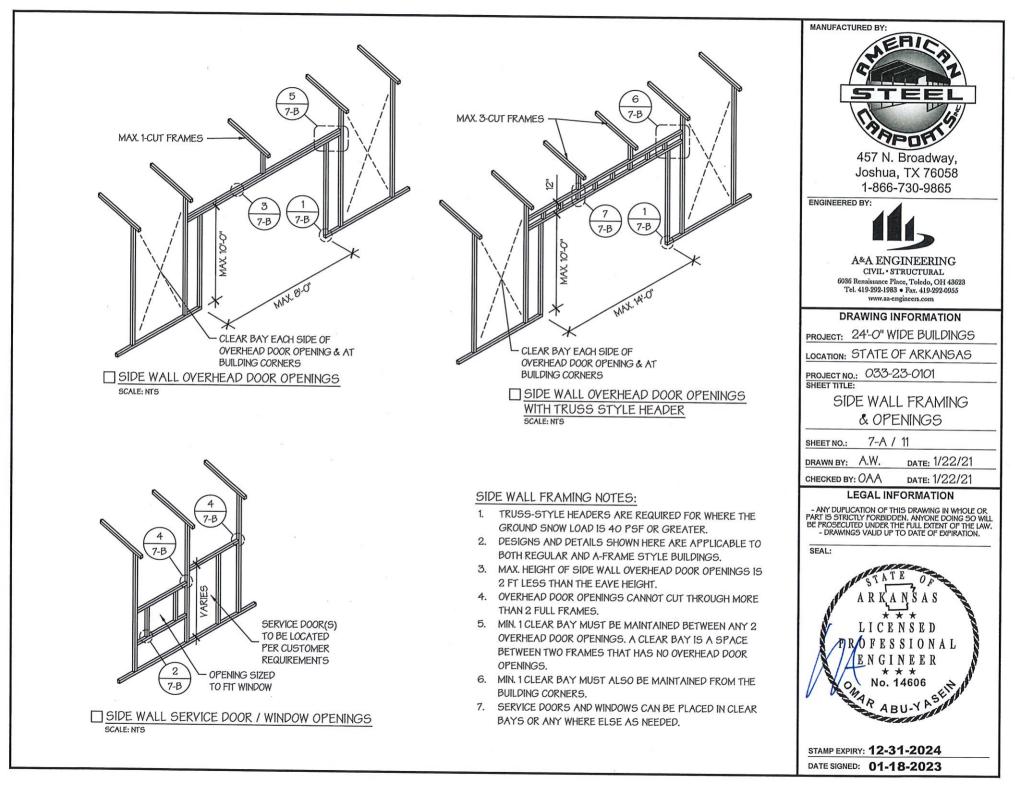
DATE: 1/22/21

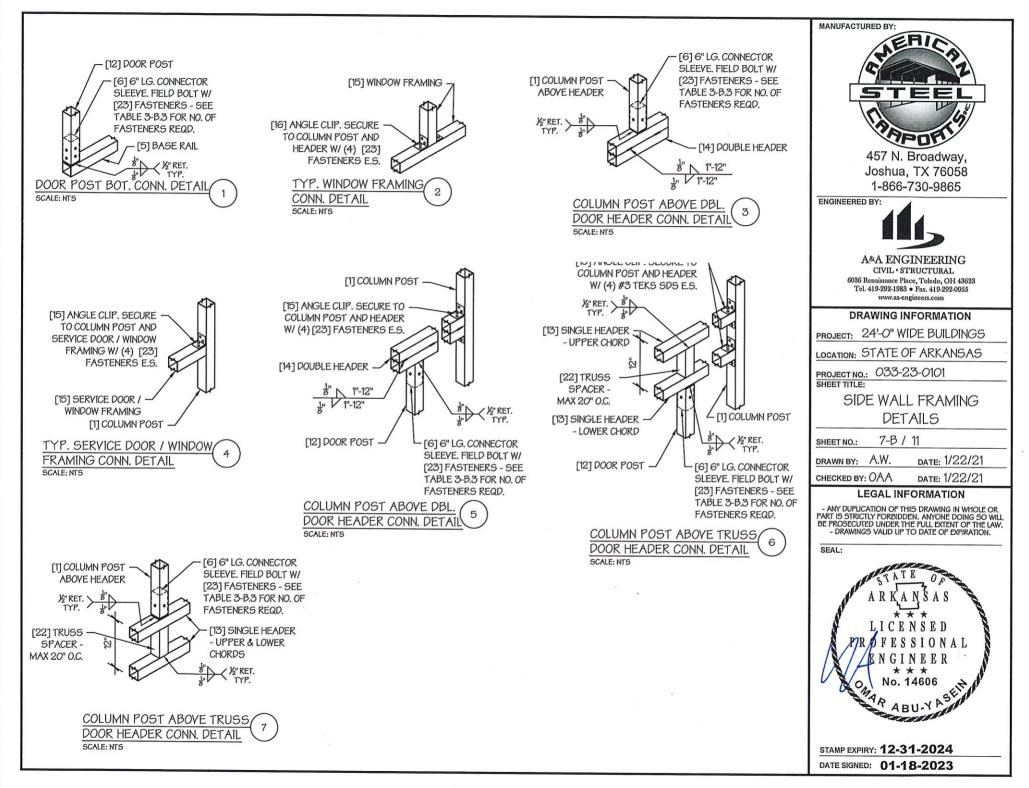
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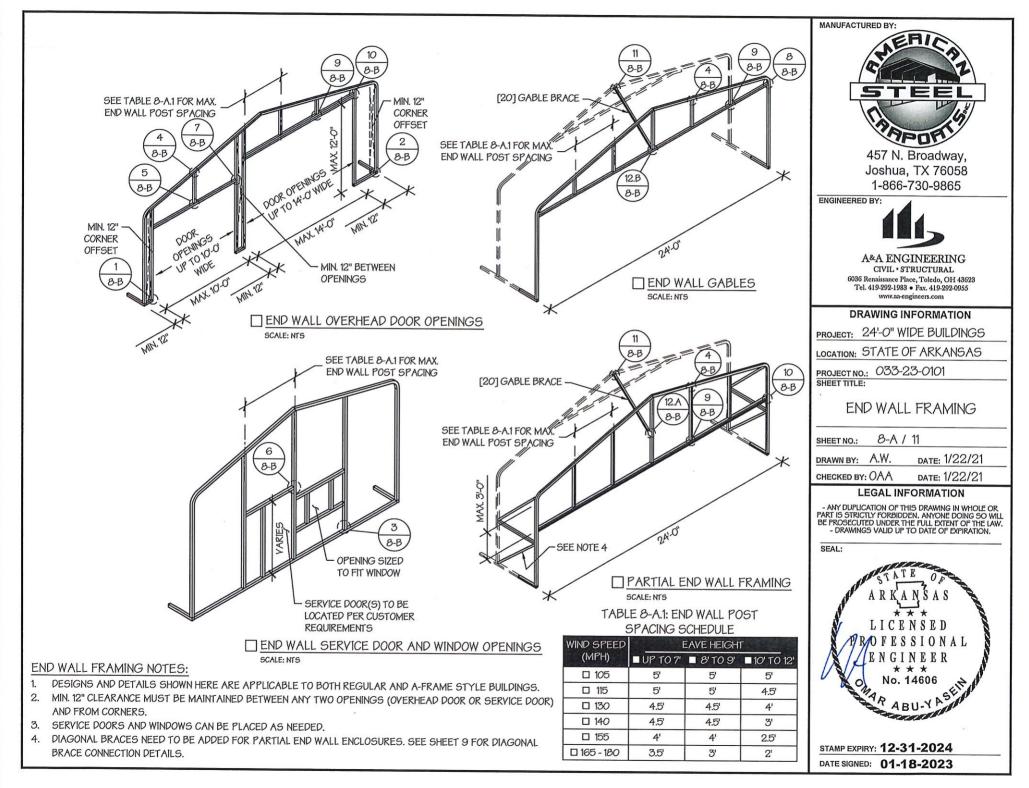
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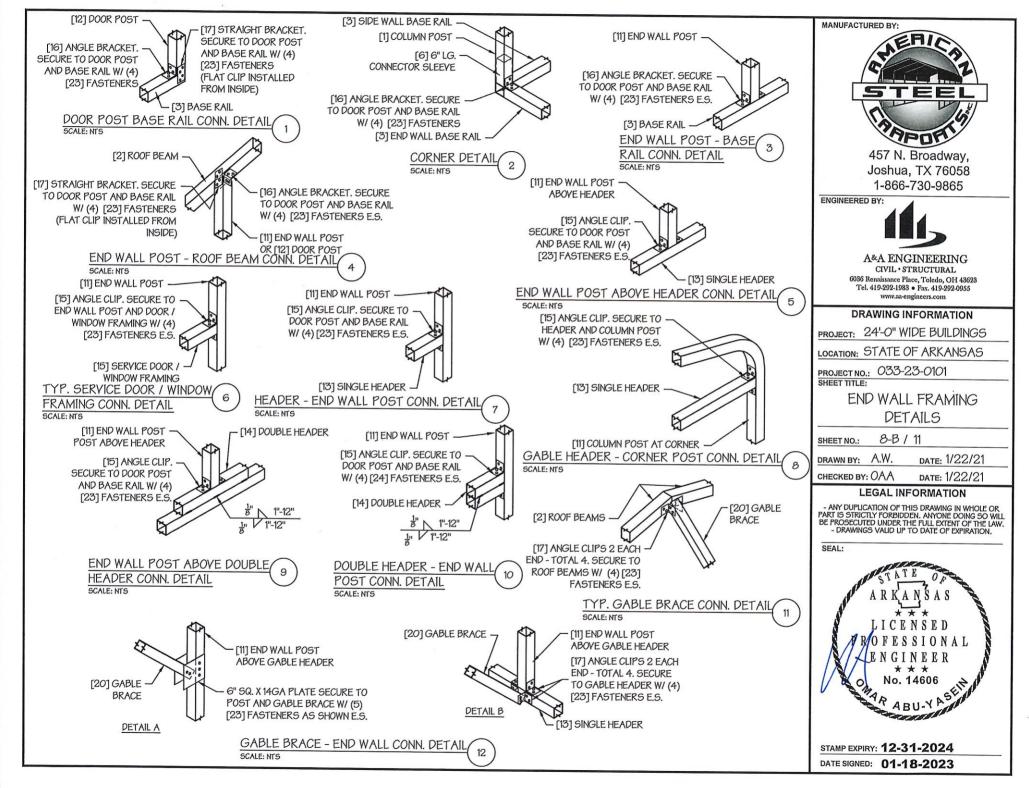
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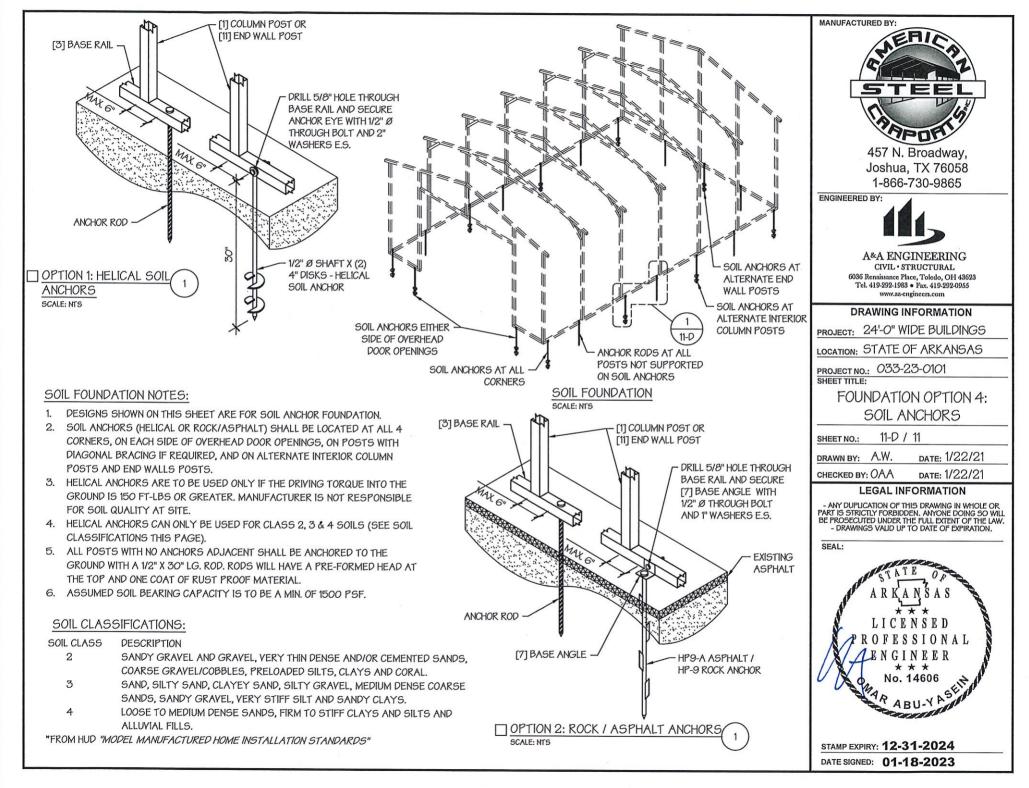






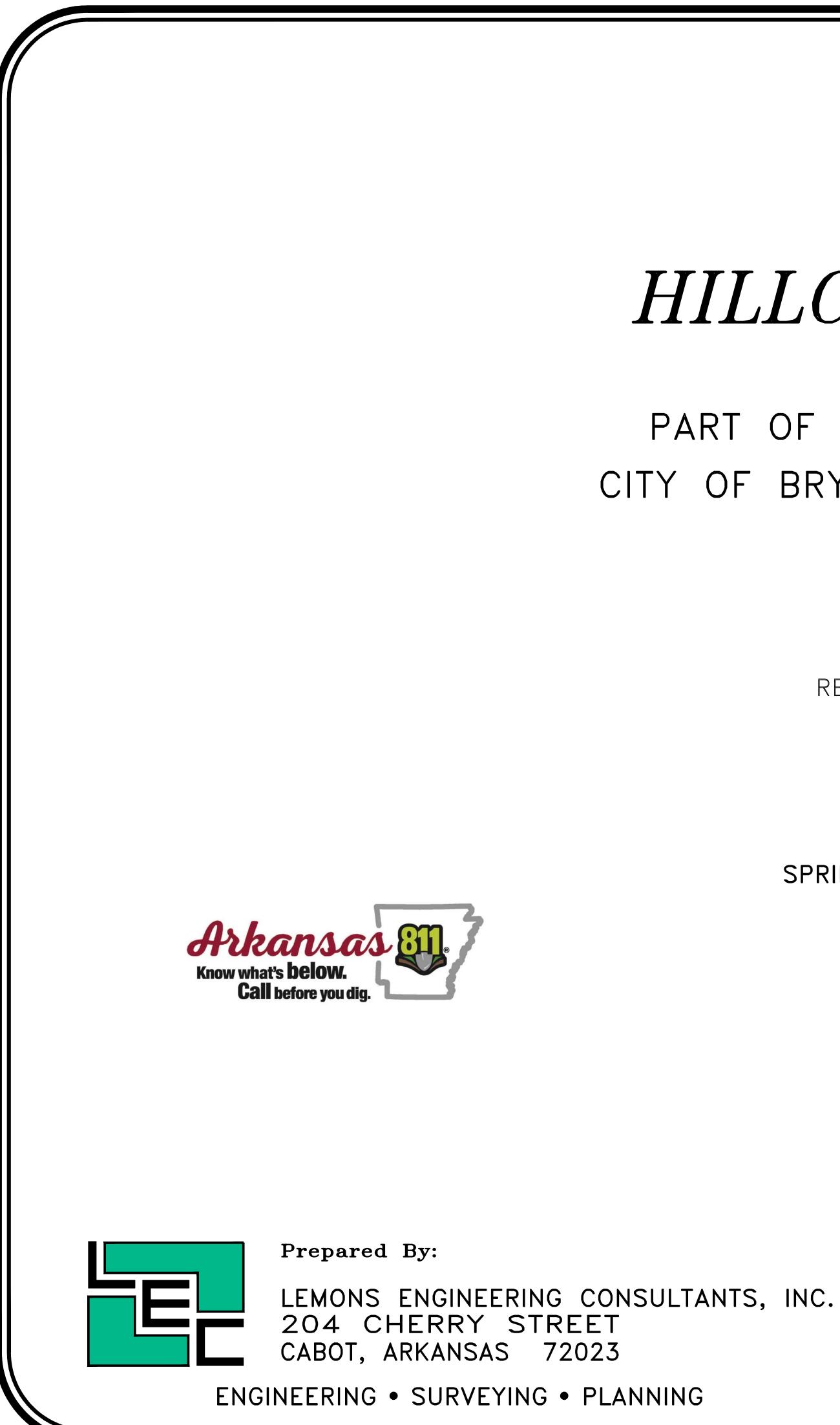












DETAILED PLANS:

HILLCREST ADDITION

PART OF SECTION 12, T-4-N, R-10-W CITY OF BRYANT, SALINE COUNTY, ARKANSAS

> JULY 9, 2024 REVISED: SEPTEMBER 20, 2024

> > PREPARED FOR:

SPRINGHILL HWY 5 DEVELOPMENT, LLC 816 E. OAK STREET CONWAY, ARKANSAS 72032

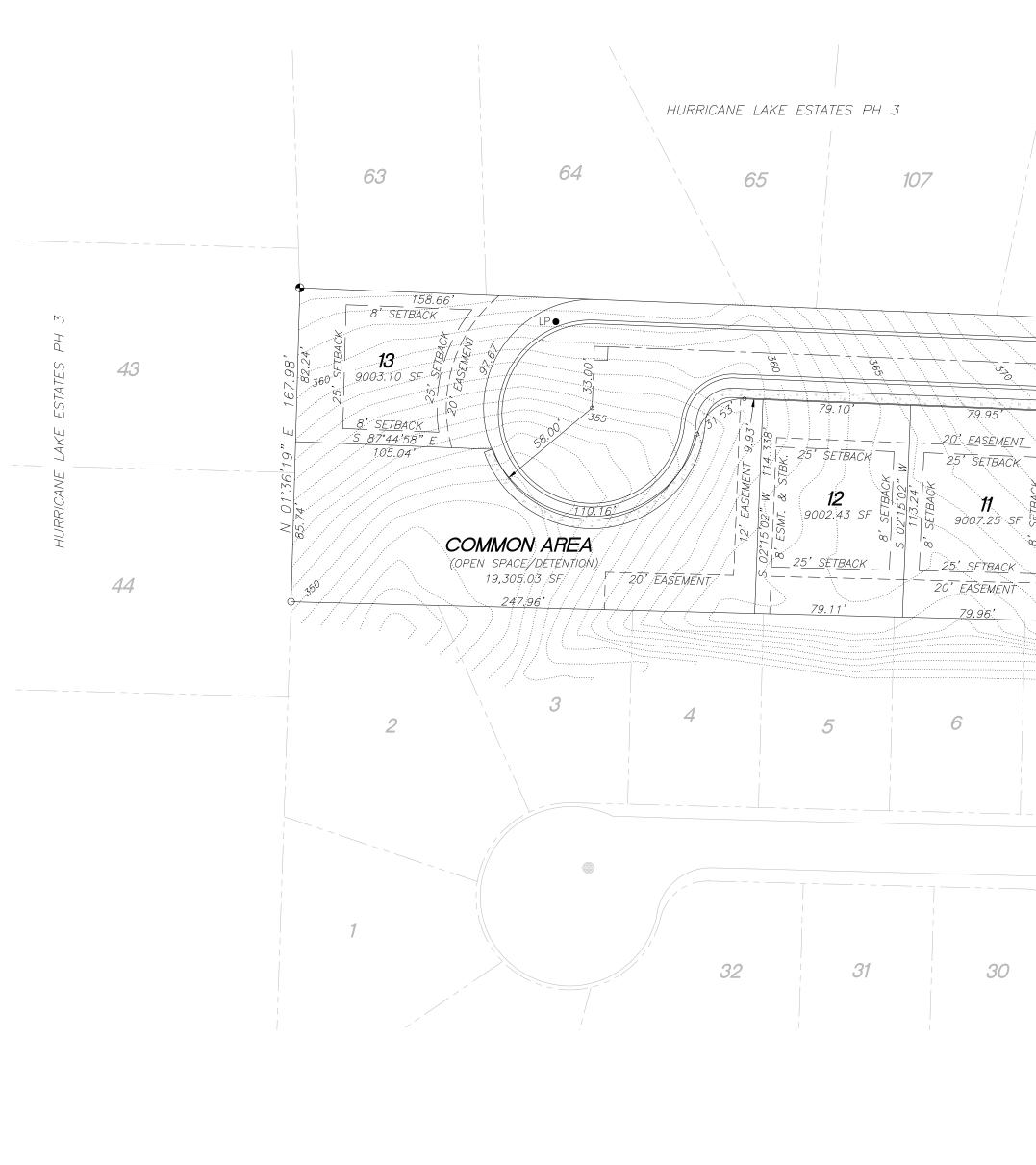


Culvert Plan/Profile Grading Plan Detention Pond Plan Water Layout Construction Details — Water Sewer Plan/Profile	
Street Plan/Profile Culvert Plan/Profile Grading Plan Detention Pond Plan Vater Layout Construction Details — Water Sewer Plan/Profile	1
Grading Plan Detention Pond Plan Water Layout Construction Details — Water Sewer Plan/Profile	2
Detention Pond Plan Water Layout Construction Details — Water Sewer Plan/Profile	3
Detention Pond Plan Water Layout Construction Details — Water Sewer Plan/Profile	1
Construction Details — Water Sewer Plan/Profile	5
Sewer Plan/Profile 8	5
Sewer Plan/Profile 8	7
Construction Datails - Sowar	3
	9
Trosion Control Plan 1	0

CERTIFICATE OF ENGINEERING ACCURACY: I, TIMOTHY B. LEMONS, HEREBY CERTIFY THAT THIS PLAT ARKANŚAS CORRECTLY REPRESENTS A PLAT MADE BY ME, OR UNDER MY * * * SUPERVISION, AND THAT ENGINEERING REQUIREMENTS OF THE REGISTERED BRYANT SUBDIVISION RULES AND REGULATIONS HAVE BEEN PROFESSIONAL COMPLIED WITH. ENGINEER * * * No. 7373 DATE TIMOTHY B. LEMONS, REGISTERED PROFESSIONAL ENGINEER NO. 7373, ARKANSAS CERTIFICATE OF SURVEYING ACCURACY: I, BILLY A. LAWRENCE, HEREBY CERTIFY THAT THIS PLAT CORRECTLY REPRESENTS A BOUNDARY SURVEY MADE BY ME AND ALL MONUMENTS SHOWN HEREON ACTUALLY EXIST AND THEIR LOCATION, SIZE, TYPE AND MATERIAL ARE CORRECTLY SHOWN. DATE BILLY A. LAWRENCE, ARKANSAS

PROFESSIONAL SURVEYOR,

NO. 1552





CERTIFICATE OF PRELIMINARY PLAT APPROVAL:

ALL REQUIREMENTS OF THE BRYANT SUBDIVISION RULES AND REGULATIONS RELATIVE TO THE PREPARATION AND SUBMITTAL OF A PRELIMINARY PLAT HAVING BEEN FULFILLED, APPROVAL OF THIS DOCUMENT IS HEREBY GRANTED, SUBJECT TO FURTHER PROVISIONS OF SAID RULES AND REGULATIONS, THIS PROVISIONS OF SAID NOLLS CERTIFICATE SHALL EXPIRE ______ DATE

DATE OF EXECUTION

SIGNED: CHAIRMAN, BRYANT PLANNING COMMISSION

<u>CERTIFICATE OF OWNERS:</u>

WE, THE UNDERSIGNED, OWNERS OF THE REAL ESTATE SHOWN AND DESCRIBED HEREIN, DO HEREBY CERTIFY THAT WE HAVE LAID OFF, PLATTED, AND SUBDIVIDED, AND DO HEREBY LAY OFF, PLAT AND SUBDIVIDE SAID REAL ESTATE IN ACCORDANCE WITH THIS PLAT.

DATE

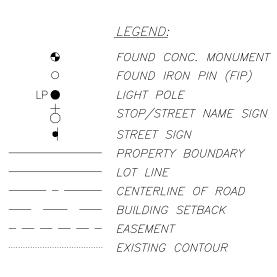
SPRINGHILL HWY 5 DEVELOPMENT, LLC 816 E. OAK STREET CONWAY, ARKANSAS 72032

PART OF THE SE $\frac{1}{4}$, OF THE SE $\frac{1}{4}$, SECTION 17, T-1-S, R-14-W, SALINE COUNTY, ARKANSAS, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCING AT THE SOUTHEAST CORNER OF THE SE 1/4, OF THE SE $\frac{1}{4}$, Section 17, T-1-S, R-14-W, SALINE COUNTY, ARKANSAS; THENCE N 02°17'33" E, 1167.19 FEET TO THE POINT OF BEGINNING; THENCE N 88°34'40" W, 1344.97 FEET; THENCE N 01°36'19"E, 167.98 FEET; THENCE S 87°44'58"E, 1346.85 FEET; THENCE S 02°18'02" W, 148.52 FEET; TO THE POINT OF BÉGINNING, CONTAINING 4.89 ACRES, MORE OR LESS. SOURCE OF TITLE: 2021-030121

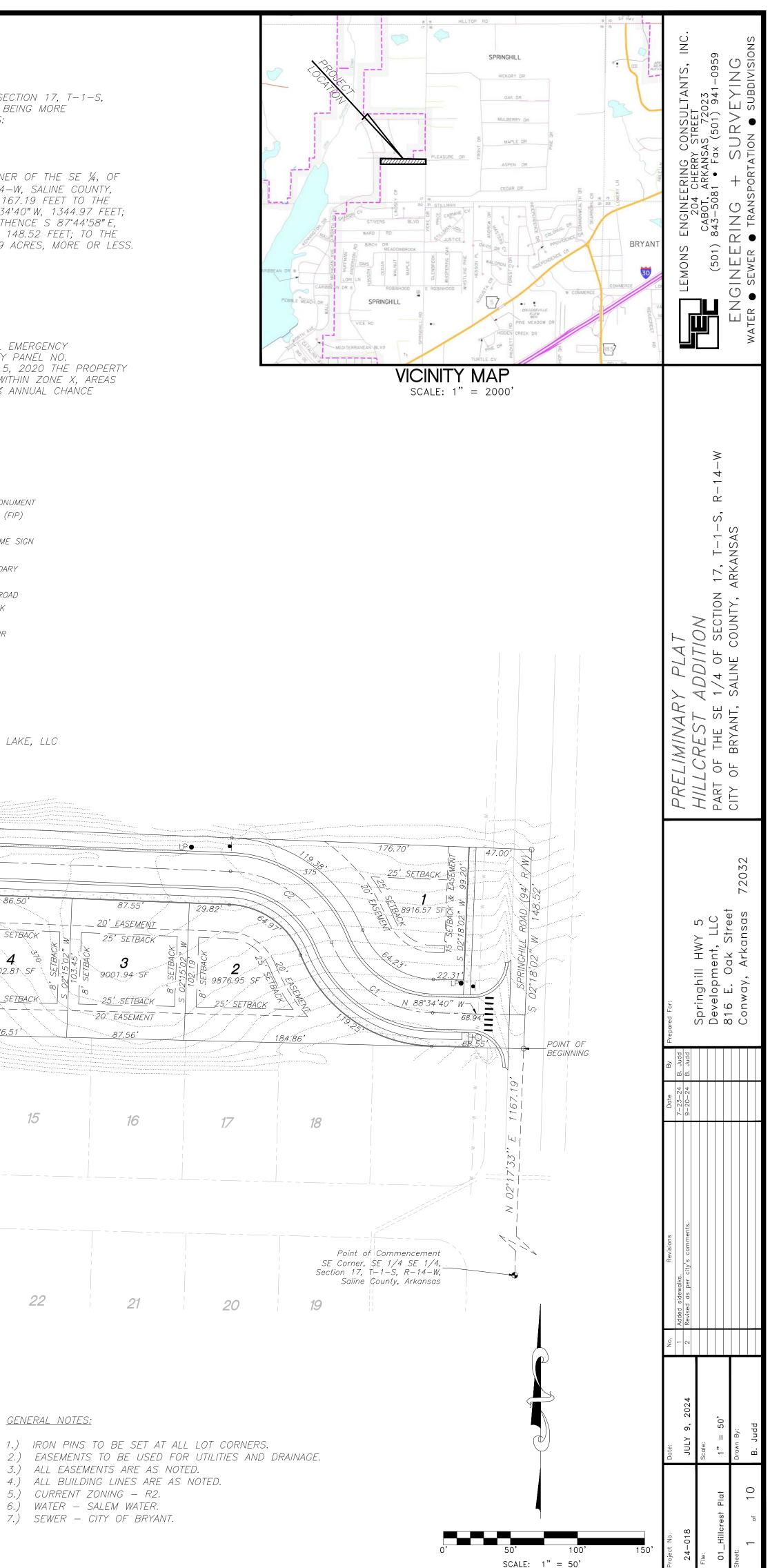
FLOOD CERTIFICATION:

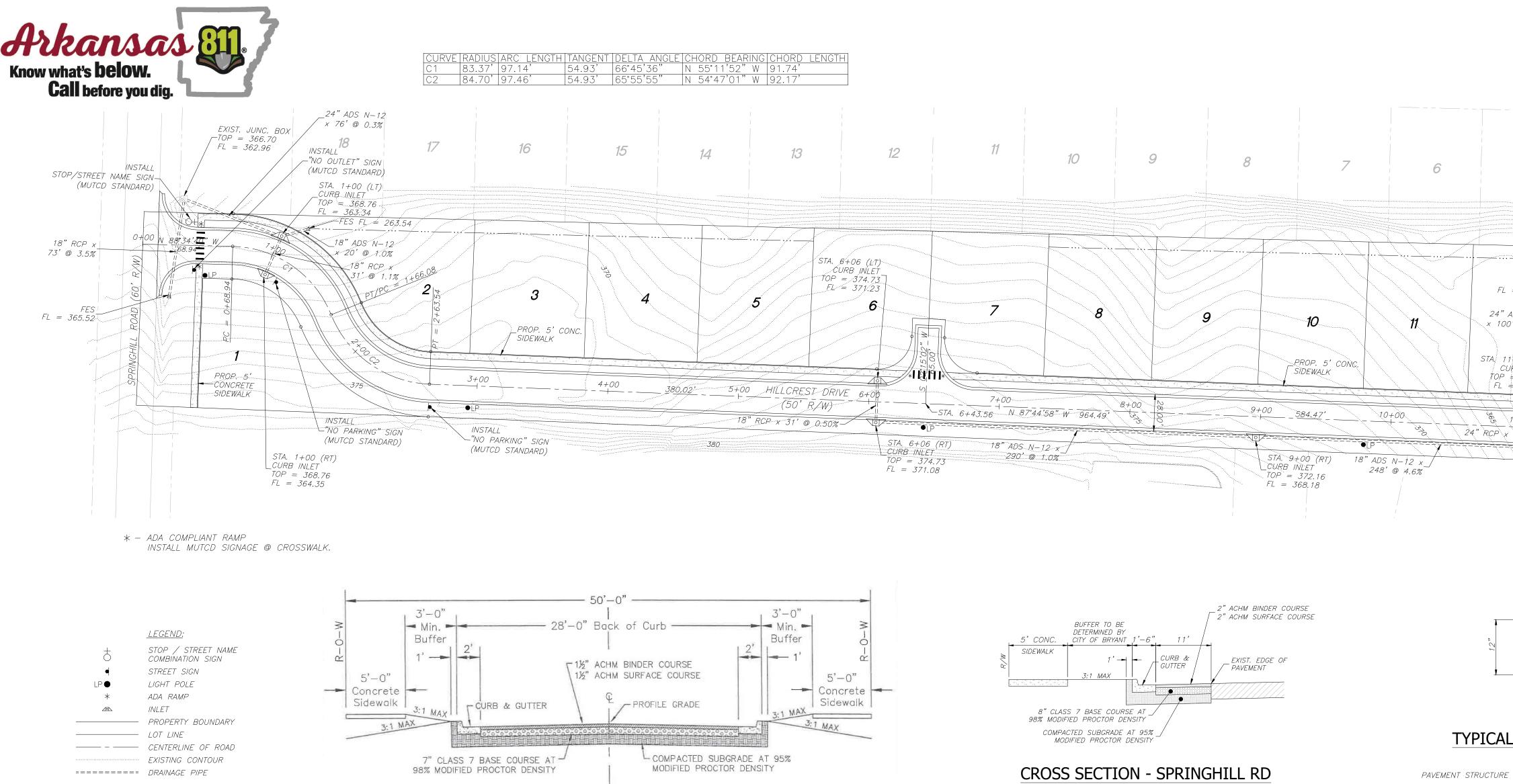
BASED UPON REVIEW OF THE FEDERAL EMERGENCY MANAGEMENT AGENCY, FIRM COMMUNITY PANEL NO. 05125C0225E, EFFECTIVE DATE: JUNE 5, 2020 THE PROPERTY DEPICTED ON THIS PLAT IS LOCATED WITHIN ZONE X, AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN.



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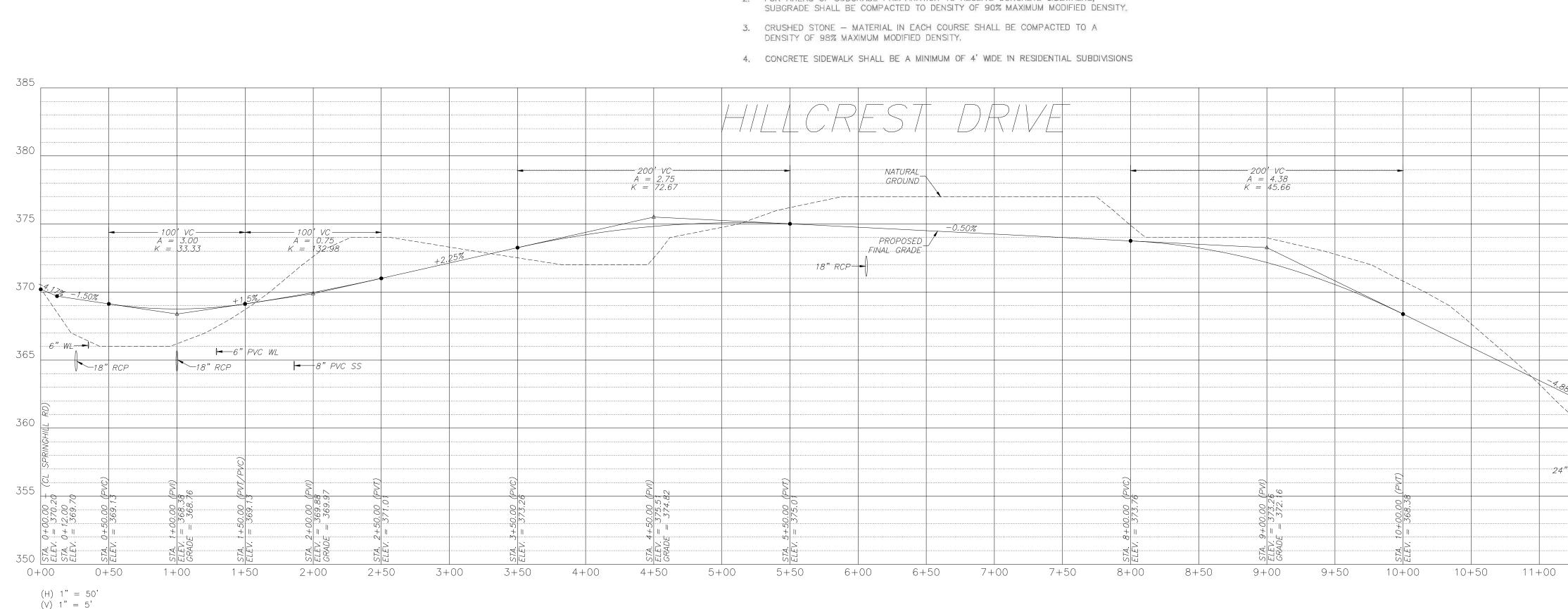
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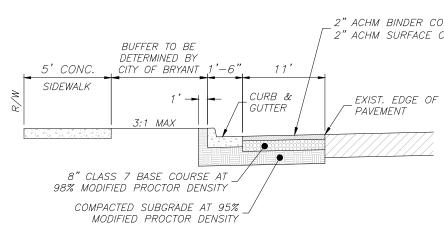


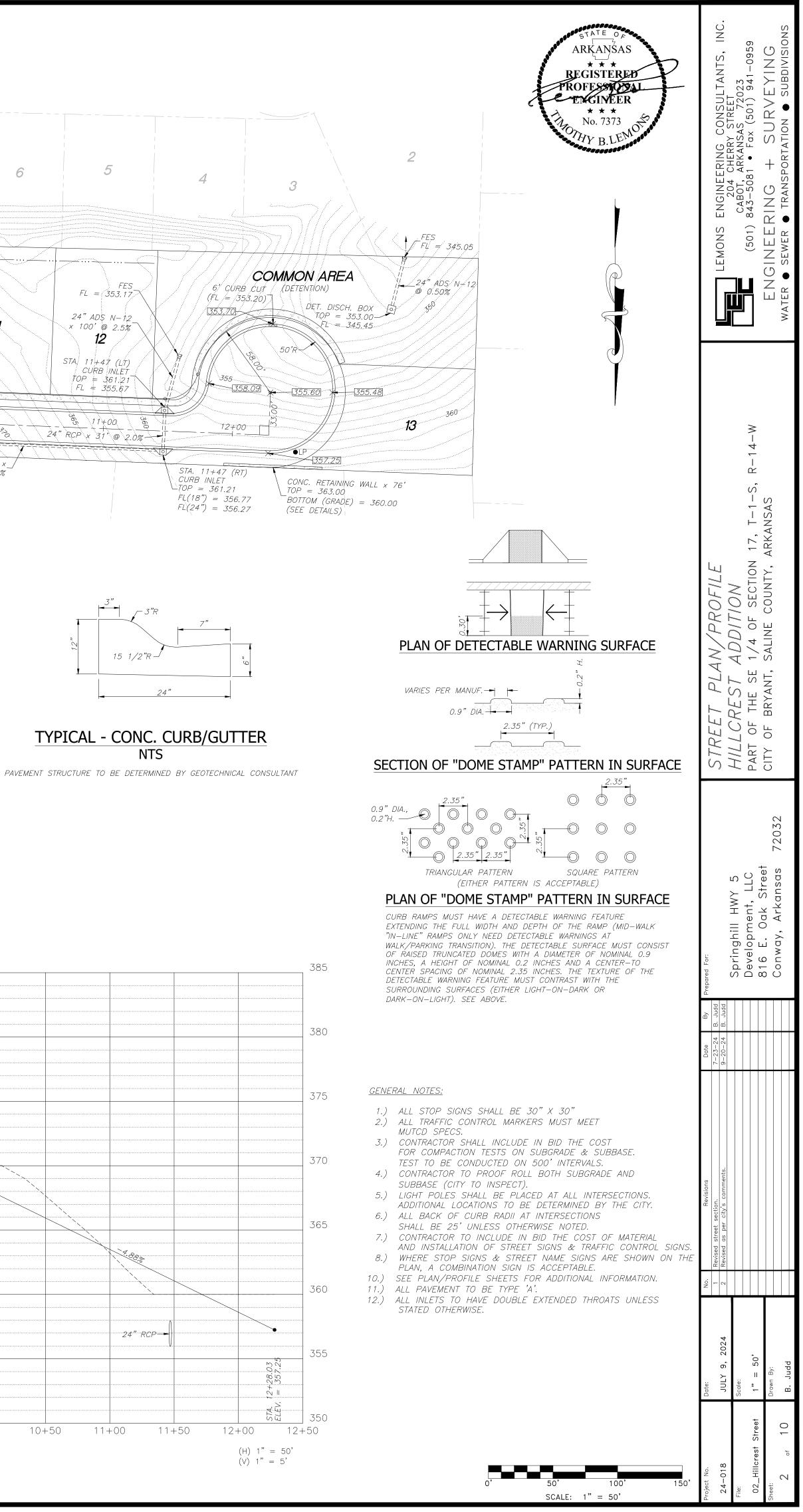
PROPOSED SIDEWALK



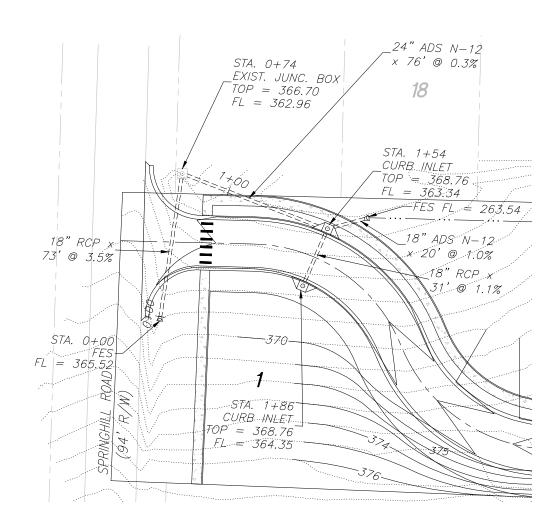
- 2. FOR AREAS OF SUBGRADE PREPARATION TO RECEIVE CONCRETE SIDEWALKS,
- 1. IN AREAS TO RECEIVE BITUMINOUS PAVING, CONCRETE DRIVEWAYS OR CURB AND GUTTER, SUBGRADE SHALL BE COMPACTED TO A DENSITY NOT LESS THAN 95% OF MAXIMUM MODIFIED DENSITY OBTAINED AT OPTIMUM MOISTURE CONTENT.
- GENERAL NOTES

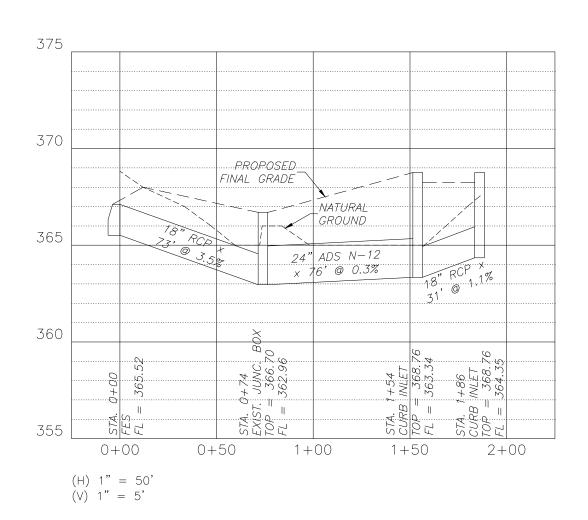


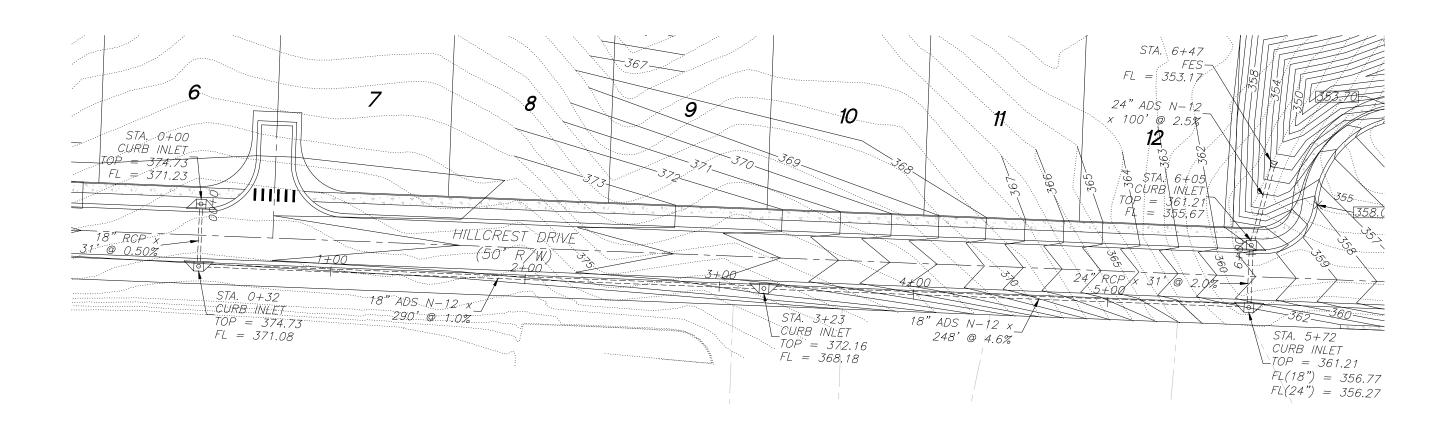


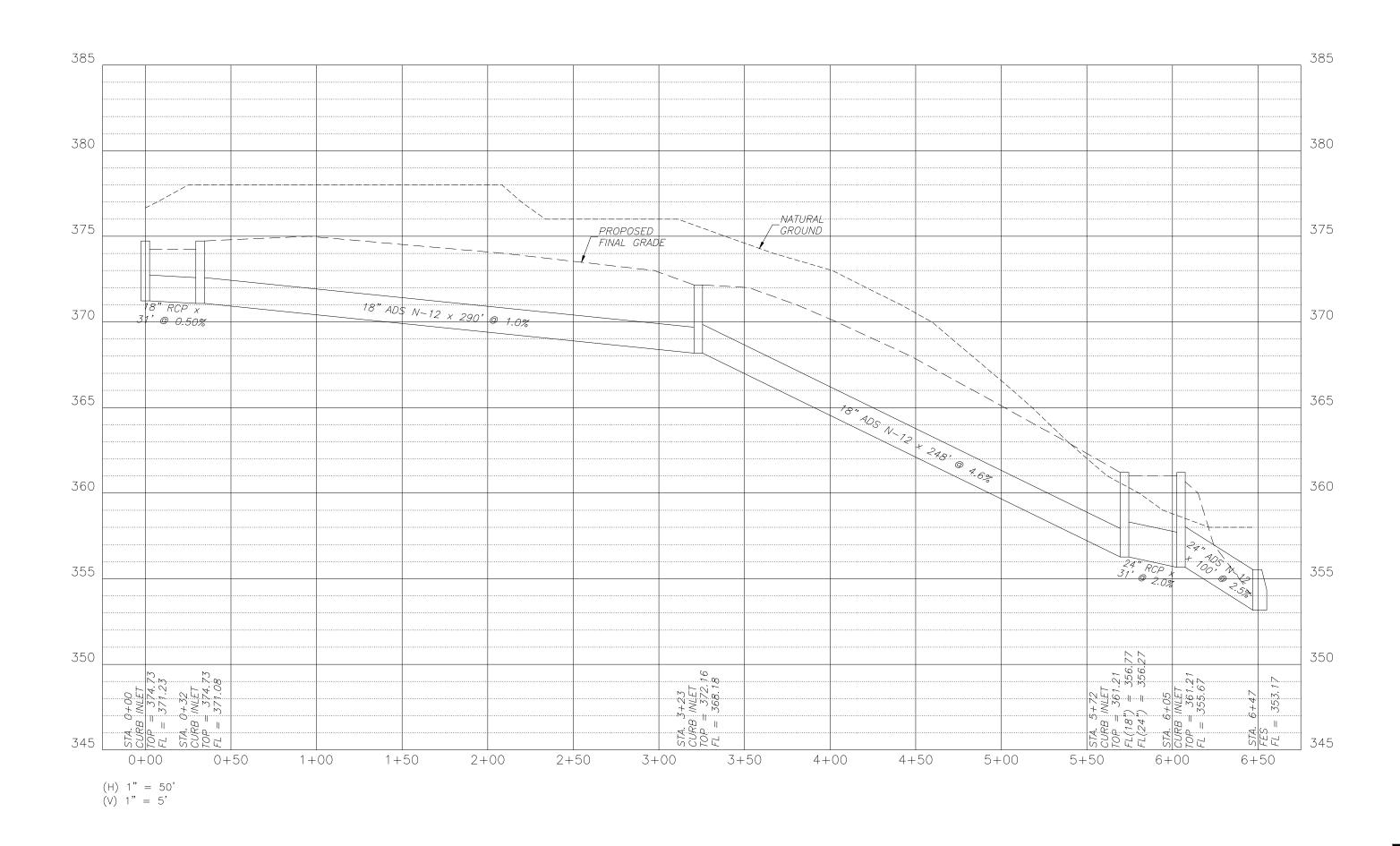








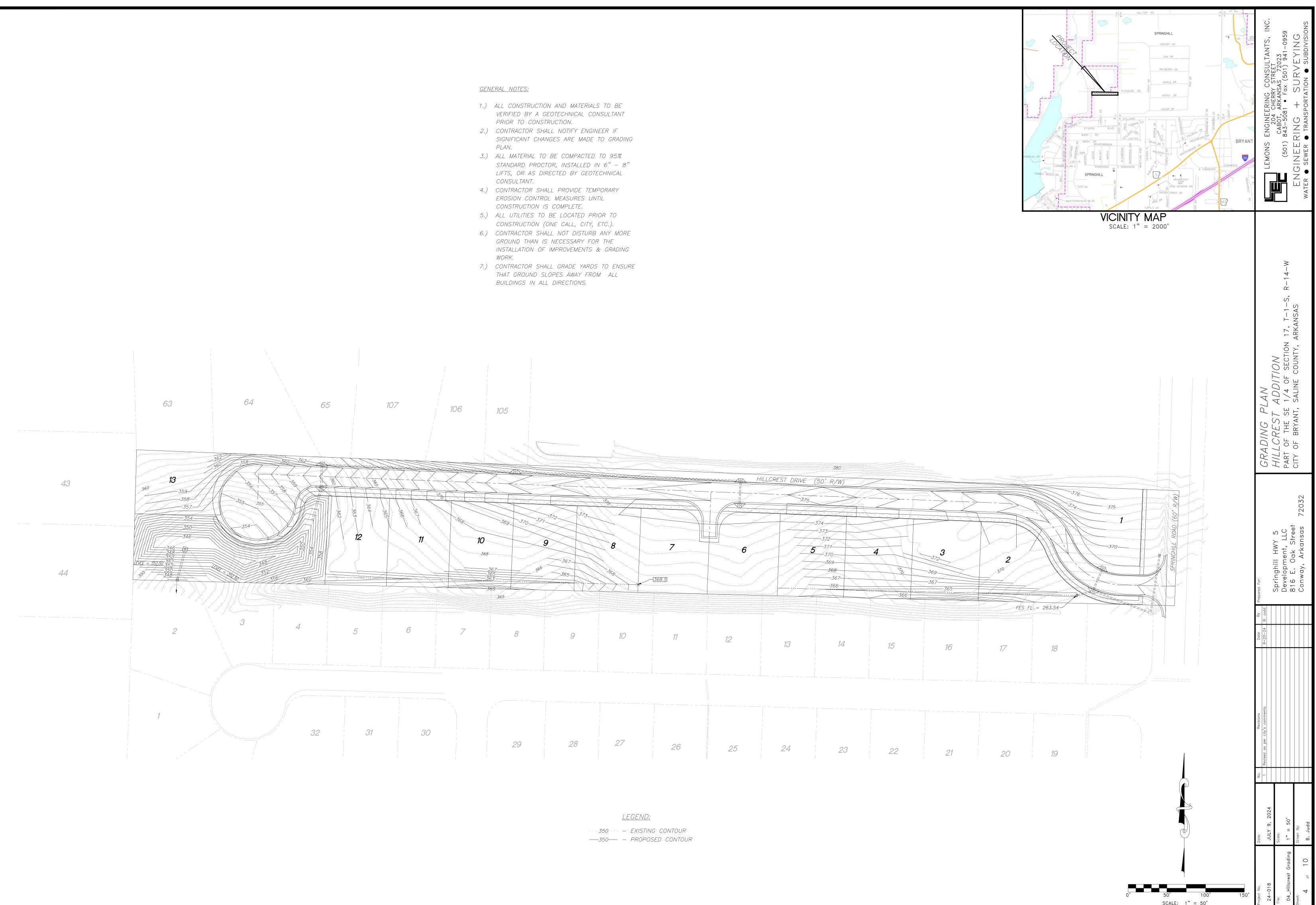






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	CULVERT PLAN/PROFILE HILLCREST ADDITION PART OF THE SE 1/4 OF SECTION 17, T-1-S, R-14-W CITY OF BRYANT, SALINE COUNTY, ARKANSAS											
	Prepared For:					Development IIC		816 F. Onk Street		Conway, Arkansas 72032		
	Date By											
	Revisions											
	No.											
	Date:		SEPTEMBER 20, 2024		Scale:		1" = 50'		Drawn By:	~	R	
150'	Project No.		24-018		File:		03 Hillcrest Culvert	1	Sheet:	1	5 _{of} 10	

SCALE: 1" = 50'



Arkansas 81.	7
Know what's below. Call before you dig.	

<u>GENERAL_NOTES:</u>

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DET. DISCHARGE BOX

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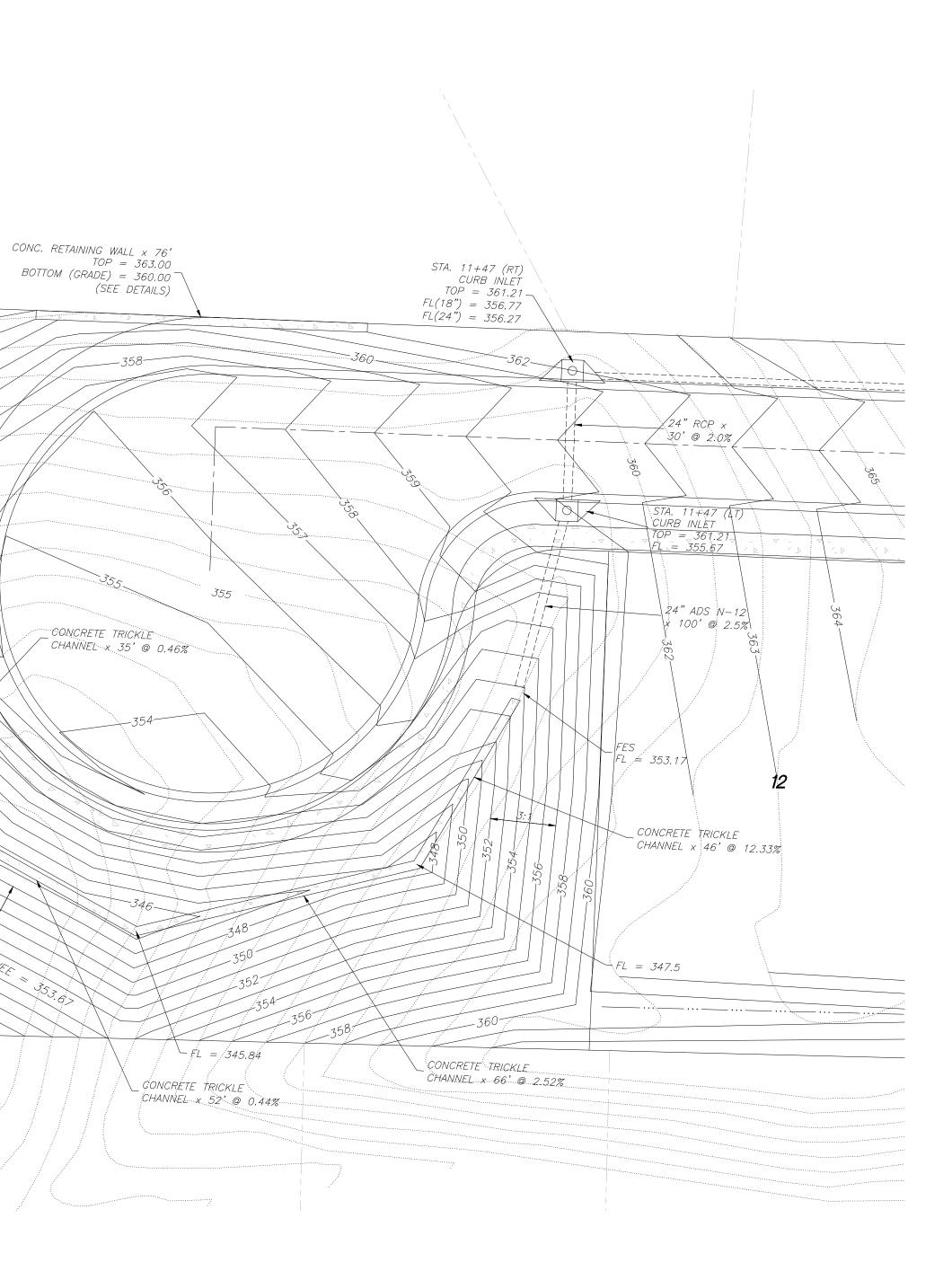
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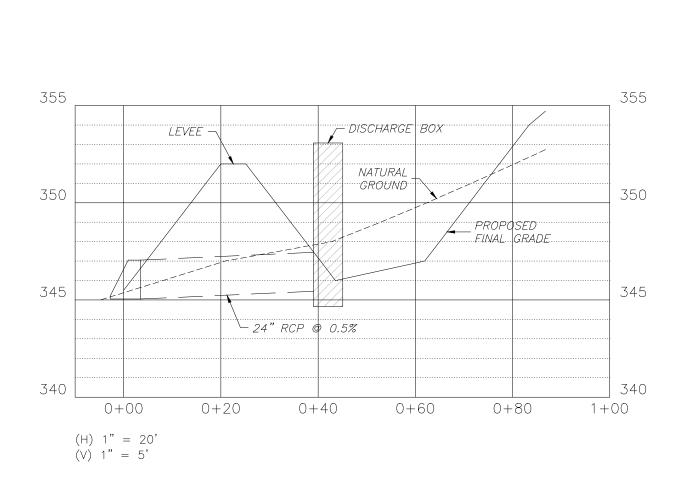
- 1.) ALL CONSTRUCTION AND MATERIALS TO BE
- PRIOR TO CONSTRUCTION.
- VERIFIED BY A GEOTECHNICAL CONSULTANT

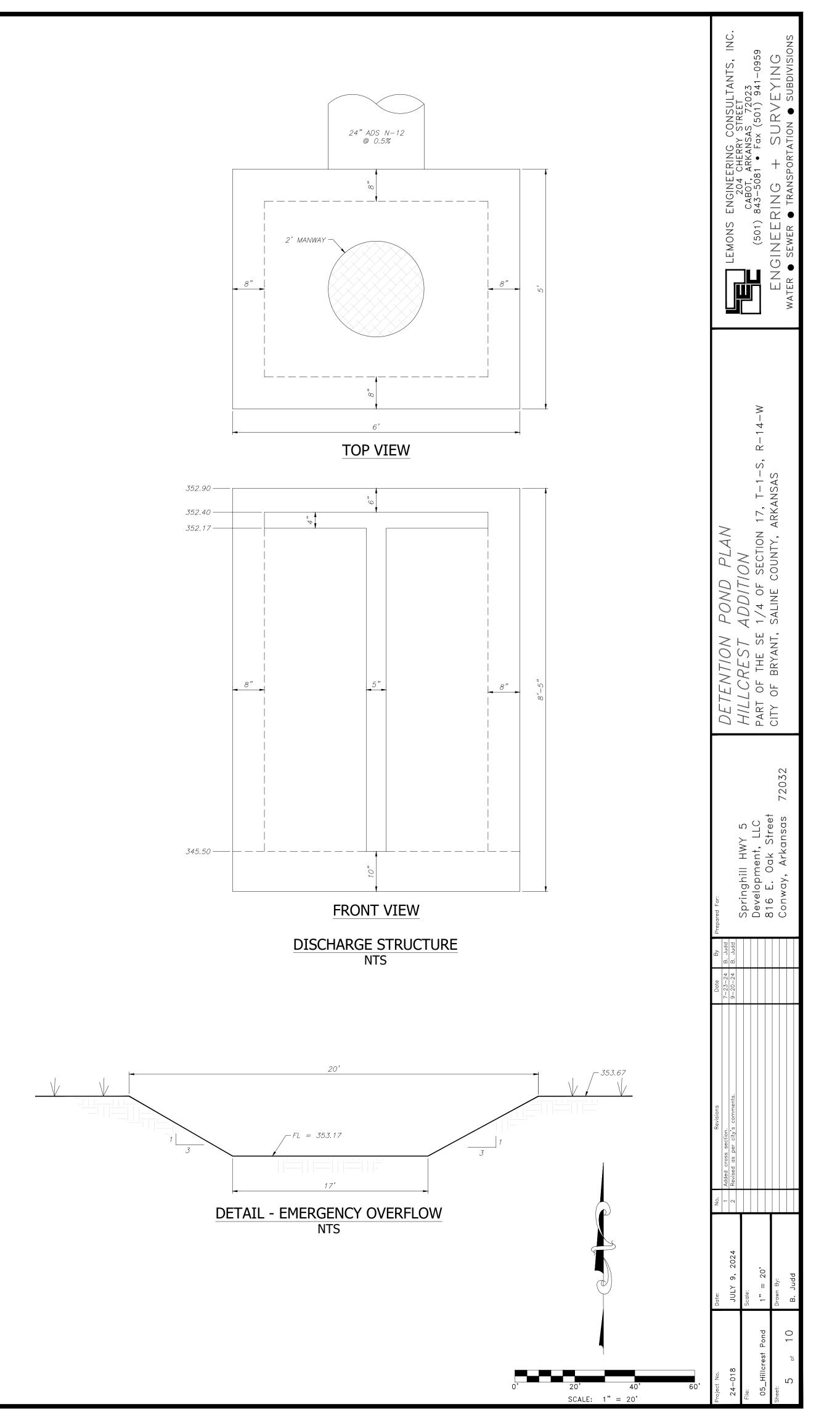
- 2.) CONTRACTOR SHALL NOTIFY ENGINEER IF

- SIGNIFICANT CHANGES ARE MADE TO GRADING PLAN. 3.) ALL MATERIAL TO BE COMPACTED TO 95%
- STANDARD PROCTOR, INSTALLED IN 6" 8"
- LIFTS, OR AS DIRECTED BY GEOTECHNICAL
- CONSULTANT. 4.) CONTRACTOR SHALL PROVIDE TEMPORARY
- EROSION CONTROL MEASURES UNTIL CONSTRUCTION IS COMPLETE.
- 5.) ALL UTILITIES TO BE LOCATED PRIOR TO CONSTRUCTION (ONE CALL, CITY, ETC.).
- 6.) CONTRACTOR SHALL NOT DISTURB ANY MORE GROUND THAN IS NECESSARY FOR THE INSTALLATION OF IMPROVEMENTS & GRADING WORK.
- 7.) ALL LEVEES ASSOCIATED WITH DETENTION FACILITY SHALL NOT HAVE A SLOPE GREATER THAN 3:1.
- 8.) ALL AREAS OF DETENTION FACILITY SHALL INCLUDE SOLID SOD STABILIZATION.

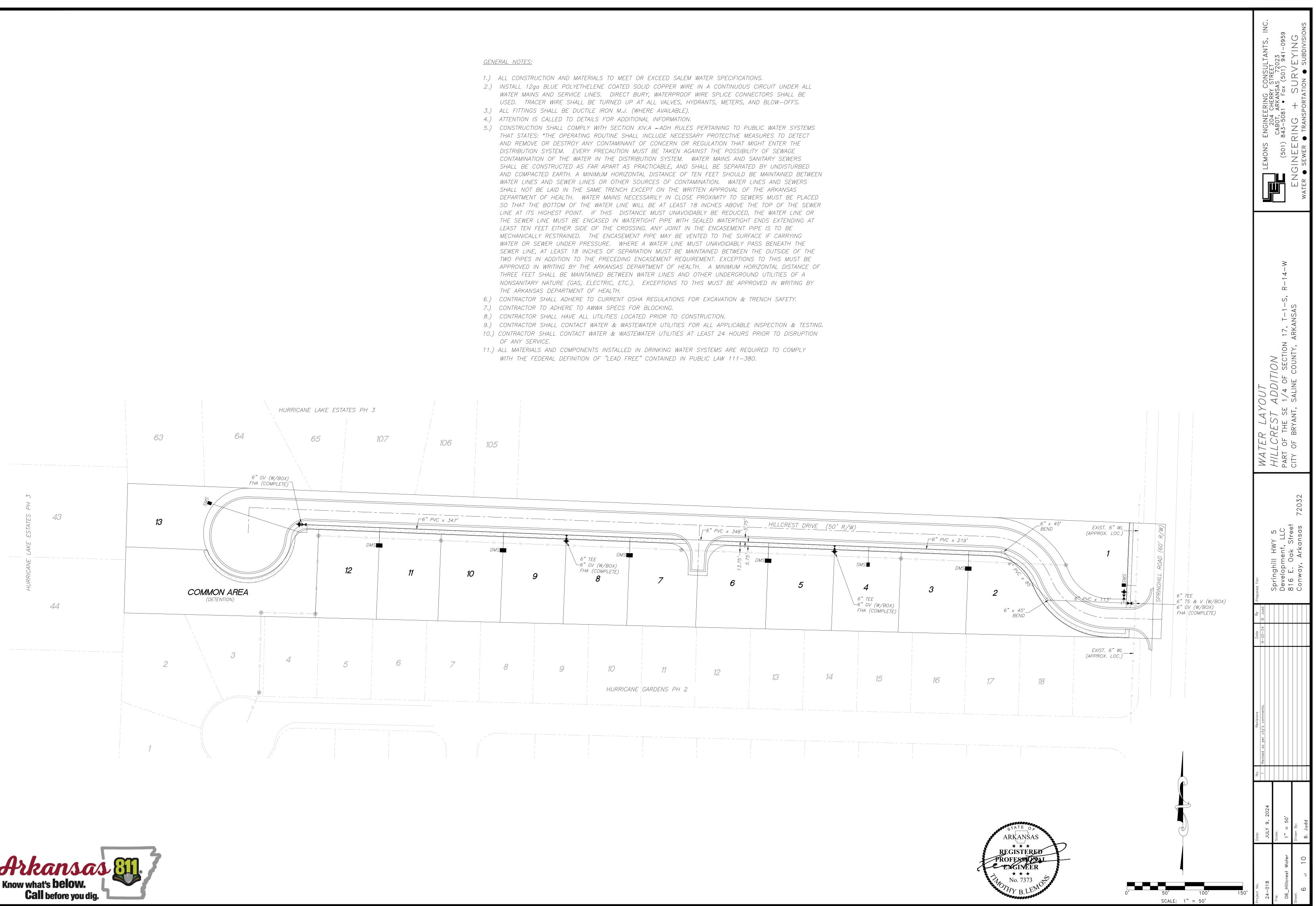
<u>LEGEND:</u>

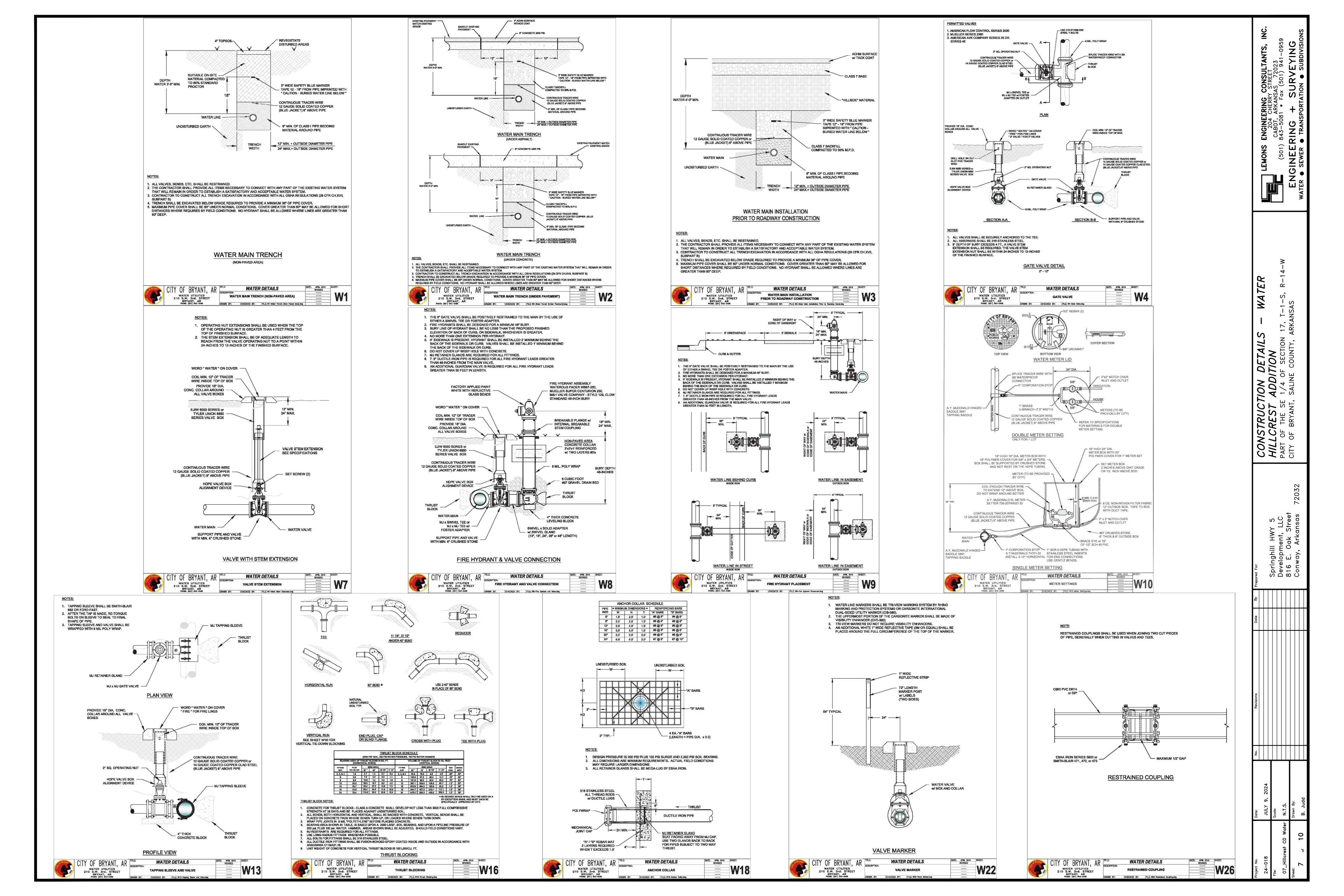


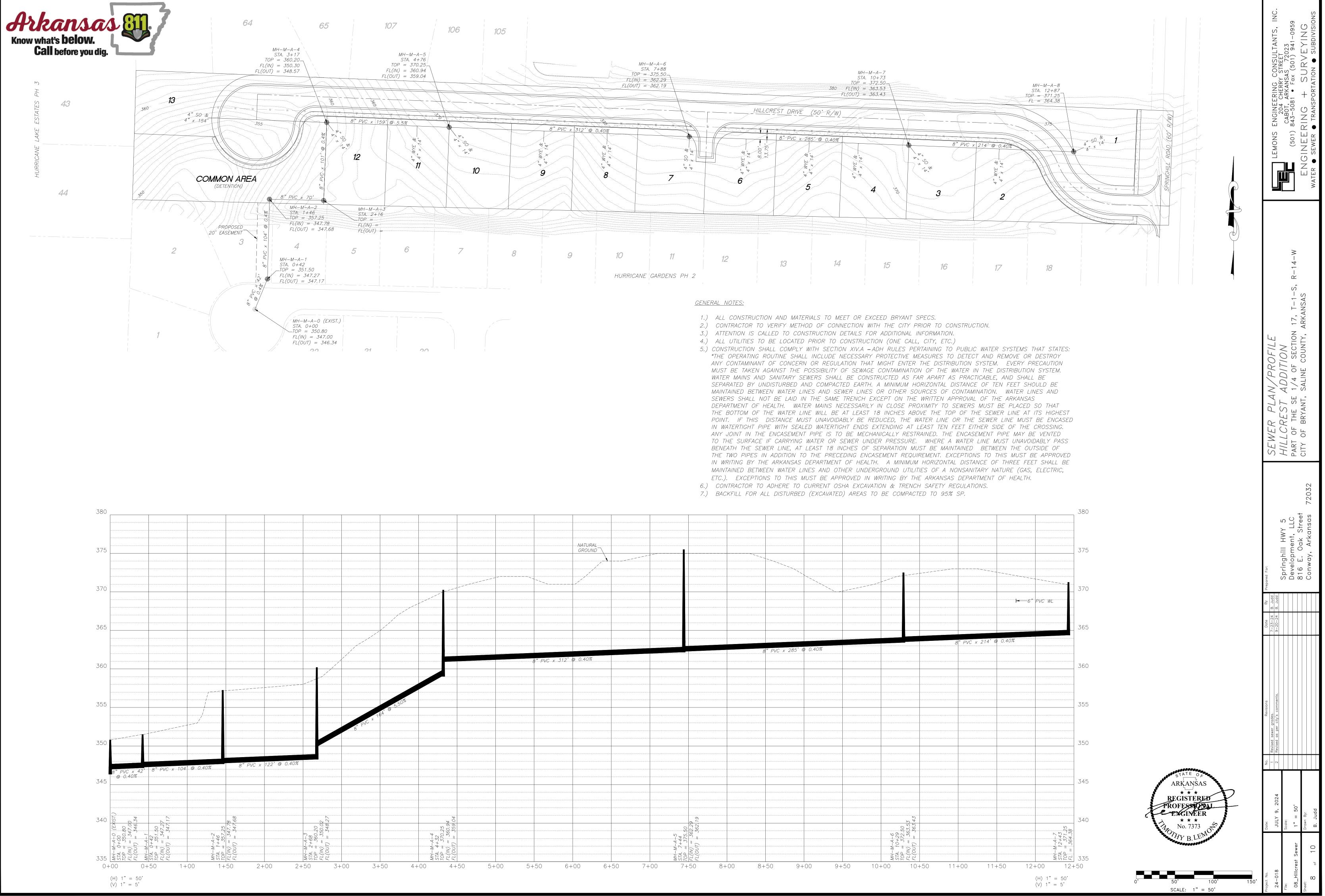


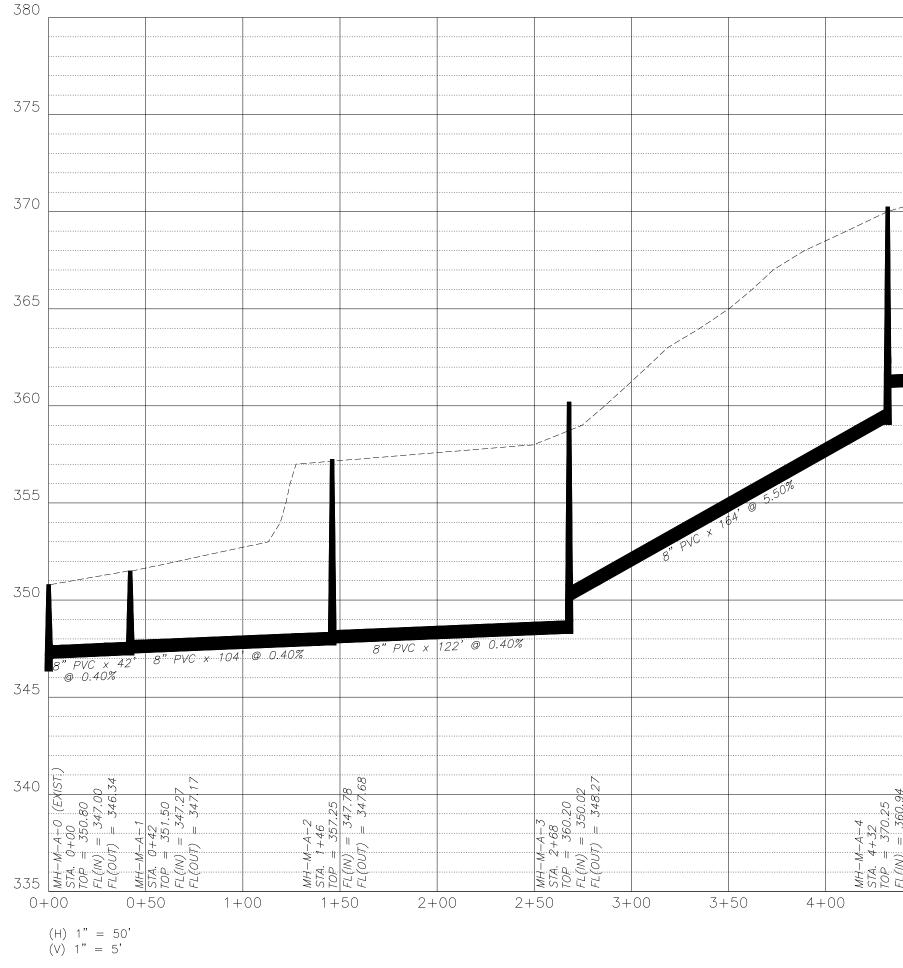




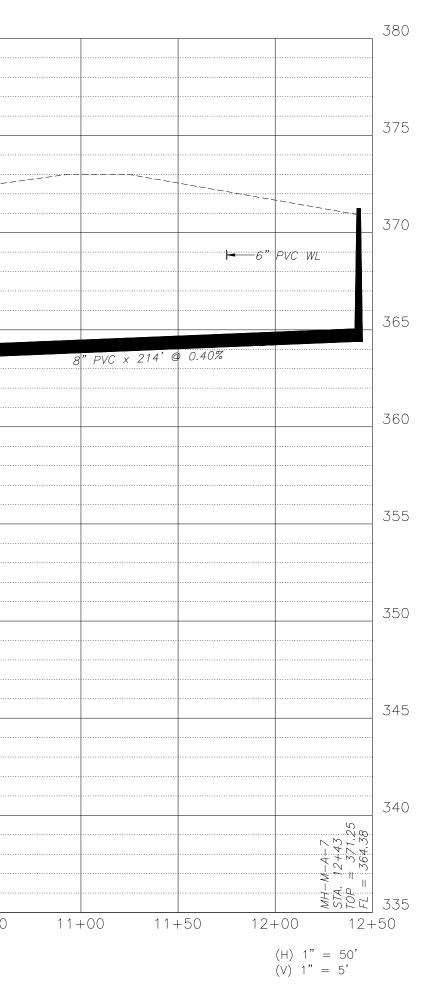


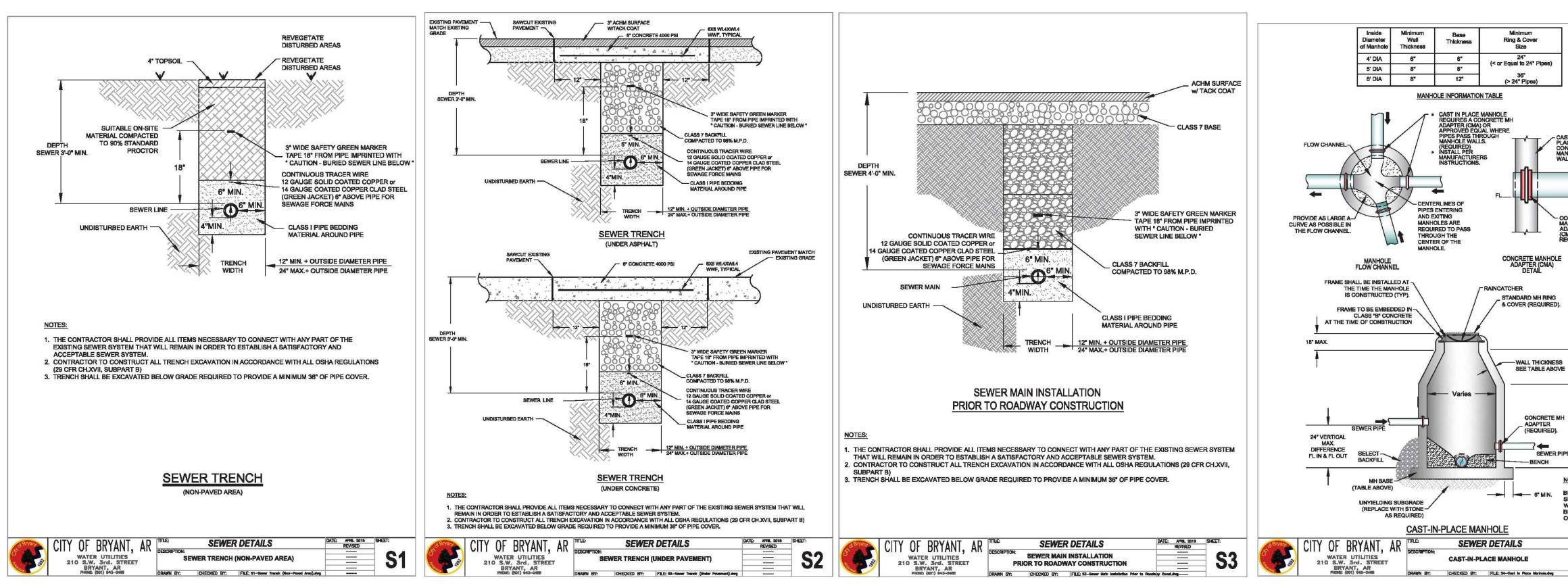


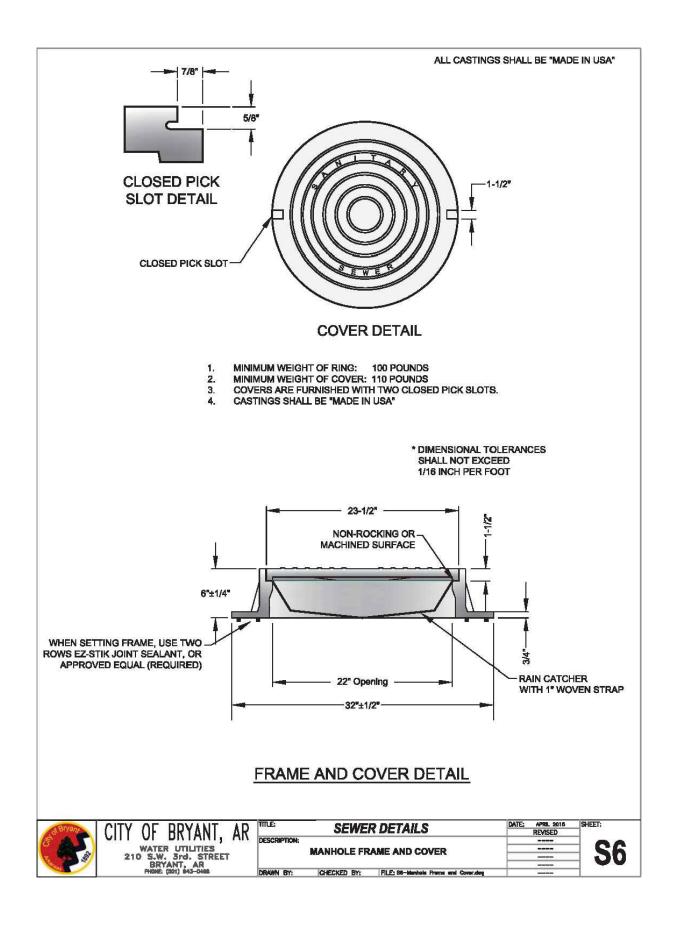


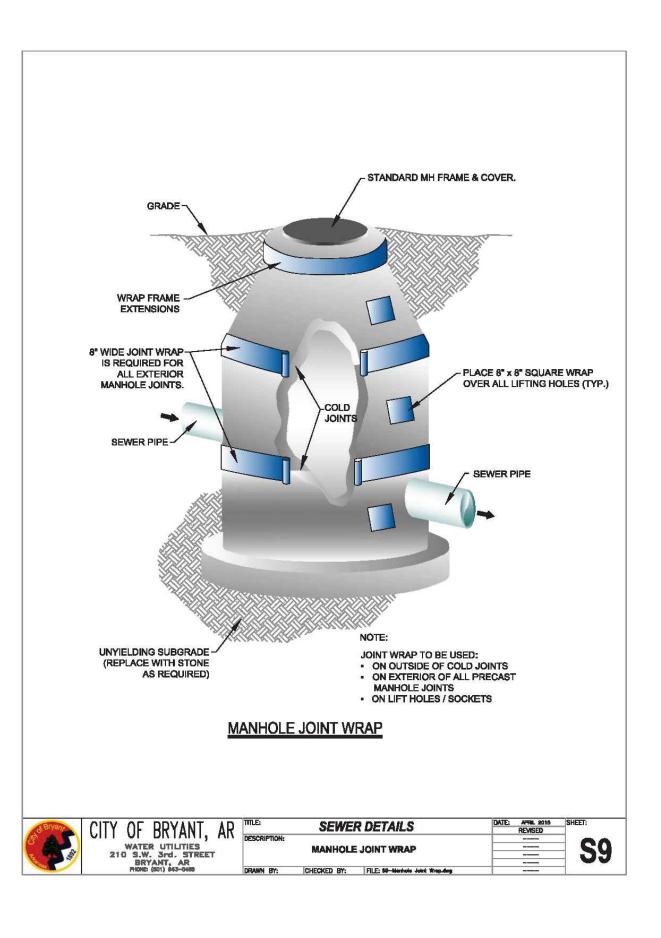


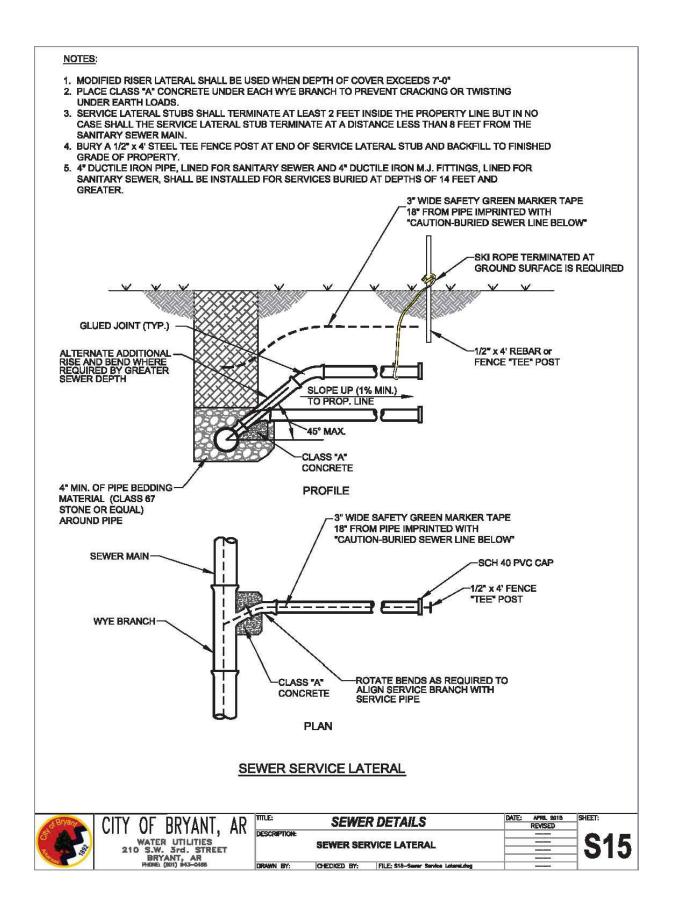
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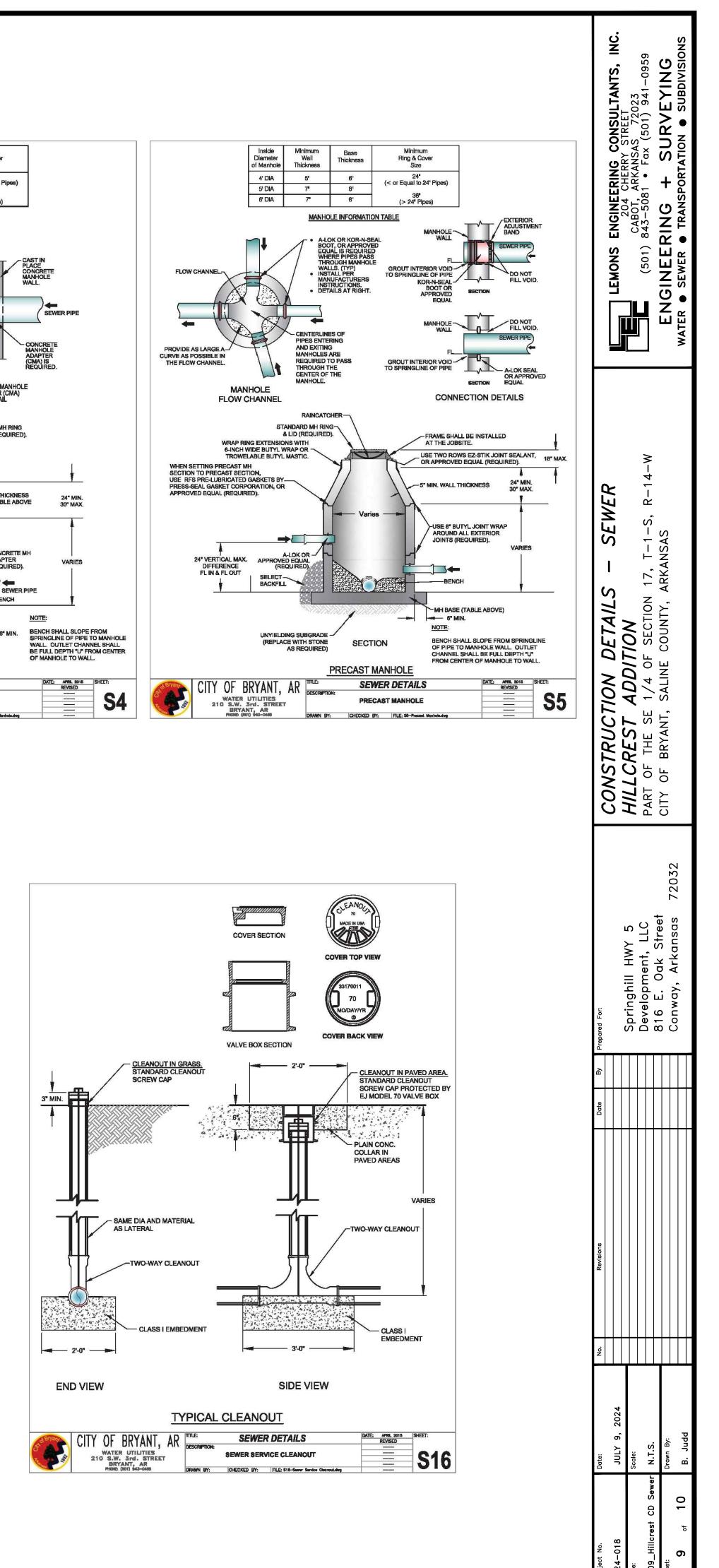








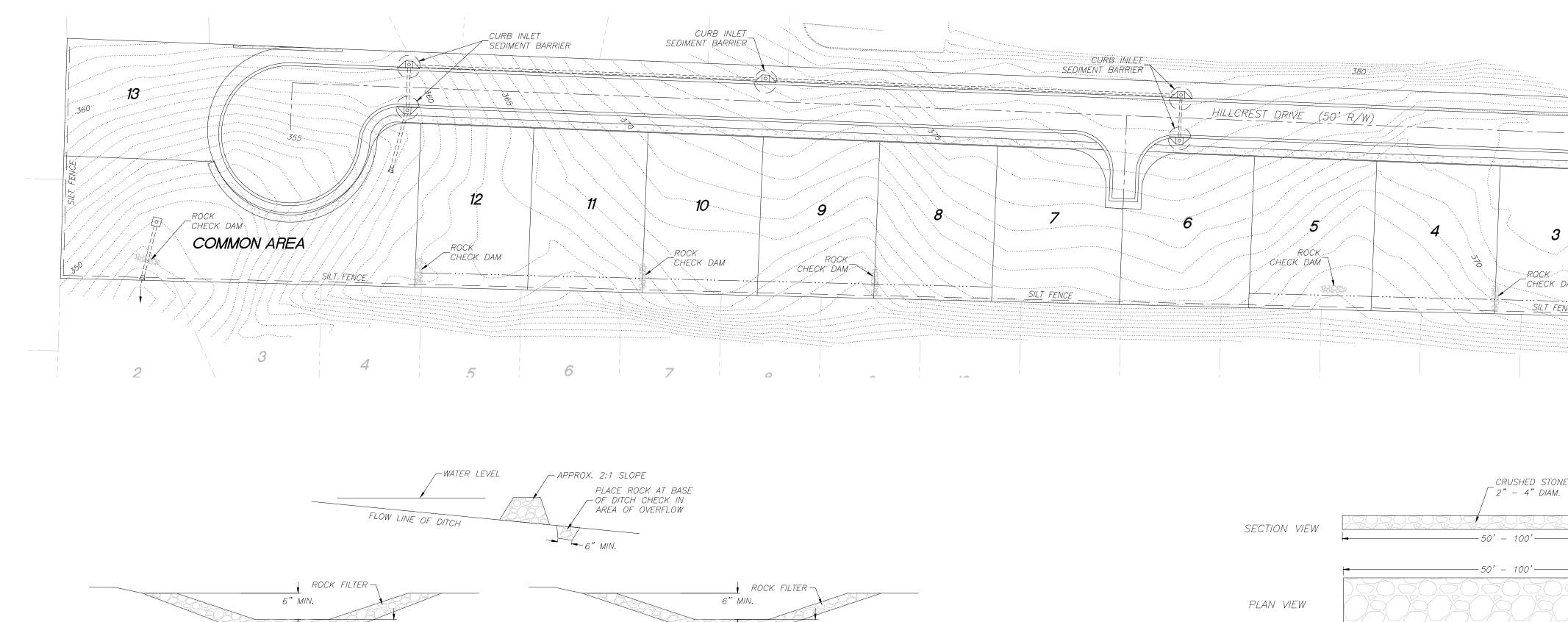






<u>GENERAL NOTES:</u>

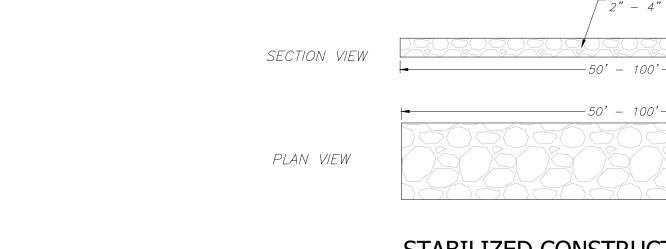
- 1.) A SILT FENCE AND STRAW BALE DIKE SHALL BE PLACED AT POTENTIAL LOCATIONS OF HEAVY EROSION.
- 2.) TEMPORARY STRAW BALE DIKES ARE TO BE CONSTRUCTED NOT TO POND WATER ON ADJACENT PROPERTY.
- 3.) ALL TEMPORARY EROSION CONTROLS SHALL BE MAINTAINED UNTIL ALL CONSTRUCTION IS COMPLETE & PERMANENT GROUND COVER HAS BEEN ESTABLISHED.
- 4.) ONE OF THE FOLLOWING GROUND COVER METHODS SHALL BE USED AT AREAS OF CLEARING OTHER THAN FUTURE PAVEMENT SURFACES:
- STRAW OR HAY-LOOSE 2.0 TONS/ACRE STRAW OR HAY-TIED, ANCHORED, OR TACKED 1.5 TONS/ACRE 5.) SOIL EXPOSED FOR MORE THAN 14 DAYS WITH NO
- CONSTRUCTION ACTIVITY SHALL BE SEEDED OR REVEGITATED. 6.) CONSTRUCTION EXITS SHALL BE MAINTAINED IN A CONDITION
- WHICH WILL PREVENT TRACKING ON FLOW OF MUD INTO PUBLIC RIGHT-OF-WAY.
- 7.) ADDITIONAL EROSION CONTROL MEASURES WILL BE EMPLOYED WHERE NECESSARY BY SITE CONDITIONS. 8.) CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING
- EROSION CONTROL MEASURES & PROVIDE RAIN FALL MONITORING & BI-WEEKLY INSPECTION REPORTS IN ACCORDANCE WITH THE NPDES PERMIT REQUIREMENTS.
- 9.) CONTRACTOR SHALL USE "BEST MANAGEMENT PRACTICES" (BMP'S) WHEN IMPLEMENTING & MAINTAINING SEDIMENT & RUN-OFF CONTROLS.
- 10.) THE USE OF "BIO-DEGRADABLE SOCK" IS ALLOWED AS OPPOSED TO SILT FENCE.



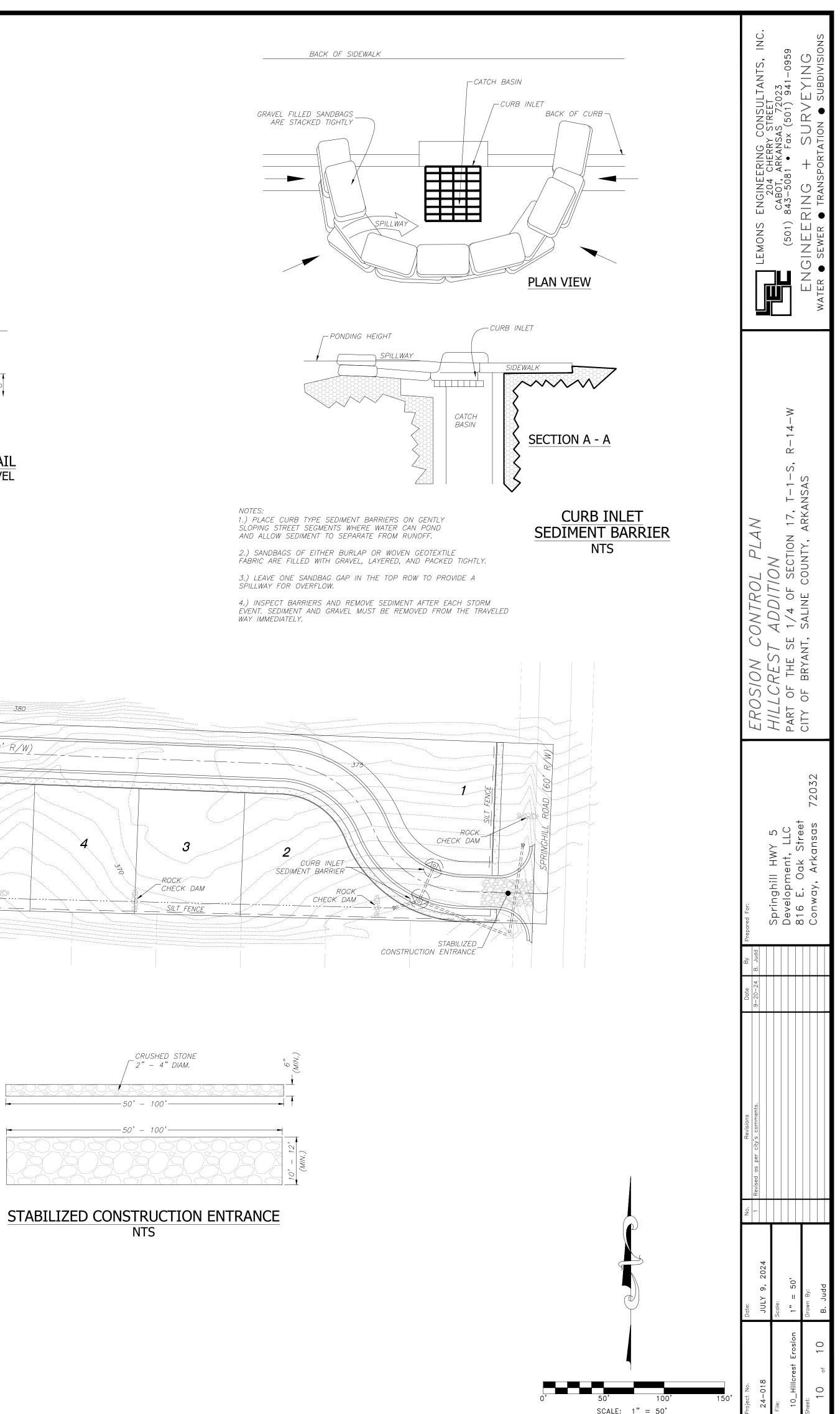
ROCK CHECK DAM NTS

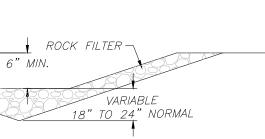
VARIABLE 18" TO 24" NORMAL

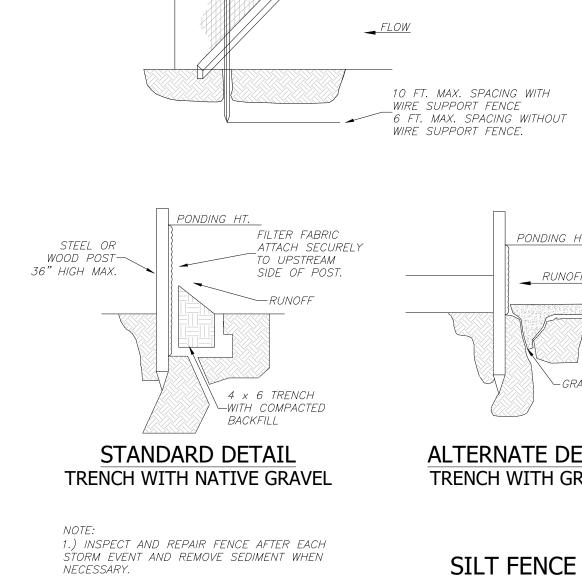




NTS







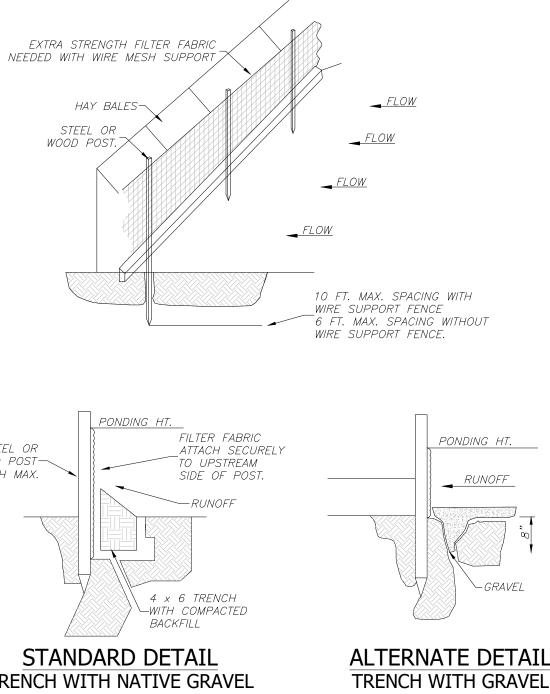
2.) REMOVED SEDIMENT SHALL BE DEPOSITED

3.) SILT FENCE SHALL BE PLACED ON SLOPE CONTOURS TO MAXIMIZE PONDING EFFICIENCY.

SEDIMENT OFF-SITE AND CAN BE PERMANENTLY

TÓ AN AREA THAT WILL NOT CONTRIBUTE

STABILIZED.





Lemons Engineering Consultants, Inc. 204 West Cherry Street Cabot, Arkansas 72023 (501) 605-7565 arstrep43@gmail.com

September 23, 2024

Mr. Colton Leonard, City Planner City of Bryant, Arkansas Community Development 210 SW 3rd Street Bryant, Arkansas 72022

Re: Preliminary Plat 3927 Springhill Road, Bryant, AR Parcel # 840-11855-000

Dear Mr. Leonard:

Enclosed you will find the revised Civil Plans, and Drainage Report as pertaining to the referenced project. A brief summary of the revisions are as follows. Please review the attached revised plans, and include this project on the agenda of the next available City of Bryant Planning Commission Meeting.

Public Works

- 1. Discuss where the sewer will tie in. *Connection is planned within the Hurricane Gardens development. See plans.*
- 2. Will roads be Public or Private? Public.
- 3. Site will require ADA Compliant Ramps and MUTCD Signage at Crosswalk. *References to this requirement have been added to the revised plans.*
- 4. Discuss Half Street Improvements. *Half Street Improvements have been added to the revised plans.*
- **5.** Gravity sewer- Manhole M-A-3 to M-A-1 will require minimum 20' easement per Bryant specifications section 1200-6-1.08, 1,2 and 3. *This easement has been added.*
- 6. How will the newly installed Gravity sewer main conflict with the existing retaining wall? No conflicts expected.
- 7. Street Department will require a geotechnical report for subdivision. *This shall be submitted upon receipt.*
- **8.** Streets will need to meet minimum subgrade and base standards. Current plans do not meet specifications. (Must be shown on plans) *Plans have been revised (see attached).*
- 9. MUTCD No Parking signs will be required to avoid Street Parking. See revised plans showing the placement of such signs.

10. Stop Sign at Springhill Rd and No Outlet sign will be required per MUTCD Standards **See** revised plans showing the placement of such signs.

11. Right of way on Springhill road will be 94 feet with half street improvements. *Revised, see plans.*

Stormwater

- 1. Discuss downstream drainage issues into Hurricane Gardens Subdivision? *Please see the attached revised plans and drainage report.*
- 2. Detention basin will require concrete trickle channels from all inflow to outflow points. *Trickle channels have been added to revised plans.*
- 3. Detention basin will require 3:1 safety slopes. Annotations with respect to slopes within the detention basin have been added.
- 4. Detention basin will require solid sod stabilization. A note pertaining to this item has been added to the revised plans.
- 5. Detention basin outflow pipes are required to be RCP, ADS does not meet specification. *This has been revised as requested.*
- 6. 8" sewer main can not run through any portion of the detention basin per Ordinance 2019-32. The sewer main is not being placed within the detention basin.
- 7. Discuss detention pond outflow pipe location and direction. See revised plans and report.
- 8. Will subdivision be put in a POA or Improvement District? POA is planned.

Engineering

- 1. Verify drainage area. Topography and eye witness accounts indicate the site receives runoff from north and east off-site and Springhill road. *This has been verified.*
- 2. 0890-DRN-03.PDF
- *a.* This off-site drainage plan shows the drainage basins that exist are apparently based upon the contours from Bryant's GIS, even though there is no reference to the source of the base map and contours. *Information added to the Drainage Report.*
- **b.** There does not appear to be any basins delineated which extend to the south property line of the proposed subdivision. Provide a map showing the pre-development basins for the site, with checkpoints at all locations where flows leave the site. *Information added to the Drainage Report.*
- 3. 0890-PLN-02.PDF
- a. On sheet 1, some of the lot dimensions are obscured on the drawing. This has been revised.
- b. On sheet 2:
- i. the curb and gutter detail does not match the detail show in the City of Bryant's standard curb and gutter section, Detail 7; *This has been revised.*
- *ii.* The typical street section does not match Detail 1 of the City of Bryant's typical section for Local 1 / Residential Streets; *This has been revised.*

*iii.*The pavement structure detail shown does not match the typical street section, see 3.b.ii. above. *This has been revised.*

- c. On sheet 3:
- i. The grading plan shows that grades will continue to slope south from the south curb and gutter on the street, unlike shown in the drainage calculations (see section 4 below). Grading Plan has been revised.
- *ii.* Significant stormwater is being allowed to drain off of the site to the south, it is recommended that the engineer look at diverting flow to the detention pond. *A swale has been added along the South*

property line to assist in diverting flow away from homes within Hurricane Gardens. See revised plans.

- *iii.* Will a separate drainage plan be developed for the plans? Profiles for the storm drainage? *Drainage profiles have been added to the plans.*
- iv. How will the discharge to Springhill Road be detained so that there is no increase in runoff from that part of the project? Due to the relatively small area flowing into the culvert system near the Southeast corner of the proposed development, no detention is being provided. However, the existing culvert does appear to have the capacity to accept the small increase in flow.
 - d. On sheet 4:
- *i.* Include a trickle channel in the pond; *Added.*
- ii. Include an emergency overflow for the pond, designed for the 100-year flow. This overflow must be 1-ft below the top of the levee. *Added.*
- *iii.* Verify that the slopes inside and outside of the pond are no steeper than 3 horizontal : 1 vertical. *All slopes have been verified.*
- *iv.* The outfall structure detail of the pond shown on the drawing is not labeled or titled. Show slopes, elevations, etc. *Information added as requested.*
- v. Provide solid sodding inside of the pond. A note pertaining to this item has been added to the plans.
- vi. How far will the closest building be to the pond? *Approximately 50 feet.*
- vii. The top of the levee on the detention pond must be .2-ft above the outfall box, include 1-ft of freeboard, and be 1-ft higher than the 100-year elevation inside of the pond. Based on our present model, we can only provide 18" of the requested 2'. We ask that the City grant a waiver on the 6" difference.
- *viii.* What downstream scour protection will there be below the outfall pipe? *Rip rap is planned for the discharge culvert from the pond.*
- *ix.* What checks have been performed in the drain to the southwest? Verify that the flows will not exceed the current flows in this area. *See Drainage Report.*
- *x.* Provide Scour protection from the inflow pipe on the east end of the pond. *A concrete trickle channel has been added to the end of the inflow FES.*
- e. On sheet 5:
- *i.* The main water line must be at least 8" up to the last fire hydrant, see city specifications. *This has been revised.*
- *ii.* The last fire hydrant shown on the west side of the cul-de-sac should be moved to the east side of the cul-de-sac. *Revised as requested.*
 - 4. Drainage Calculations:

a. For the runoff coefficient calculations on pages 5 and 6, please explain which basins A1, A2, A3, and A4 reflect. What about Basins B1 thru D3 and Ao thru Do2? **The runoff coefficients shown on pages 5 and 6 are used on all basins.**

b. Each return storm has it's own C-factor. Show the C-factor used for each of the return storms, on each basin. *This was shown in the previous revised report.*

c. Detention pond design volume must be increased by 25% as a safety factor. At the 25 year storm elevation (in the pond) the volume is 21,300.7 cf. At the 100 year storm elevation, the volume is 31,596.5 cf. That is above the 25% safety factor.

d. The emergency overflow spillway must be designed for the 100-year storm + 50% for blockage. *Emergency spillway has a capacity of 44.22 cfs, with a 100 year storm flowrate of 23.62 cfs.*

e. The map on page 9 does not show the pre-development basins, including the current discharge locations from the site. *Study Points have been added to all maps.*

f. Determine pre-development basins from the same discharge points for both pre-development and post-development conditions. *The comparison between Pre and Post flowrates are shown near the end of the Drainage Report (see Page 29).*

g. The basins shown on the map on page 10 does not reflect what the grading plan shows, see sheet 3 in the plans. It shows drainage of half of the lots going to the street, when the grading plan shows that flows will go to the south. **This has been corrected. See revised Drainage Report.**

h. Show check points for each basin so calculations can be followed. *Check points have been added to the maps within the Drainage Report.*

i. Show the hydraulic grade calculations for all of the storm drainage on the project (see section 600 in the stormwater manual). *This has been added to the Drainage Report.*

j. Adjust drainage calculations for all flows for a 28-ft street instead of a 27-ft street. Revised.

k. Show calculations for emergency spillway (include 50% blockage). *See previous comment and response.*

I. Are the time of concentration calculations on pages 16 and 17 showing that there is the same velocity for all basins? Which basins do these graphs apply to for the pond? *This is a comprehensive analysis using the entire watershed.*

m. On page 15 it refers to the C-factor for the detention facility was shown on pages 5 and 6 but the C-factor is different for each storm event, and that is not reflected on pages 5 and 6. *This was revised in an earlier submittal.*

n. For the detention pond calculations, what basin(s) did you use for the pre- and postdevelopment peak flows? *The areas behind the proposed houses in this new development were not included in the Detention computations. However, we have added a sod swale along the South of this development that will assist in sending most of the area to the Detention facility. See revised plans.*

o. Sheet 4 in the plans shows a detail for a box with a slotted weir topped with a 5" high rectangular weir. Please verify that calculations reflect the correct weir type. *Yes the calculations are based on this type of weir.*

p. Refer to section 1000.4.3 of the stormwater manual for multi-stage outlet design considerations.

q. Refer to section 1000.5.6 of the stormwater manual for configuration of the outlet structure.

r. Note allowable computer software in section 600.6 of the stormwater manual. *Our program is based on the City of Rogers Drainage Manual which has been used by basically all municipalities in Northwest Arkansas, and is considered an acceptable method.*

Com Dev

- 1. Cul-de-sac turnarounds must have a 50ft radius. Currently the plans show 49' to BOC. *This has been revised as requested.*
- 2. According to Subdivision Code, Cul-de-sac streets or courts designed to have one end permanently closed shall be no more than 550' long. This will have to be met or a modification from the subdivision code for a waiver on this requirement will have to be requested. Considering that all properties adjacent to this development have been developed, we formally ask for this waiver.
- **3.** Half-Street improvements to Springhill are required as part of this development. Springhill is designated as a minor arterial with a trail along the East Side. **Plans have been modified to show this.**
- 4. Typical street cross section shown on page 3 of plans does not meet our street specifications. City Street specification for local road shown below. *This has been corrected. Please see attached revised plans.*

- 5. Will this street be privately owned/maintained? *Street shall be public.*
- 6. Discuss stormwater and outfall of detention pond area. See previous responses in this letter.
- 7. Preliminary Plat application fees required to be paid \$664. *I will inform the owner.*

Please let me know if you need anything additional.

Sincerely,

enter

Tim Lemons, PE

Drainage Report

For

Hillcrest Addition

Springhill Road Bryant, Arkansas

Revised: September 23, 2024

Prepared By:



Lemons Engineering Consultants, Inc. 204 West Cherry Street Cabot, Arkansas 72023 (501) 605-7565 arstrep43@gmail.com

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Project Information

Project Title:	Hillcrest Addition
Project Description:	13 lot single family development located on the West side of Springhill Road, North of and adjacent to Hurricane Gardens, Bryant, Arkansas (address: 3927 Springhill Road)
Owner/Developer:	Springhill – Hwy 5 Development, LLC 816 East Oak Street Conway, Arkansas 72032
Engineer of Record:	Lemons Engineering Consultants, Inc. Tim Lemons, PE 204 Cherry Street Cabot, Arkansas 72023 (501) 605-7565

General Information

This proposed development shall include 13 single family lots. This property is essentially the Northern Most tract of land within the city limits of Bryant as they presently exist. The property to the North of the subject site is developed with duplex style residential structures. The property to the South is an established subdivision (Hurricane Gardens). The property drains North to South. There have been several reports of drainage issues by the residents of Hurricane Gardens. At present, the drainage from the subject property, and that to the north of the subject property, flows onto Hurricane Gardens. No detention exists on the property located north of the subject property. In this report, we will design a detention facility to accommodate the possible increase in flow for the subject property (Hillcrest Addition). Also, our goal is to divert a large majority of the drainage falling onto Hurricane Gardens. This diversion will force the runoff to the proposed detention facility on Hillcrest Addition as shown in the civil plans.

Project Vicinity Map



Source: ARCOUNTYDATA.com

Hydrological Computations

For this analysis, we will use the Rational Method in determining culvert sizes, culvert capacity computations, and other related issues on site. The total watershed size for this development is estimated at 10.57 acres. Attention is called to the Watershed Map included in this report. As per the Rational Method, the following equation is used:

Q = C x I x A, where: Q = Flowrate (cfs) C = Runoff Coefficient I = Intensity (from tables) A = area (acres)

The selection of the appropriate intensity is based on the estimated time of concentration (tc).

Determination of Runoff Coefficients "C"

In determining the Pre Construction C, we must consider the property to the North that is developed, and discharging onto the subject property. The C factor for Pre and Post Conditions are based on Table 400-1 "Runoff Coefficients for Surface Types" as provided in the Bryant Drainage Manual. A factored (weighted) value of C is determined in the following tables:

Storm Event	vent Off Site Off Site	Off Site Off Site	Off Site On Site	On Site	On Site	On Site	Weighted		
	C1	A1	C2	A2	C3	A3	C4	A4	С
		(acres)		(acres)		(acres)		(acres)	Factor
2	0.75	3.04	0.29	4.73	0.75	0.19	0.29	4.59	0.41
5	0.8	3.04	0.32	4.73	0.8	0.19	0.32	4.59	0.44
10	0.83	3.04	0.35	4.73	0.83	0.19	0.35	4.59	0.47
25	0.88	3.04	0.39	4.73	0.88	0.19	0.39	4.59	0.52
50	0.92	3.04	0.42	4.73	0.92	0.19	0.42	4.59	0.55
100	0.97	3.04	0.46	4.73	0.97	0.19	0.46	4.59	0.59

Pre Construction Conditions

C1 (off site for homes, streets, etc.)

C2 (off site for grass, landscaping, etc.)

C3 (on site for homes, streets, etc.)

C4 (on site for grass. Landscaping, etc.)

A1 (off site area for C1) A2 (off site area for C2) A3 (on site area for C3) A4 (on site area for C4)

Post Construction Conditions

Storm Event	Off Site	ff Site Off Site Off Site	Off Site	Off Site On	On Site	On Site	On Site	On Site	Weighted
	C1	A1	C2	A2	C3	A3	C4	A4	С
		(acres)		(acres)		(acres)		(acres)	Factor
2	0.75	3.04	0.29	4.73	0.75	1.81	0.29	3.07	0.47
5	0.8	3.04	0.32	4.73	0.8	1.81	0.32	3.07	0.50
10	0.83	3.04	0.35	4.73	0.83	1.81	0.35	3.07	0.53
25	0.88	3.04	0.39	4.73	0.88	1.81	0.39	3.07	0.58
50	0.92	3.04	0.42	4.73	0.92	1.81	0.42	3.07	0.61
100	0.97	3.04	0.46	4.73	0.97	1.81	0.46	3.07	0.66

C1 (off site for homes, streets, etc.) C2 (off site for grass, landscaping, etc.) C3 (on site for homes, streets, etc.)

C4 (on site for grass. Landscaping, etc.)

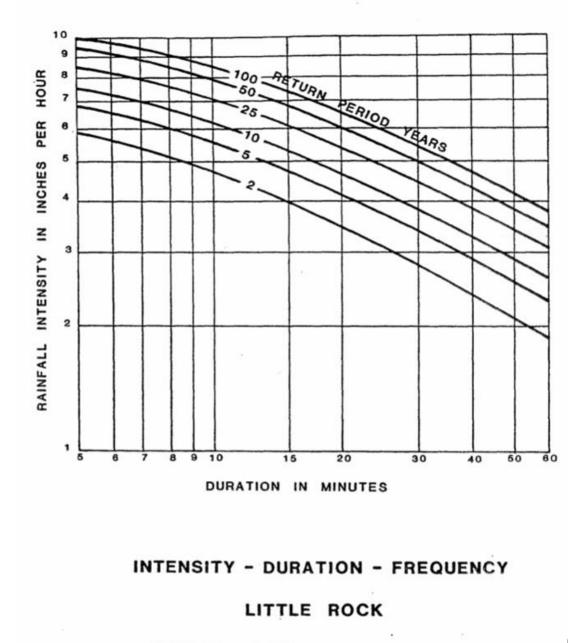
A1 (off site area for C1) A2 (off site area for C2) A3 (on site area for C3) A4 (on site area for C4)

The above variable values will be used in designing the Detention Facility. For culvert design,

we will use the Post C values for the 25 year storm.

Determination of Intensity Values "I"

For this analysis, we will use the Intensity – Duration - Frequency Chart from the Little Rock Drainage Manual. Whereas the calculated value of I shall be used for Detention, we will use a tc (time of concentration) of 5 min for the culverts to also provide a conservative value.





Determination of Flowrates for Culverts & Spreadflow

Attention is called to the following chart which provides C, Intensity, Area, and Flowrate (Q) of each Tract. Again, the Rational Method is being used for all basins. Attention is called to the Maps on the next two pages for a detailed drawings showing the various watershed tracts. The 25 year storm event will be used for culvert design. A conservative tc of 5 minutes is used for the culvert design.

Tract	C (post)	I	Α	Q
		(in/hr)	(ac)	(cfs)
Ao	0.58	8.5	2	9.86
Во	0.58	8.5	1.53	7.54
Со	0.58	8.5	1.73	8.53
Do1	0.58	8.5	1.92	9.47
Do2	0.58	8.5	0.59	2.91
A1	0.58	8.5	0.54	2.66
A2	0.58	8.5	0.29	1.43
A3	0.58	8.5	0.25	1.23
B1	0.58	8.5	0.47	2.32
B2	0.58	8.5	0.33	1.63
C1	0.58	8.5	0.49	2.42
C2	0.58	8.5	0.36	1.77
D1	0.58	8.5	0.44	2.17
D2	0.58	8.5	0.32	1.58
D3	0.58	8.5	1.15	5.67

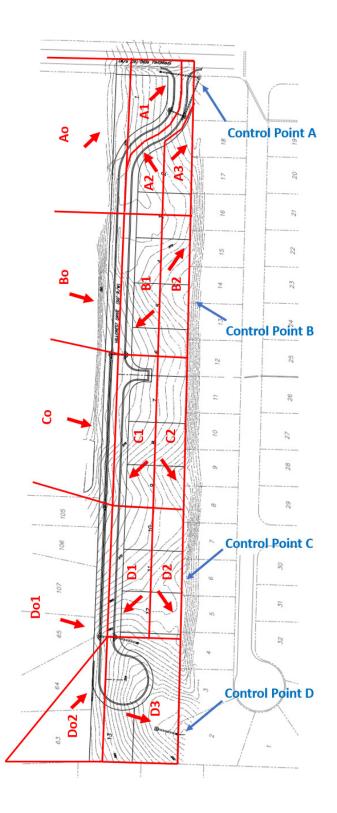


Drainage Watershed Map (Off Site)

Source: Saline County GIS Map

Drainage Watershed Map (On Site)

Revised 9/23/2024



Culvert Sizing

All culverts are sized to meet a 25 year storm, and the Rational Method is used. We will use a Manning's Coefficient of 0.012 shall be for all culverts (concrete and HDPE).

FES 1a

Q = Qao (2/3) = 9.86 (2/3) = 6.58 cfs Use 18" @ 0.7% Q capacity = 9.23 cfs V actual = 5.80 fps (d/D = 0.61)

Inlet 1

Q = Qao (2/3) + Qa1 = 9.86 (2/3) + 2.66 = 9.24 cfs Use 18" @ 1.1% Q capacity = 12.30 cfs V actual = 7.45 fps (d/D = 0.66)

Inlet 2

Q = Inlet 1 + Qa2 = 9.24 + 1.43 = 10.67 cfs Use 18" @ 1.4% Q capacity = 13.87 cfs V actual = 8.44 fps (d/D = 0.67)

Junction Box 3 (verify capacity)

Q = Qao + Qa1 + Qa2 + Exist 18" in Hurricane Gardens (Culvert in Hurricane Gardens is an 18" ADS at 0.46%, Capacity = 7.95 cfs at d/D=0.85) Q = 9.86 + 2.66 + 1.43 + 7.95 = 21.90 cfs Existing 24" Discharging from Junc Box is 24" ADS @ 5.20% Q capacity = 57.58 cfs *Capacity appears to exist* V actual = 16.61 fps (d/D = 0.43)

Inlet 4

Q = Qb1 = 2.32 cfs Use 18" @ 0.5% Q capacity = 8.29 cfs V actual = 4.00 fps (d/D = 0.38)

Inlet 5

Q = Inlet 4 + Qbo = 2.32 + 7.54 = 9.86 cfs Use 18" @ 1.0% Q capacity = 11.73 cfs V actual = 7.35 fps (d/D = 0.71)

Inlet 6

Q = Inlet 5 + Qco = 9.86 + 8.53 = 18.39 cfs Use 18" @ 4.60% Q capacity = 25.15 cfs V actual = 15.18 fps (d/D = 0.65)

Inlet 7

Q = Inlet 6 + Qdo1 = 18.39 + 9.47 = 27.86 cfs Use 24" @ 2.0% Q capacity = 35.71 cfs V actual = 12.22 fps (d/D = 0.67)

Inlet 8

Q = Inlet 7 + Qd1 = 27.86 + 2.17 = 30.03 cfs Use 18" @ 2.25% Q capacity = 37.87 cfs V actual = 13.06 fps (d/D = 0.69)

Street Spreadflow Analysis (Gutter Capacity)

In this Section of the Report, we will examine how the stormwater in the street gutters may impact in proposed inlets. We will use our 27' street width (back of curb to back of curb), while giving allowances for the vertical portion of the curb on each side. The crown on the street shall be 3.0%. The available street width, to handle the stormwater, has a width of 26'. Our goal is to provide a minimum "non submerged" street width ("clear space") of 8 feet. A Manning's Coefficient of 0.12 is used for the pavement surface. Attention is called to the Appendix for the spreadsheets used to evaluate these areas.

Check Inlet 1 & 2 – Hillcrest Drive

Inlet 1 Q = QaO(1/2) + Qa1/2 = 9.86 (0.5) + 2.66 (0.5) = 6.26 cfsGutter Slope = 1.50% Height of water (from gutter) = 0.30' Width of water (from gutter) = 10.0' Clear space (half street) = 13.0 - 10.0' = 3.0' Inlet 2 Q = Qa2 = 1.43 cfsGutter Slope = 1.50% Height of water (from gutter) = 0.17' Width of water (from gutter) = 5.5' Clear space (half street) = 13.0 - 5.5' = 7.5' Total Clear Space = 3.0 + 7.5 = 10.5'

Check Inlet 4 & 5 – Hillcrest Drive

Inlet 4 Q = Qb1 = 2.32 cfsGutter Slope = 0.5%Height of water (from gutter) = 0.26' Width of water (from gutter) = 8.5' Clear space (half street) = 13.0 - 8.5' = 4.5'Inlet 5 $Q = Qbo = 7.54 \, \text{fps}$ Gutter Slope = 0.5% Height of water (from gutter) = 0.39' Width of water (from gutter) = 13.0'Clear space (half street) = 13.0 - 13.0' = 0.0'Total Clear Space = 4.5 + 0.0 = 4.5' **TRY 10 YEAR STORM** Inlet 4 Q = Qb1 = 1.97 cfsGutter Slope = 0.5% Height of water (from gutter) = 0.23'Width of water (from gutter) = 7.5' Clear space (half street) = 13.0 - 7.5' = 5.5'Inlet 5 Q = Qbo = 6.41 fpsGutter Slope = 0.5%Height of water (from gutter) = 0.36' Width of water (from gutter) = 12.0' Clear space (half street) = 13.0 - 12.0' = 1.0'Total Clear Space = 5.5 + 1.0 = 6.5'

Check Inlet 6 & Across Street – Hillcrest Drive

Inlet 6 Q = Qco = 8.53 cfs Gutter Slope = 2.67% Height of water (from gutter) = 0.30' Width of water (from gutter) = 10.0' Clear space (half street) = 13.0 - 10.0' = 3.0'Across from Inlet 6 Q = Qc1 = 2.42 cfs Gutter Slope = 2.67% Height of water (from gutter) = 0.18' Width of water (from gutter) = 6.0' Clear space (half street) = 13.0 - 6.0' = 7.0'Total Clear Space = 3.0 + 7.0 = 10.0'

Check Inlet 7 & 8 – Hillcrest

Inlet 7

Q = Qdo1(1/2) = 9.47 (0.5) = 4.74 cfs Gutter Slope = 4.88% Height of water (from gutter) = 0.21' Width of water (from gutter) = 7.0' Clear space (half street) = 13.0 - 7.0' = 6.0'Inlet 8 Q = Qc1 + Qd1 = 2.42 + 2.17 = 4.59 cfs Gutter Slope = 4.88%Height of water (from gutter) = 0.21'Width of water (from gutter) = 7.0'Clear space (half street) = 13.0 - 7.0' = 6.0'Total Clear Space = 6.0 + 6.0 = 12.0'

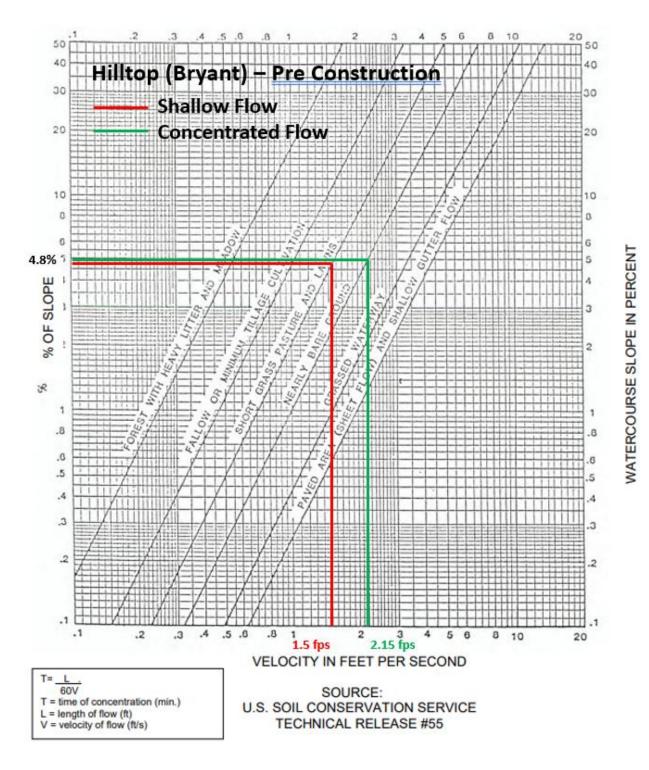
Detention Facility Computations

In this section, we will size the detention facility located in the Common Area (West side of the project). At the completion of this section, a summary of pre and post flows will be provided. Whereas the time of concentration will be used to determine the intensity (I), the runoff coefficient (C) for each storm analysis shall be based on that determined on pages 5 and 6 of this report.

Time of Concentration (tc)

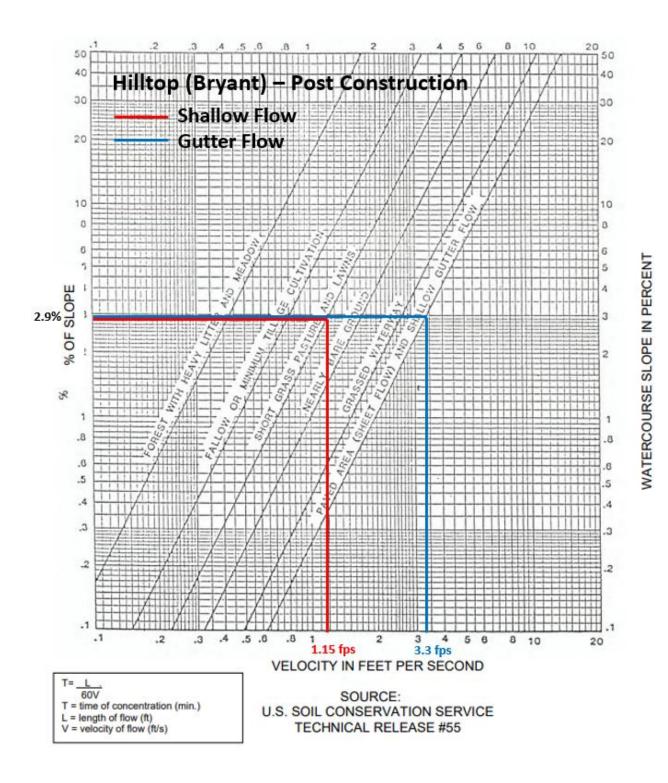
In determining the time of concentration, we must first determine the velocity of the runoff based on the type of ground cover and type of flow. The total tc is a sum of the tc for overland flow, the tc for shallow concentrated flow, and the tc for channelized flow. For this analysis, we will use the US Soil Conservation Service Technical Release #55, "Watercourse Slope vs Velocity" graph. A Pre Construction and Post Construction graph for each watershed is provided on the following pages.

Pre Construction Time of Concentration (tc)



Pre-Construction tc = $\Sigma(L/(60)(V))$ = 6 min

Post Construction Time of Concentration (tc)



Post-Construction tc = $\Sigma(L/(60)(V))$ = 9 min

Stage – Storage Table

The following Stage Storage Table is provided, based on the grading plan contained in the Civil

TYPE 3							
Stage - Storage for Irregular Detention Basin							
Top Elev	Bottom Elev	Increment					
353.5	345.5	1					
Stage	Area	Δ Volume	Volume				
msl	sf	cf	cf				
345.50	1	0	0				
346.50	1853.50	927.25	927.25				
347.50	2951.40	2402.45	3329.70				
348.50	4240.13	3595.77	6925.47				
349.50	5637.46	4938.80	11864.26				
350.50	7118.75	6378.11	18242.37				
351.50	8673.71	7896.23	26138.60				
352.50	10265.99	9469.85	35608.45				
353.50	11858.27	11062.13	46670.58				

Plans. The accumulative storage is provided in the right most column.

Stage – Discharge Table

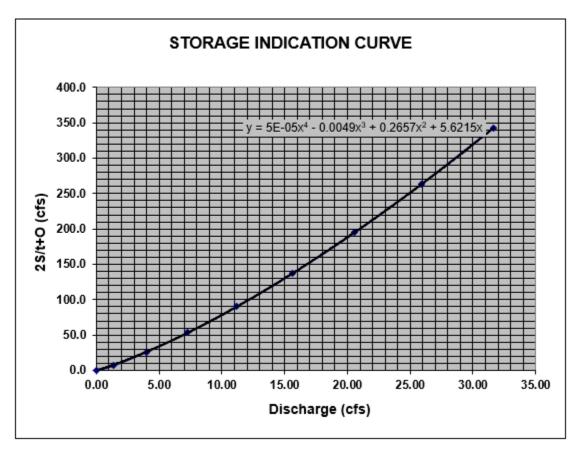
The following Stage Discharge Table is provided, based on the grading plan contained in the Civil Plans. The discharge structure planned for this facility is shown later in this report.

т	YI	PE	Ξ2	2
		-	_	-

Stage - Discharge for Rectangular Weir						
Beginning	Elevation	Tan of Davin				
Elevation	Increment	Top of Basin				
345.50	1.00	353.50				
	Beginning Elevation	Beginning Elevation Elevation Increment				

Stage	Head (H)	Weir Length (L)	Area (A)	Orifice Coefficient (C)	Velocity	Discharge (Q)
Stage	neau (n)	(Ľ)	Alea (A)	(0)	velocity	Discharge (G)
msl	ft	ft	sf		ft/s	cfs
345.50	0.00	0.42	0.00	3.33	0.00	0.00
346.50	1.00	0.42	0.42	3.33	3.33	1.40
347.50	2.00	0.42	0.84	3.33	4.71	3.96
348.50	3.00	0.42	1.26	3.33	5.77	7.27
349.50	4.00	0.42	1.68	3.33	6.66	11.19
350.50	5.00	0.42	2.10	3.33	7.45	15.64
351.50	6.00	0.42	2.52	3.33	8.16	20.56
352.50	7.00	0.42	2.94	3.33	8.81	25.90
353.50	8.00	0.42	3.36	3.33	9.42	31.65

Storage Indication Curve



Alternate Routing Time

The following spreadsheets represent the Hydrograph Routing for the various storm events. In each case, the Routing Storm Duration time was adjusted to provide the maximum storage required. Also, runoff coefficients C have been adjusted for each storm event:

Storm Event	Pre C	Post C
2	0.41	0.47
5	0.44	0.50
10	0.47	0.53
25	0.52	0.58
50	0.55	0.61
100	0.59	0.66

Coefficients for Storage Indication Curve from Chart								
Ax ⁴	Bx ³	Cx ²	Dx					
0.0001	-0.0049	0.2657	5.6215					

HYDROGRAPH ROUTING FOR 2 YEAR DESIGN STORM

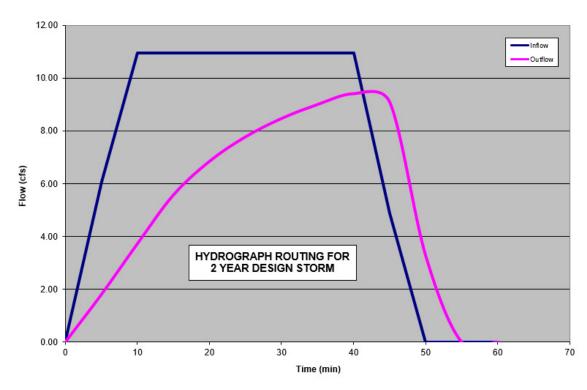
Routing Storm Duration

20 minutes

	1	2	3	4	5	6	7	8
Time	l ₁	$ _{1}+ _{2}$	2S ₁ /t-Q ₁	2S ₂ /t+Q ₂	Q ₂	S ₂	2S/t-Q	Col 4 - 7
min	cfs	cfs	cfs	cfs	cfs	cf	from eqn.	
0	0.00	15.97	0	15.973	0	0	15.972	0.001
5	8.87	24.85	10.876	35.723	2.548	2013.7	35.724	-0.001
10	15.97	31.95	25.334	57.280	5.194	4579.3	57.280	0.000
15	15.97	31.95	41.818	73.764	7.731	7432.4	73.764	-0.001
20	15.97	23.07	54.737	77.809	9.513	9637.5	77.810	-0.001
25	7.10	7.10	57.939	65.038	9.935	10181.1	8.503	56.535
30	0.00	0.00	62.199	62.199	1.520	9527.8	0.000	62.199
35	0.00	0.00	50.999	50.999	0.000	9329.8	0.000	50.999
40	0.00	0.00	51.199	51.199	0.000	7649.8	0.000	51.199
45	0.00	0.00	51.399	51.399	0.000	7679.8	0.000	51.399
50	0.00	0.00	51.599	51.599	0.000	7709.8	0.000	51.599
55	0.00	0.00	51.799	51.799	0.000	7739.8	0.000	51.799
60	0.00	0.00	51.999	51.999	0.000	7769.8	0.000	51.999

Actual Maximum Storage needed is 10181.1 cubic feet Maximum Storage required is achieved at an elev. = 349.32 Maximum Allowable (undeveloped) Discharge is 11.99 cfs Maximum Discharge for the above storm is 9.93 cfs

DETENTION HYDROGRAPH



Coefficients for Storage Indication Curve from Chart								
Ax ⁴	Bx ³	Cx ²	Dx					
0.0001	-0.0049	0.2657	5.6215					

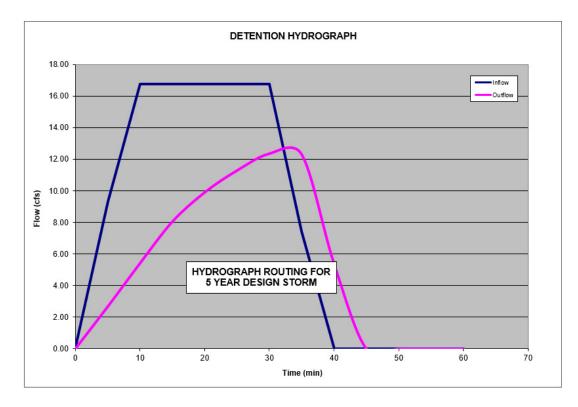
HYDROGRAPH ROUTING FOR 5 YEAR DESIGN STORM

Routing Storm Duration

30 minutes

	1	2	3	4	5	6	7	8
Time	l ₁	$ _{1}+ _{2}$	2S ₁ /t-Q ₁	2S ₂ /t+Q ₂	Q ₂	S ₂	2S/t-Q	Col 4 - 7
min	cfs	cfs	cfs	cfs	cfs	cf	from eqn.	
0	0.00	16.75	0	16.750	0	0	16.749	0.001
5	9.31	26.06	11.428	37.484	2.661	2113.4	37.485	-0.001
10	16.75	33.50	26.658	60.157	5.413	4810.6	60.158	-0.001
15	16.75	33.50	44.056	77.555	8.051	7816.0	77.555	0.001
20	16.75	33.50	57.739	91.238	9.908	10147.0	91.236	0.002
25	16.75	33.50	68.648	102.147	11.295	11991.4	102.146	0.001
30	16.75	24.19	77.420	101.614	12.364	13467.5	101.615	-0.001
35	7.44	7.44	76.989	84.434	12.312	13395.2	36.484	47.950
40	0.00	0.00	73.856	73.856	5.389	11856.7	0.000	73.856
45	0.00	0.00	74.056	74.056	0.000	11078.4	0.000	74.056
50	0.00	0.00	74.256	74.256	0.000	11108.4	0.000	74.256
55	0.00	0.00	74.456	74.456	0.000	11138.4	0.000	74.456
60	0.00	0.00	74.656	74.656	0.000	11168.4	0.000	74.656

Actual Maximum Storage needed is 13467.5 cubic feet Maximum Storage required is achieved at an elev. = 349.89 Maximum Allowable (undeveloped) Discharge is 14.9 cfs Maximum Discharge for the above storm is 12.36 cfs



Coefficients for Storage Indication Curve from Chart							
Ax ⁴	Bx ³	Cx ²	Dx				
0.0001	-0.0049	0.2657	5.6215				

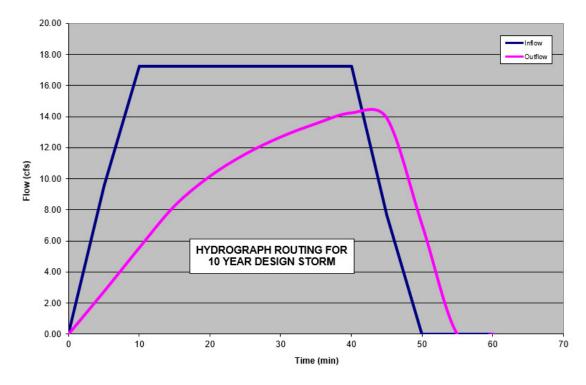
HYDROGRAPH ROUTING FOR 10 YEAR DESIGN STORM

Routing Storm Duration 40 minutes

40	minutes							
	1	2	3	4	5	6	7	8
Time	l ₁	$ _{1}+ _{2}$	2S ₁ /t-Q ₁	2S ₂ /t+Q ₂	Q ₂	S ₂	2S/t-Q	Col 4 - 7
min	cfs	cfs	cfs	cfs	cfs	cf	from eqn.	
0	0.00	17.24	0	17.240	0	0	17.239	0.001
5	9.58	26.82	11.778	38.596	2.731	2176.3	38.597	-0.001
10	17.24	34.48	27.496	61.977	5.550	4956.9	61.976	0.001
15	17.24	34.48	45.475	79.956	8.251	8058.9	79.956	0.000
20	17.24	34.48	59.644	94.124	10.156	10469.9	94.123	0.001
25	17.24	34.48	70.962	105.442	11.581	12381.4	105.442	0.000
30	17.24	34.48	80.081	114.561	12.681	13914.2	114.560	0.001
35	17.24	34.48	87.472	121.953	13.544	15152.5	121.953	0.000
40	17.24	24.90	93.489	118.391	14.232	16158.1	118.390	0.001
45	7.66	7.66	90.587	98.249	13.902	15673.3	50.238	48.011
50	0.00	0.00	84.384	84.384	7.032	13682.5	0.000	84.384
55	0.00	0.00	84.584	84.584	0.000	12657.6	0.000	84.584
60	0.00	0.00	84.784	84.784	0.000	12687.6	0.000	84.784

Actual Maximum Storage needed is 16158.1 cubic feet Maximum Storage required is achieved at an elev. = 350.25 Maximum Allowable (undeveloped) Discharge is 17.36 cfs Maximum Discharge for the above storm is 14.23 cfs

DETENTION HYDROGRAPH



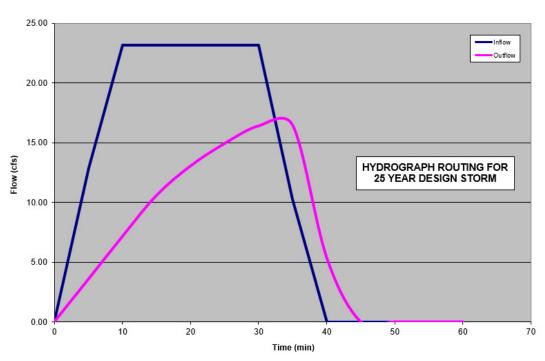
Coefficients for Storage Indication Curve from Chart								
Ax ⁴	Bx ³	Cx ²	Dx					
0.0001	-0.0049	0.2657	5.6215					

HYDROGRAPH ROUTING FOR 25 YEAR DESIGN STORM

Routing	Storm Duration	l
30	minutes	

30	minutes							
	1	2	3	4	5	6	7	8
Time	l ₁	$ _{1}+ _{2}$	2S ₁ /t-Q ₁	2S ₂ /t+Q ₂	Q ₂	S ₂	2S/t-Q	Col 4 - 7
min	cfs	cfs	cfs	cfs	cfs	cf	from eqn.	
0	0.00	23.16	0	23.158	0	0	23.159	-0.001
5	12.87	36.02	16.041	52.065	3.559	2939.9	52.066	-0.001
10	23.16	46.32	37.780	84.097	7.142	6738.4	84.098	-0.001
15	23.16	46.32	62.939	109.256	10.579	11027.7	109.256	0.000
20	23.16	46.32	83.168	129.485	13.044	14431.8	129.484	0.001
25	23.16	46.32	99.641	145.958	14.922	17184.4	145.957	0.001
30	23.16	33.45	113.166	146.617	16.396	19434.3	146.616	0.001
35	10.29	10.29	113.708	124.001	16.454	19524.4	36.239	87.762
40	0.00	0.00	113.484	113.484	5.359	17796.3	0.000	113.484
45	0.00	0.00	113.684	113.684	0.000	17022.6	0.000	113.684
50	0.00	0.00	113.884	113.884	0.000	17052.6	0.000	113.884
55	0.00	0.00	114.084	114.084	0.000	17082.6	0.000	114.084
60	0.00	0.00	114.284	114.284	0.000	17112.6	0.000	114.284

Actual Maximum Storage needed is 19524.4 cubic feet Maximum Storage required is achieved at an elev. = 350.65 Maximum Allowable (undeveloped) Discharge is 19.53 cfs Maximum Discharge for the above storm is 16.45 cfs



DETENTION HYDROGRAPH

Coefficients for Storage Indication Curve from Chart								
Ax ⁴	Bx ³	Cx ²	Dx					
0.0001	-0.0049	0.2657	5.6215					

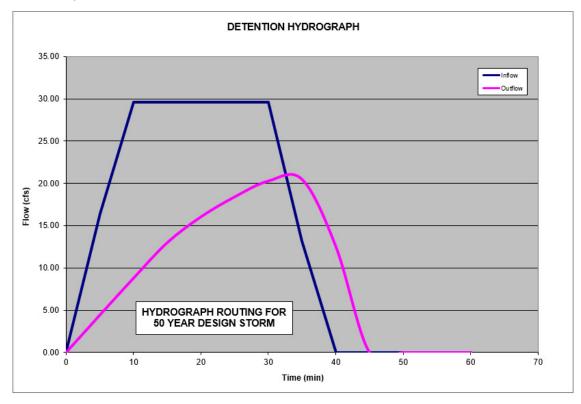
HYDROGRAPH ROUTING FOR 50 YEAR DESIGN STORM

Routing Storm Duration

30	minutes
00	mmutes

	1	2	3	4	5	6	7	8
Time	I ₁	$ _{1}+ _{2}$	2S ₁ /t-Q ₁	2S ₂ /t+Q ₂	Q ₂	S ₂	2S/t-Q	Col 4 - 7
min	cfs	cfs	cfs	cfs	cfs	cf	from eqn.	
0	0.00	29.62	0	29.616	0	0	29.615	0.001
5	16.45	46.07	20.781	66.850	4.417	3779.8	66.850	0.000
10	29.62	59.23	49.291	108.522	8.779	8710.6	108.523	-0.001
15	29.62	59.23	82.573	141.804	12.974	14332.1	141.804	0.000
20	29.62	59.23	109.747	168.978	16.029	18866.3	168.978	0.000
25	29.62	59.23	132.202	191.433	18.388	22588.5	191.434	0.000
30	29.62	42.78	150.904	193.682	20.265	25675.3	193.683	-0.001
35	13.16	13.16	152.783	165.945	20.449	25984.8	102.860	63.085
40	0.00	0.00	141.080	141.080	12.532	23011.9	0.000	141.080
45	0.00	0.00	141.280	141.280	0.000	21162.0	0.000	141.280
50	0.00	0.00	141.480	141.480	0.000	21192.0	0.000	141.480
55	0.00	0.00	141.680	141.680	0.000	21222.0	0.000	141.680
60	0.00	0.00	141.880	141.880	0.000	21252.0	0.000	141.880

Actual Maximum Storage needed is 25984.8 cubic feet Maximum Storage required is achieved at an elev. = 351.37 Maximum Allowable (undeveloped) Discharge is 25.68 cfs Maximum Discharge for the above storm is 20.45 cfs



Coefficients for Storage Indication Curve from Chart									
Ax ⁴	Bx ³	Cx ²	Dx						
0.0001	-0.0049	0.2657	5.6215						

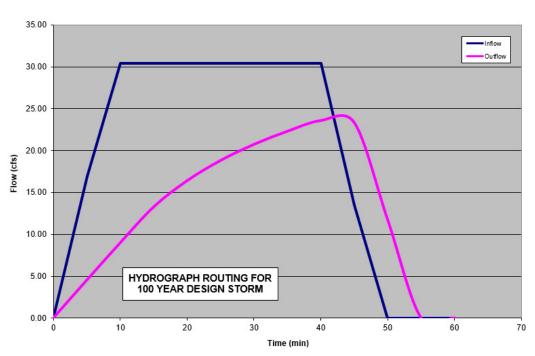
HYDROGRAPH ROUTING FOR 100 YEAR DESIGN STORM

Routing Storm Duration

40	minutes
----	---------

	1	2	3	4	5	6	7	8
Time	l ₁	$ _{1}+ _{2}$	2S ₁ /t-Q ₁	2S ₂ /t+Q ₂	Q ₂	S ₂	2S/t-Q	Col 4 - 7
min	cfs	cfs	cfs	cfs	cfs	cf	from eqn.	
0	0.00	30.44	0	30.441	0	0	30.440	0.001
5	16.91	47.35	21.393	68.745	4.524	3887.5	68.746	-0.001
10	30.44	60.88	50.780	111.662	8.983	8964.4	111.661	0.001
15	30.44	60.88	85.119	146.000	13.272	14758.5	146.001	-0.001
20	30.44	60.88	113.200	174.082	16.400	19440.0	174.083	-0.001
25	30.44	60.88	136.442	197.324	18.820	23289.3	197.323	0.001
30	30.44	60.88	155.829	216.711	20.747	26486.5	216.710	0.001
35	30.44	60.88	172.091	232.972	22.310	29160.1	232.973	-0.001
40	30.44	43.97	185.788	229.759	23.592	31407.1	229.758	0.001
45	13.53	13.53	183.077	196.607	23.341	30962.7	94.375	102.232
50	0.00	0.00	173.395	173.395	11.706	27735.1	0.000	173.395
55	0.00	0.00	173.595	173.595	0.000	26009.2	0.000	173.595
60	0.00	0.00	173.795	173.795	0.000	26039.2	0.000	173.795

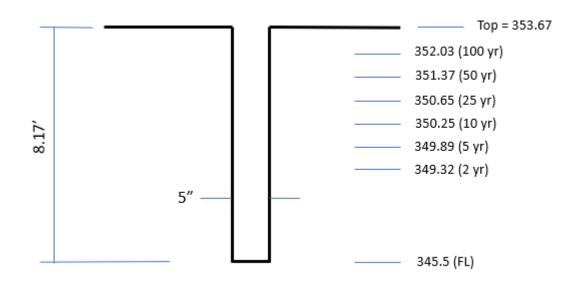
Actual Maximum Storage needed is 31407.1 cubic feet Maximum Storage required is achieved at an elev. = 352.03 Maximum Allowable (undeveloped) Discharge is 29.36 cfs Maximum Discharge for the above storm is 23.59 cfs



DETENTION HYDROGRAPH

	Sur	mmary – De	tention	
Storm Event	Volume Needed (cf)	WSE	Max Discharge Allowed (cfs)	Max Discharge Model (cfs)
2	10181.1	349.32	11.99	9.93
5	13467.5	349.89	14.90	12.36
10	16158.1	350.25	17.36	14.23
25	19524.4	350.65	19.53	16.45
50	25984.8	351.37	25.68	20.45
100	31407.1	352.03	29.36	23.59

Discharge Structure Detail





Study Point Summary (25 yr Storm)

Study Point	Pre Construction	Post Construction	Change
А	15.18 cfs	16.81 cfs	1.63 cfs*
В	11.49 cfs	0.00 cfs	-11.49 cfs
С	12.72 cfs	0.00 cfs	-12.72 cfs
D	18.46 cfs	16.45 cfs	-2.01 cfs
Total:			-24.59 cfs

* Existing culvert originating at the Junction Box near the NE corner of Hurricane Gardens has adequate capacity to accept this slight increase in flow.

Engineering Certification

I, Tim Lemons, Arkansas Registered Professional Engineer No. 7373, hereby certify that the drainage reports, and calculations contained in this report, have been prepared in accordance with sound engineering practice and principles, and based on best known available data. Improvements as outlined in this report and depicted on the preliminary plat and design drawings should not increase the risk of endangerment to life or have negative impacts on adjacent or downstream property or watersheds.



Timothy B. Lemons, PE Arkansas Professional Engineer, #7373

Appendix

		ITY OF STE						
Slope =	= 0.5%, r	า = 0.012						
Width	Slope	Height	Area	R	R^2/3	S	S^1/2	Q
(ft)	-	(ft)	(sf)		-			(cfs)
0.5	0.030	0.02	0.00	0.01	0.04	0.00500	0.0707	0.00
1	0.030	0.03	0.02	0.01	0.06	0.00500	0.0707	0.01
1.5	0.030	0.05	0.03	0.02	0.08	0.00500	0.0707	0.02
2	0.030	0.06	0.06	0.03	0.10	0.00500	0.0707	0.05
2.5	0.030	0.08	0.09	0.04	0.11	0.00500	0.0707	0.09
3	0.030	0.09	0.14	0.04	0.13	0.00500	0.0707	0.15
3.5	0.030	0.11	0.18	0.05	0.14	0.00500	0.0707	0.22
4	0.030	0.12	0.24	0.06	0.15	0.00500	0.0707	0.32
4.5	0.030	0.14	0.30	0.07	0.16	0.00500	0.0707	0.44
5	0.030	0.15	0.38	0.07	0.18	0.00500	0.0707	0.58
5.5	0.030	0.17	0.45	0.08	0.19	0.00500	0.0707	0.75
6	0.030	0.18	0.54	0.09	0.20	0.00500	0.0707	0.94
6.5	0.030	0.20	0.63	0.10	0.21	0.00500	0.0707	1.17
7	0.030	0.21	0.74	0.10	0.22	0.00500	0.0707	1.43
7.5	0.030	0.23	0.84	0.11	0.23	0.00500	0.0707	1.71
8.5	0.030	0.26	1.08	0.13	0.25	0.00500	0.0707	2.39
9	0.030	0.27	1.22	0.13	0.26	0.00500	0.0707	2.79
9.5	0.030	0.29	1.35	0.14	0.27	0.00500	0.0707	3.22
10	0.030	0.30	1.50	0.15	0.28	0.00500	0.0707	3.69
10.5	0.030	0.32	1.65	0.16	0.29	0.00500	0.0707	4.21
11	0.030	0.33	1.82	0.16	0.30	0.00500	0.0707	4.76
11.5	0.030	0.35	1.98	0.17	0.31	0.00500	0.0707	5.36
12	0.030	0.36	2.16	0.18	0.32	0.00500	0.0707	6.01
12.5	0.030	0.38	2.34	0.19	0.33	0.00500	0.0707	6.70
13	0.030	0.39	2.54	0.19	0.33	0.00500	0.0707	7.44

		ITY OF ST						
Slope =	= 1.50%,	n = 0.012	2					
Width	Slope	Height	Area	R	R^2/3	S	S^1/2	Q
	Slope			R	R ⁴ 2/5	3	5/1/2	
(ft)		(ft)	(sf)					(cfs)
0.5	0.030	0.02	0.00	0.01	0.04	0.01500	0.1225	0.00
1	0.030	0.03	0.02	0.01	0.06	0.01500	0.1225	0.01
1.5	0.030	0.05	0.03	0.02	0.08	0.01500	0.1225	0.04
2	0.030	0.06	0.06	0.03	0.10	0.01500	0.1225	0.09
2.5	0.030	0.08	0.09	0.04	0.11	0.01500	0.1225	0.16
3	0.030	0.09	0.14	0.04	0.13	0.01500	0.1225	0.26
3.5	0.030	0.11	0.18	0.05	0.14	0.01500	0.1225	0.39
4	0.030	0.12	0.24	0.06	0.15	0.01500	0.1225	0.55
4.5	0.030	0.14	0.30	0.07	0.16	0.01500	0.1225	0.76
5	0.030	0.15	0.38	0.07	0.18	0.01500	0.1225	1.01
5.5	0.030	0.17	0.45	0.08	0.19	0.01500	0.1225	1.30
6	0.030	0.18	0.54	0.09	0.20	0.01500	0.1225	1.64
6.5	0.030	0.20	0.63	0.10	0.21	0.01500	0.1225	2.03
7	0.030	0.21	0.74	0.10	0.22	0.01500	0.1225	2.47
7.5	0.030	0.23	0.84	0.11	0.23	0.01500	0.1225	2.97
8.5	0.030	0.26	1.08	0.13	0.25	0.01500	0.1225	4.15
9	0.030	0.27	1.22	0.13	0.26	0.01500	0.1225	4.83
9.5	0.030	0.29	1.35	0.14	0.27	0.01500	0.1225	5.58
10	0.030	0.30	1.50	0.15	0.28	0.01500	0.1225	6.40
10.5	0.030	0.32	1.65	0.16	0.29	0.01500	0.1225	7.29
11	0.030	0.33	1.82	0.16	0.30	0.01500	0.1225	8.25
11.5	0.030	0.35	1.98	0.17	0.31	0.01500	0.1225	9.29
12	0.030	0.36	2.16	0.18	0.32	0.01500	0.1225	10.41
12.5	0.030	0.38	2.34	0.19	0.33	0.01500	0.1225	11.61
13	0.030	0.39	2.54	0.19	0.33	0.01500	0.1225	12.89

Slong -	2 67%	n = 0.012						
siope -	- 2.0770,	11 - 0.017	2					
Width	Slope	Height	Area	R	R^2/3	S	S^1/2	Q
(ft)		(ft)	(sf)					(cfs)
0.5	0.030	0.02	0.00	0.01	0.04	0.02670	0.1634	0.00
1	0.030	0.03	0.02	0.01	0.06	0.02670	0.1634	0.02
1.5	0.030	0.05	0.03	0.02	0.08	0.02670	0.1634	0.05
2	0.030	0.06	0.06	0.03	0.10	0.02670	0.1634	0.12
2.5	0.030	0.08	0.09	0.04	0.11	0.02670	0.1634	0.21
3	0.030	0.09	0.14	0.04	0.13	0.02670	0.1634	0.34
3.5	0.030	0.11	0.18	0.05	0.14	0.02670	0.1634	0.52
4	0.030	0.12	0.24	0.06	0.15	0.02670	0.1634	0.74
4.5	0.030	0.14	0.30	0.07	0.16	0.02670	0.1634	1.01
5	0.030	0.15	0.38	0.07	0.18	0.02670	0.1634	1.34
5.5	0.030	0.17	0.45	0.08	0.19	0.02670	0.1634	1.73
6	0.030	0.18	0.54	0.09	0.20	0.02670	0.1634	2.18
6.5	0.030	0.20	0.63	0.10	0.21	0.02670	0.1634	2.70
7	0.030	0.21	0.74	0.10	0.22	0.02670	0.1634	3.29
7.5	0.030	0.23	0.84	0.11	0.23	0.02670	0.1634	3.96
8.5	0.030	0.26	1.08	0.13	0.25	0.02670	0.1634	5.53
9	0.030	0.27	1.22	0.13	0.26	0.02670	0.1634	6.44
9.5	0.030	0.29	1.35	0.14	0.27	0.02670	0.1634	7.44
10	0.030	0.30	1.50	0.15	0.28	0.02670	0.1634	8.53
10.5	0.030	0.32	1.65	0.16	0.29	0.02670	0.1634	9.72
11	0.030	0.33	1.82	0.16	0.30	0.02670	0.1634	11.0
11.5	0.030	0.35	1.98	0.17	0.31	0.02670	0.1634	12.4
12	0.030	0.36	2.16	0.18	0.32	0.02670	0.1634	13.8
12.5	0.030	0.38	2.34	0.19	0.33	0.02670	0.1634	15.4
13	0.030	0.39	2.54	0.19	0.33	0.02670	0.1634	17.2

siope =	- 4.88%,	n = 0.012						
Width	Slope	Height	Area	R	R^2/3	S	S^1/2	Q
(ft)		(ft)	(sf)					(cfs)
0.5	0.030	0.02	0.00	0.01	0.04	0.04880	0.2209	0.00
1	0.030	0.03	0.02	0.01	0.06	0.04880	0.2209	0.02
1.5	0.030	0.05	0.03	0.02	0.08	0.04880	0.2209	0.07
2	0.030	0.06	0.06	0.03	0.10	0.04880	0.2209	0.16
2.5	0.030	0.08	0.09	0.04	0.11	0.04880	0.2209	0.28
3	0.030	0.09	0.14	0.04	0.13	0.04880	0.2209	0.46
3.5	0.030	0.11	0.18	0.05	0.14	0.04880	0.2209	0.70
4	0.030	0.12	0.24	0.06	0.15	0.04880	0.2209	1.00
4.5	0.030	0.14	0.30	0.07	0.16	0.04880	0.2209	1.37
5	0.030	0.15	0.38	0.07	0.18	0.04880	0.2209	1.81
5.5	0.030	0.17	0.45	0.08	0.19	0.04880	0.2209	2.34
6	0.030	0.18	0.54	0.09	0.20	0.04880	0.2209	2.95
6.5	0.030	0.20	0.63	0.10	0.21	0.04880	0.2209	3.65
7	0.030	0.21	0.74	0.10	0.22	0.04880	0.2209	4.45
7.5	0.030	0.23	0.84	0.11	0.23	0.04880	0.2209	5.35
8.5	0.030	0.26	1.08	0.13	0.25	0.04880	0.2209	7.48
9	0.030	0.27	1.22	0.13	0.26	0.04880	0.2209	8.71
9.5	0.030	0.29	1.35	0.14	0.27	0.04880	0.2209	10.06
10	0.030	0.30	1.50	0.15	0.28	0.04880	0.2209	11.54
10.5	0.030	0.32	1.65	0.16	0.29	0.04880	0.2209	13.14
11	0.030	0.33	1.82	0.16	0.30	0.04880	0.2209	14.88
11.5	0.030	0.35	1.98	0.17	0.31	0.04880	0.2209	16.76
12	0.030	0.36	2.16	0.18	0.32	0.04880	0.2209	18.77
12.5	0.030	0.38	2.34	0.19	0.33	0.04880	0.2209	20.94
13	0.030	0.39	2.54	0.19	0.33	0.04880	0.2209	23.25

CONSTRUCTION PLANS FIRST SOUTHERN CHURCH OF BRYANT 604 S REYNOLDS ROAD, BRYANT, SALINE COUNTY, ARKANSAS





OWNER:

Name: Peter Cunningham

Address:	604 S Reynolds Rd, Bryant,
	Arkansas 72022
Email &	peter@fsbcbryant.org
Phone:	501-847-3014

DEVELOPER:

Name: Peter Cunningham (04 C D arra alda D d D r

Address:	604 S Reynolds Rd, Bryant,
_	Arkansas 72022
Email &	peter@fsbcbryant.org
Phone:	501-847-3014

PREPARED BY:

129 N. Main Street, Benton, Arkansas 72015 PH. (501)315-2626 FAX (501) 315-0024 www.hopeconsulting.com

CIVIL ENGINEER HOPE CONSULTING INC 129 N. MAIN STREET BENTON, AR 72015

DRAWING INDEX

SHEET NO.	TITLE
	BOUNDARY & TOPO SURVEY
C-1.0	SITE PLAN
C-2.0	UTILITY PLAN
C-3.0	SEWER PLAN & PROFILE
C-4.0	GRADING PLAN
C-5.0	STORM DRAINAGE PLAN AND PROFILE
C-6.0	RETENTION PLAN
C-7.0	LANDSCAPE PLAN
C-8.0	EROSION PLAN
C-9.0	DEMOLITION PLAN



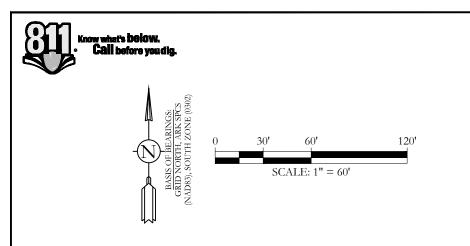




FIRST SOUTHERN CHURCH OF BRYANT FSCB EXPANSION & REMODEL

FOR USE AND BENEFIT OF:

	E		604 S I	REYN	R SHI NOLDS R COUNT		SAS	5	
DATE:	9/4/2024		C.A.D	. BY:			D	RAWING	G NUMBER:
REVISED :	9-25-2024		CHEC	KEI	O BY:			$\mathcal{O}A$	-0260
SHEET:			SCAL	E:				<u> </u>	-0260
500	01S	14	W	0	34	310		62	1664



REFERENCE DOCUMENTS CITED RECORDED SURVEY PLATS BY: JOHN C. WILLIAMS (PS 1091), FOR MURPHY, DATED 10/5/2000 KERRY D. LANE (PS 1141), FOR W. GRAY, DATED 8/12/2013 FINAL PLAT OF BRYANT MEADOWS SUBD., PHASE 3, AS RECORDED IN BOOK 337, PAGE 452 JOHN A. LANE (PS 1740), FOR B. GRAY, DATED 3/18/2014 PLAT OF FISCHER'S SUBDIVISION, AS RECORDED IN BOOK 35, PAGE 400 SURVEY DETAILS AND NOTES:

OWNER OF RECORD: FIRST SOUTHERN BAPTIST CHURCH PHYSICAL ADDRESS: 604 S. REYNOLDS RD, BRYANT, AR COUNTY PARCEL TAX ID: 840-15972-003, 840-15972-000, 840-15973-000

ALL DIMENSIONS LISTED ARE AS MEASURED BY THIS SURVEYOR UNLESS OTHERWISE NOTED. FOR RECORD DIMENSIONS SEE DOCUMENTS OF RECORD.

OWNERSHIP INFORMATION, IF SHOWN, IS LISTED AS PUBLISHED BY THE LOCAL COUNTY TAX ASSESSOR AND IS LISTED FOR REFERENCE ONLY. NO STATEMENTS OF OWNERSHIP, RIGHTS, OR INTERESTS ARE MADE

THIS SURVEY IS BASED ON PUBLIC RECORDS AND/OR TITLE INVESTIGATIONS FURNISHED BY THIRD PARTIES. NO INDEPENDENT SEARCH OR INVESTIGATION HAS BEEN MADE BY THIS FIRM FOR ANY RECORDS, PUBLIC OR PRIVATE. LISTED REFERENCE DOCUMENTS HEREON WERE USED AND CONSIDERED AS A PART OF THIS SURVEY; HOWEVER OTHER RECORDS, IF ANY, COULD FURTHER AFFECT THIS SURVEY. NO STATEMENT OR GUARANTEES OF OWNERSHIP, RIGHTS, OR OTHER INTERESTS ARE MADE BY THIS SURVEY PLAT.

RECORD PROPERTY DESCRIPTION SALINE COUNTY INSTRUMENT 2000-05544

ALL THAT PART OF THE NORTHEAST QUARTER OF THE SOUTHWEST QUARTER OF SECTION 34, TOWNSHIP SOUTH, RANGE 14 WEST, CITY OF BRYANT, SALINE COUNTY, ARKANSAS, MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCING AT A 5/8 INCH REBAR ACCEPTED AS BEING THE SOUTHWEST CORNER OF SAID NE1/4 OF THE SW 1/4. SECTION 34: THENCE NORTH 89 DEG. 55 MIN. 27 SEC. EAST. ALONG THE SOUTH LINE THEREOF. 451.2 FEET TO A COTTON PICKER SPINDLE IN THE CENTERLINE OF GRIFFIS ROAD AND BEING THE POINT OF BEGINNING OF LANDS HEREIN DESCRIBED; THENCE SOUTH 89 DEG. 52 MIN. 03 SEC. EAST, ALONG THE CENTERLINE OF GRIFFIS ROAD AND THE SOUTH LINE OF SAID NE 1/4 OF THE SW 1/4, 313.90 FEET TO A RAILROAD SPIKE; THENCE SOUTH 89 DEG. 59 MIN. 25 SEC. EAST, ALONG THE CENTERLINE OF GRIFFIS ROAD AND THE SOUTH LINE OF SAID NE 1/4 OF THE SW 1/4, 197.38 FEET TO A COTTON PICKER SPINDLE IN THE WEST RIGHT OF WAY LINE OF ARKANSAS STATE HIGHWAY NO. 183; THENCE ALONG SAID STATE HIGHWAY RIGHT OF WAY LINE AND A CURVE TO THE RIGHT HAVING A RADIUS OF 1472.40 FEET FOR A CHORD OF NORTH 22 DEG. 3 MIN 45 SEC. EAST 359.84 FEET TO A REBAR: THENCE NORTH 60 DEG. 22 MIN. 07 SEC. WEST. ALONG SAID STATE HIGHWAY RIGHT OF WAY LINE, 20.00 FEET TO A REBAR; THENCE NORTH 29 DEG. 37 MIN. 53 SEC. EAST, ALONG SAID STATE HIGHWAY RIGHT OF WAY LINE, 477.50 FEET TO A REBAR IN THE EAST LINE OF SAID NE 1/4 OF THE SW 1/4: THENCE NORTH 02 DEG. 45 MIN. 15 SEC. EAST. ALONG EAST LINE OF SAID NE 1/4 OF THE SW 1/4. 288.55 FEET TO A RAILROAD SPIKE IN THE CENTERLINE OF ELM STREET AND THE SOUTHEAST CORNER OF LAND CONVEYED TO CITY OF BRYANT, ARKANSAS IN WARRANTY DEED DATED JUNE 15, 1961 AND FILED IN SALINE COUNTY DEED RECORD BOOK 103 AT PAGE 119 (SAID POINT BEING LOCATED SOUTH 02 DEG. 46 MIN. 30 SEC. WEST 296.59 FEET FROM THE NORTHEAST CORNER OF SAID NE 1/4 OF SW 1/4, SECTION 34); THENCE LEAVING SAID ELM STREET, SOUTH 89 DEG. 53 MIN. 52 SEC. WEST 130.05 FEET TO A REBAR AND THE SOUTHWEST CORNER OF SAID LAND CONVEYED TO CITY OF BRYANT; THENCE NORTH 02 DEG. 48 MIN. 12 SEC. EAST 111.00 FEET TO A REBAR AND THE NORTHWEST CORNER OF SAID LAND CONVEYED TO CITY OF BRYANT; THENCE SOUTH 89 DEG. 54 MIN. 12 SEC. WEST, ALONG THE SOUTH LINE OF LAND GRANTED TO T. W. COLE, JR. IN DECREE OF CONFIRMATION AND QUIETING OF TITLE DATED OCTOBER 31, 1968 AND FILED IN SALINE COUNTY DEED RECORD BOOK 135 AT PAGE 848, 15.10 FEET: THENCE SOUTH 02 DEG. 48 MIN. 12 SEC. WEST, ALONG THE EAST LINE OF LAND CONVEYED TO T. W. COLE, JR. AND MARY COLE, HUSBAND AND WIFE, IN WARRANTY DEED DATED JULY 29, 1974 AND FILED IN SALINE COUNTY DEED RECORD BOOK 182 AT PAGE 164, 111.50 FEET;

THENCE SOUTH 89 DEG. 34 MIN. 17 SEC. WEST, ALONG THE SOUTH LINE OF LANDS CONVEYED TO T. W. COLE. IR, AND MARY COLE, HUSBAND AND WIFE, IN SAID DEED BOOK 182 AT PAGE 164, 185.00 FEET; THENCE NORTH 02 DEG. 48 MIN. 12 SEC. EAST, ALONG THE WEST LINE OF SAID LANDS CONVEYED IN DEED BOOK 182 AT PAGE 164, 112.98 FEET; THENCE SOUTH 89 DEG. 34 MIN. 17 SEC. WEST, ALONG THE SOUTH LINE OF LANDS CONVEYED TO ROBERT LEE NOWLIN IN WARRANTY DEED DATED MARCH 16, 1971 AND FILED IN SALINE COUNTY DEED RECORD BOCK 146 AT PAGE 487 AND AS CONVEYED TO ROBERT LEE NOWLIN AND MARIE NELL NOWLIN, HUSBAND AND WIFE, IN CORRECTION WARRANTY DEED DATED SEPTEMBER 19, 1974 AND FILED IN SALINE COUNTY DEED RECORD BOOK 194 AT PAGE 104, 187.62 FEET; THENCE NORTH 00 DEG, 25 MIN. 43 SEC. WEST. ALONG THE WEST LINE OF LANDS CONVEYED TO SAID ROBERT LEE NOWLIN AND MARIE NELL NOWLIN HUSBAND AND WIFE, 16.50 FEET TO THE SOUTHEAST CORNER OF LOT 6, FISCHER'S SUBDIVISION; THENCE SOUTH 89 DEG. 34 MIN. 17 SEC. WEST, ALONG THE SOUTH LINE OF SAID FISCHER'S SUBDIVISION, 436.00 FEET TO THE SOUTHWEST CORNER OF LOT 2, FISCHER'S SUBDIVISION; THENCE SOUTH 00 DEG. 25 MIN. 43 SEC. EAST, ALONG THE EAST LINE OF LAND CONVEYED TO PAUL DOUGLAS GATTIN AND ALISA ANN GATTIN. HUSBAND AND WIFE, IN WARRANTY DEED DATED JANUARY 14, 1986 AND FILED IN SALINE COUNTY DEED RECORD BOOK 279 AT PAGE 512, 19.00 FEET; THENCE SOUTH 89 DEG. 34 MIN. 17 SEC. WEST, ALONG THE SOUTH LINE OF LAND CONVEYED TO SAID PAUL DOUGLAS GATTIN AND ALISA ANN GATTIN, HUSBAND AND WIFE, 87.20 FEET; THENCE NORTH 00 DEG. 25 MIN. 43 SEC. WEST, ALONG THE WEST LINE OF LAND CONVEYED TO SAID PAUL DOUGLAS GATTIN AND ALISA ANN GATTIN. HUSBAND AND WIFE, 19.00 FEET TO THE SOUTHWEST CORNER OF LOT 1, FISCHER'S SUBDIVISION; THENCE SOUTH 89 DEG. 35 MIN. 22 SEC. WEST, ALONG THE SOUTH LINE OF LAND CONVEYED TO JOHN L. JACKSON AND ROBIN A. JACKSON, HUSBAND AND WIFE, IN WARRANTY DEED DATED JULY 3, 1986 AND FILED IN SALINE COUNTY DEED RECORD BOOK 284 AT PAGE 118, 269.38 FEET TO A COTTON PICKER SPINDLE IN THE CENTERLINE OF SANDERS ROAD AND THE WEST LINE OF SAID NE 1/4 OF THE SW 1/4 (SAID POINT BEING LOCATED SOUTH 01 DEG. 24 MIN. 00 SEC. WEST 166.5 FEET FROM THE NORTHWEST CORNER THEREOF); THENCE SOUTH 01 DEG. 24 MIN. 00 SEC. WEST, ALONG THE CENTERLINE OF SANDERS ROAD AND THE WEST LINE OF SAID NE 1/4 OF SW 1/4, 33.54 FEET TO A COTTON PICKER SPINDLE; THENCE LEAVING SAID ROAD. SOUTH 8S DEG. 26 MIN. 35 SEC. EAST. ALONG THE NORTH LINE OF LAND

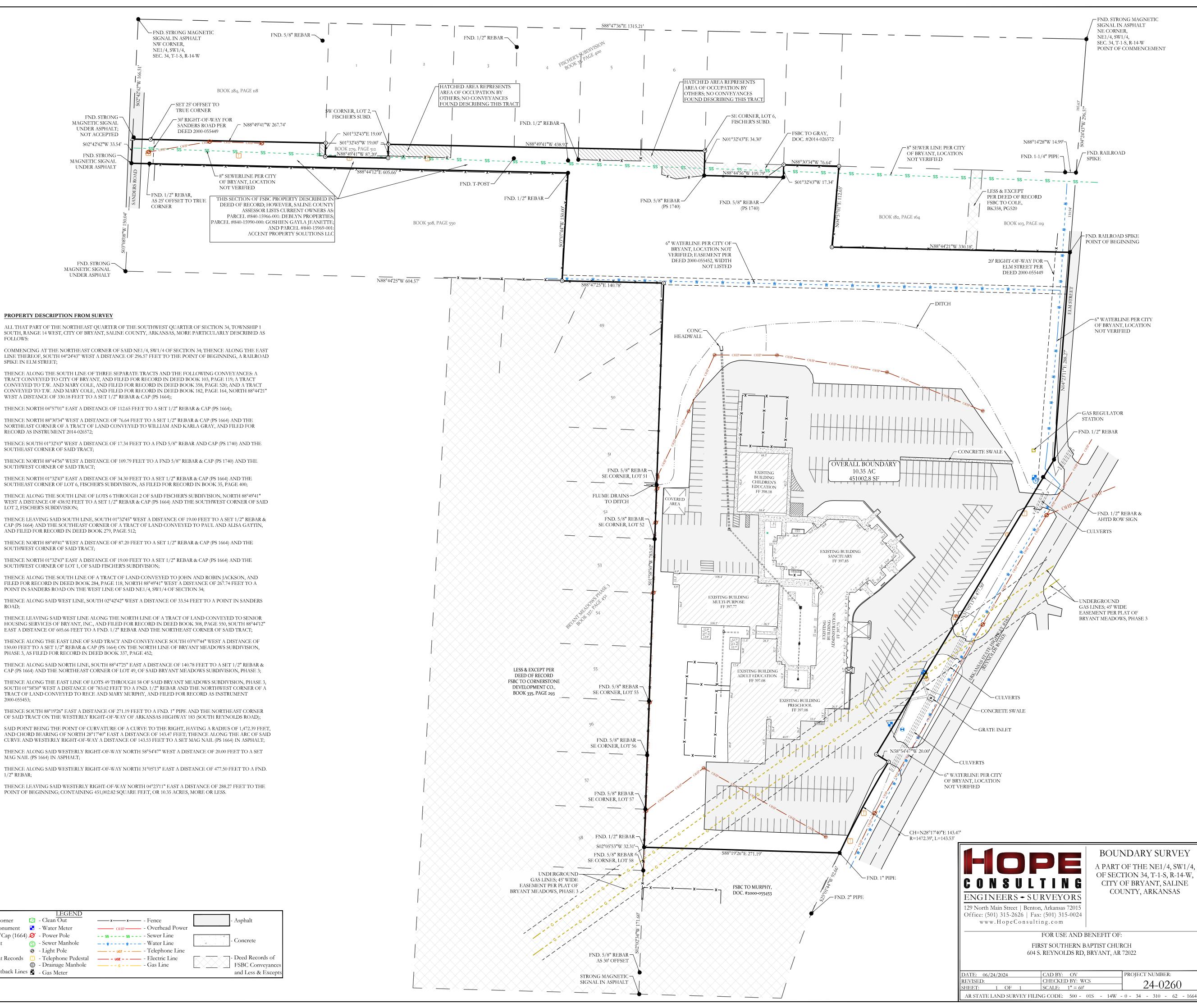
FILED IN SALINE COUNTY DEED RECORD BOOK 308 AT PAGE 550, 606.92 FEET TO A REBAR; THENCE SOUTH 01 DEG. 29 MIN. 25 SEC. WEST, ALONG THE EAST LINE OF LAND CONVEYED TO SAID SENIOR HOUSING SERVICES OF BRYANT, INC., 150.00 FEET TO A REBAR; THENCE NORTH 89 DEG. 26 MIN. 34 SEC. WEST, ALONG THE SOUTH LINE OF LAND CONVEYED TO SAID SENIOR HOUSING SERVICES OF BRYANT, INC., 152.99 FEET TO A REBAR; THENCE SOUTH 01 DEG 33 MIN 04 SEC WEST ALONG THE EAST LINE OF LANDS CONVEYED TO CORNERSTONE DEVELOPMENT CO. IN WARRANTY DEED DATED SEPTEMBER 6, 1989 AND FILED IN SALINE COUNTY DEED RECORD BOOK 322 AT PAGE 285, 877.01 FEET TO THE POINT OF BEGINNING, CONTAINING 18.4662 ACRES, MORE OR LESS. SUBJECT TO A 30 FOOT ROAD RIGHT OF WAY FOR GRIFFIS ROAD. SUBJECT TO A 20 FOOT ROAD RIGHT OF WAY FOR ELM STREET. SUBJECT TO A 30 FOOT ROAD RIGHT OF WAY FOR SANDERS ROAD. SUBJECT TO A WATER LINE EASEMENT TRAVERSING THE NORTHERN PORTION OF LANDS DESCRIBED HEREINABOVE. SUBJECT TO A GAS LINE EASEMENT TRAVERSING SUBJECT PROPERTY.

CONVEYED TO SENIOR HOUSING SERVICES OF BRYANT. INC. IN WARRANTY DEED DATED JULY 11, 1988 AND

LESS AND EXCEPT: THAT PORTION OF THE NORTHEAST QUARTER OF THE SOUTHWEST QUARTER OF SECTION 34, TOWNSHIP 1 SOUTH, RANGE 14 WEST, CITY OF BRYANT, SALINE COUNTY, ARKANSAS, MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGINNING AT THE SOUTHWEST CORNER OF THE NE 1/4 OF SW 1/4, THENCE NORTH 02 DEG. 45 MIN. 10 SEC. EAST (ASTRONOMIC) 982.05 FEET (DEED NORTH 01 DEG. 24 MIN. EAST 982.11 FEET) ALONG THE WEST LINE THEREOF: THENCE SOUTH 89 DEG. 01 MIN. 06 SEC. EAST 452.81 FEET TO THE NORTHEAST CORNER OF LANDS DESCRIBED IN SALINE COUNTY DEED RECORD BOOK 322 AT PAGE 285, SAID POINT BEING THE POINT OF BEGINNING OF LAND HEREIN DESCRIBED; THENCE CONTINUE SOUTH 89 DEG. 01 MIN. 06 SEC. EAST 294.31 FEET; THENCE SOUTH 01 DEG. 41 MIN. 45 SEC. WEST 986.72 FEET TO THE SOUTH LINE OF SAID NE 1/4 OF SW 1/4: THENCE NORTH 88 DEG. 43 MIN. 48 SEC. WEST. ALONG SAID SOUTH LINE. 313.90 FEET TO THE SOUTHEAST CORNER OF LANDS DESCRIBED IN SALINE COUNTY DEED RECORD BOOK 322 AT PAGE 285: THENCE NORTH 02 DEG. 50 MIN. 15 SEC. EAST 984.37 FEET, ALONG THE EAST LINE OF LANDS DESCRIBED IN SAID DEED, TO THE POINT OF BEGINNING.

ALSO, LESS AND EXCEPT: THAT PART OF THE NORTHEAST QUARTER OF THE SOUTHWEST QUARTER OF SECTION 34, TOWNSHIP 1 SOUTH, RANGE 14 WEST, SALINE COUNTY, ARKANSAS, DESCRIBED AS FOLLOWS: BEGINNING AT A POINT THAT IS 185.5 FEET SOUTH OF THE NORTHEAST CORNER OF SAID NE 1/4 OF SW 1/4 AND RUN THENCE WEST FOR 130 FEET FOR THE POINT OF BEGINNING OF LAND HEREIN DESCRIBED; RUN THENCE WEST FOR 15 FEET; THENCE SOUTH 111.5 FEET; THENCE EAST 15 FEET; THENCE NORTH 111.5 FEET TO THE POINT OF BEGINNING



PROPERTY DESCRIPTION FROM SURVEY

FOLLOWS:

SPIKE IN ELM STREET;

WEST A DISTANCE OF 330.18 FEET TO A SET 1/2" REBAR & CAP (PS 1664);

SOUTHEAST CORNER OF SAID TRACT;

SOUTHWEST CORNER OF SAID TRACT;

CAP (PS 1664) AND THE SOUTHEAST CORNER OF A TRACT OF LAND CONV

SOUTHWEST CORNER OF SAID TRACT:

POINT IN SANDERS ROAD ON THE WEST LINE OF SAID NE1/4, SW1/4 OF SECTION 34;

ROAD;

PHASE 3, AS FILED FOR RECORD IN DEED BOOK 337, PAGE 452;

2000-055453:

MAG NAIL (PS 1664) IN ASPHALT;

1/2" REBAR;

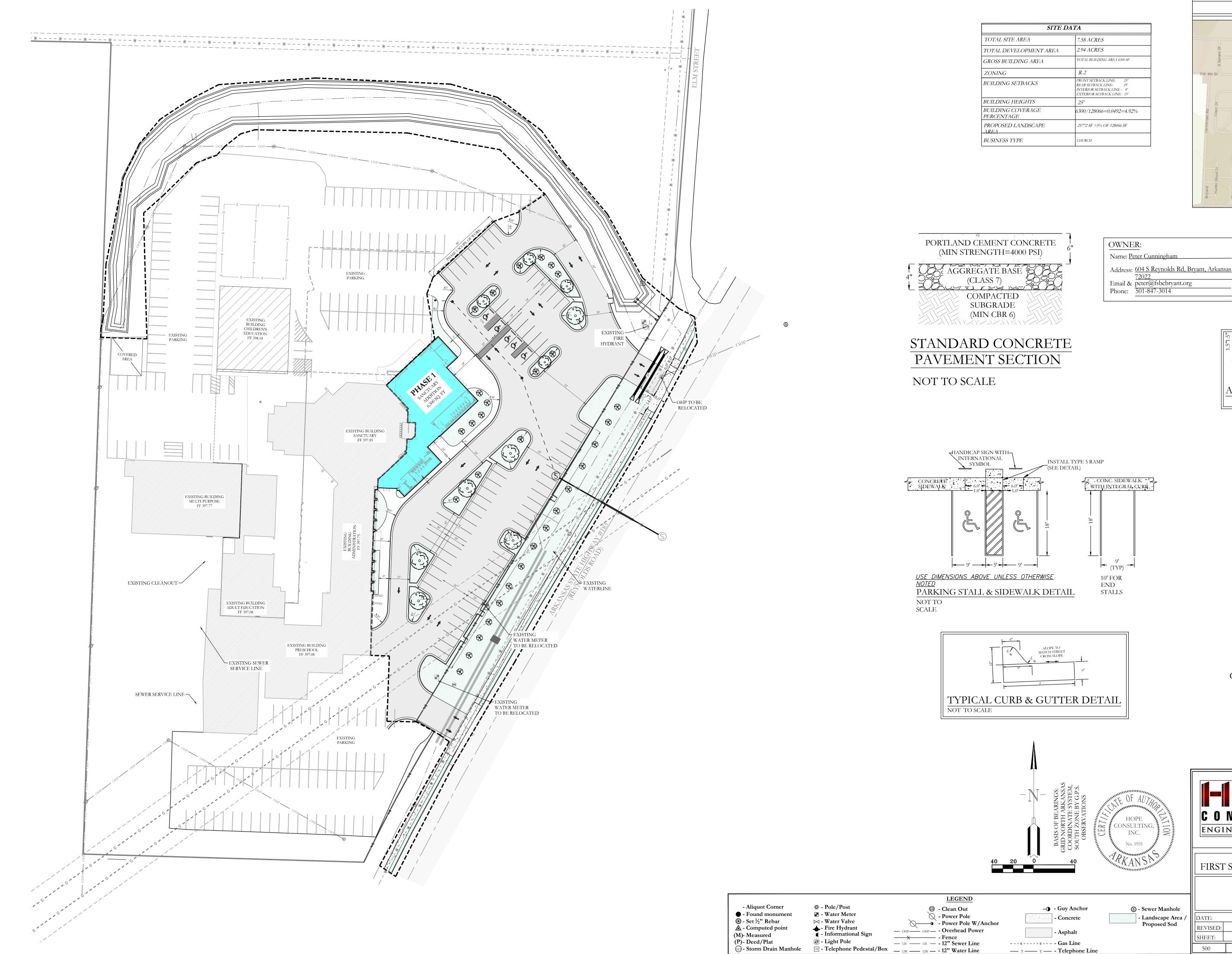


NO PORTION OF THE PROPERTY DESCRIBED HEREON LIES WITHIN A SPECIAL FLOOD HAZARD AREA, ACCORDING TO THE FEMA FLOOD INSURANCE RATE MAP LISTED BELOW: PANEL # 05125C0380E , DATED: 06/05/2020

FLOOD ZONE INFORMATION

🕀 - PLSS Aliquot Corner	070	- Cl
 Fnd. Corner Monument 		- W
⊙ - Set 1/2" Rebar/Cap (1664)	Ø	- Po
\triangle - Computed Point	\bigcirc	- Se
(M)- As Measured	ø	- Liş
(P) - Per Deed or Plat Records		- Te
ESMT - Easement	D	- D1
B.S.L Building Setback Lines	c۷	- Gé

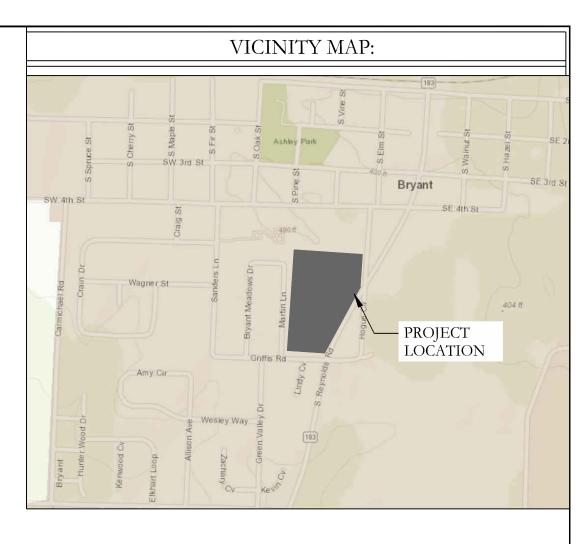


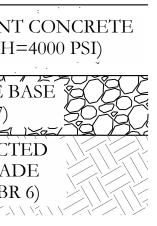


TB - Telephone Pedestal/Box

— 12W — 12W — - 12" Water Line

SITE L	DATA
AL SITE AREA	7.58 ACRES
L DEVELOPMENT AREA	2.94 ACRES
S BUILDING AREA	TOTAL BUILDING AREA 6300 SF
ING	R-2
DING SETBACKS	FRONT SETBACK LINE: 25' REAR SETBACK LINE: 25' INTERIOR SETBACK LINE: 8' EXTERIOR SETBACK LINE: 15'
DING HEIGHTS	25'
DING COVERAGE ENTAGE	6300/128066=0.0492=4.92%
POSED LANDSCAPE	25772 SF >5% OF 128066 SF
NESS TYPE	CHURCH





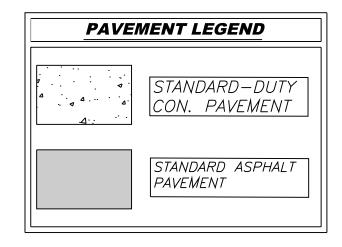
DEVELOPER: Name: <u>Peter Cunningham</u>

Address: <u>604 S Reynolds Rd, Bryant, Arkansas</u> 72022 Email & peter@fsbcbryant.org

Phone: 501-847-3014

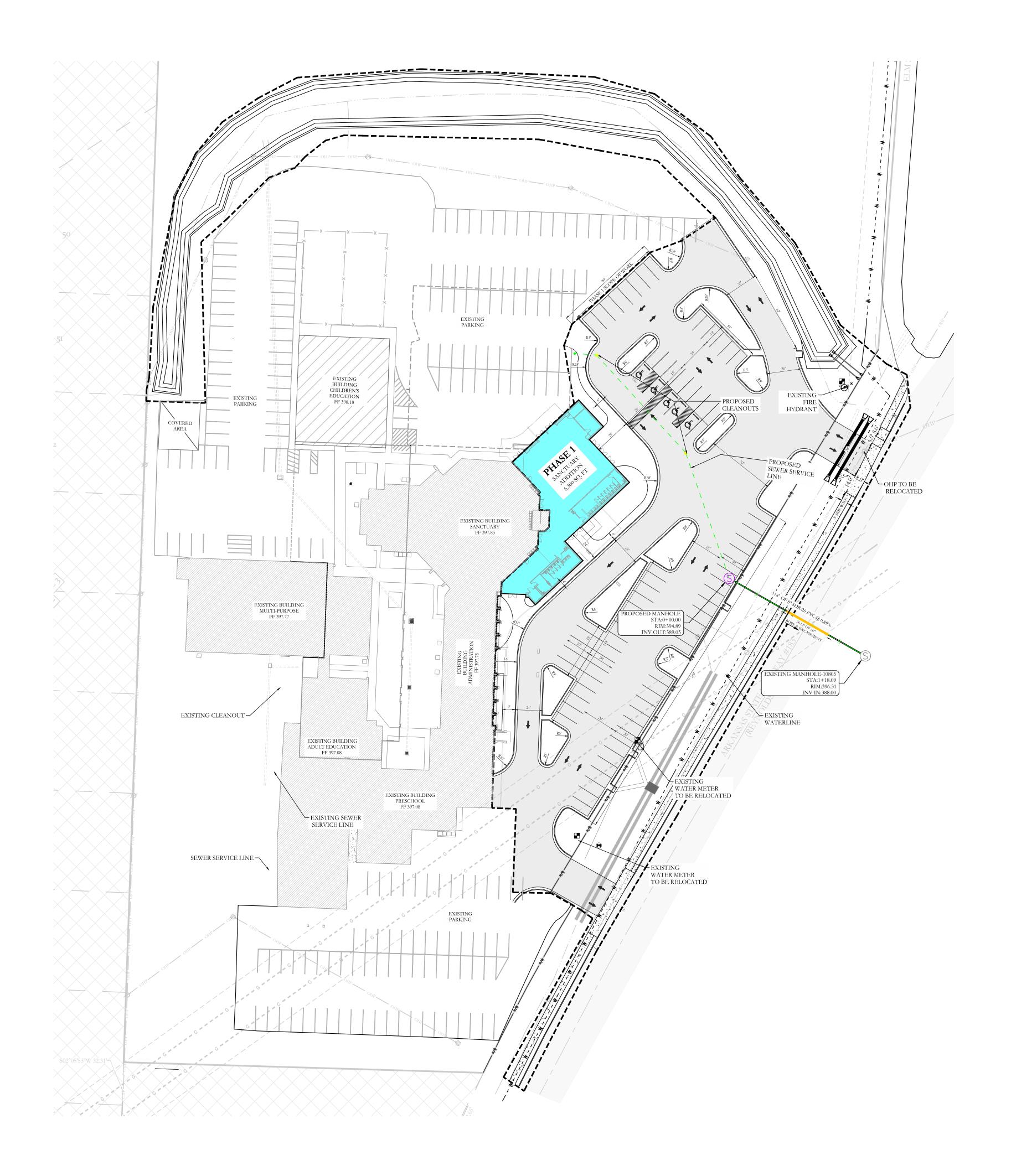


ASPHALT PAVEMENT SECTION NOT TO SCALE

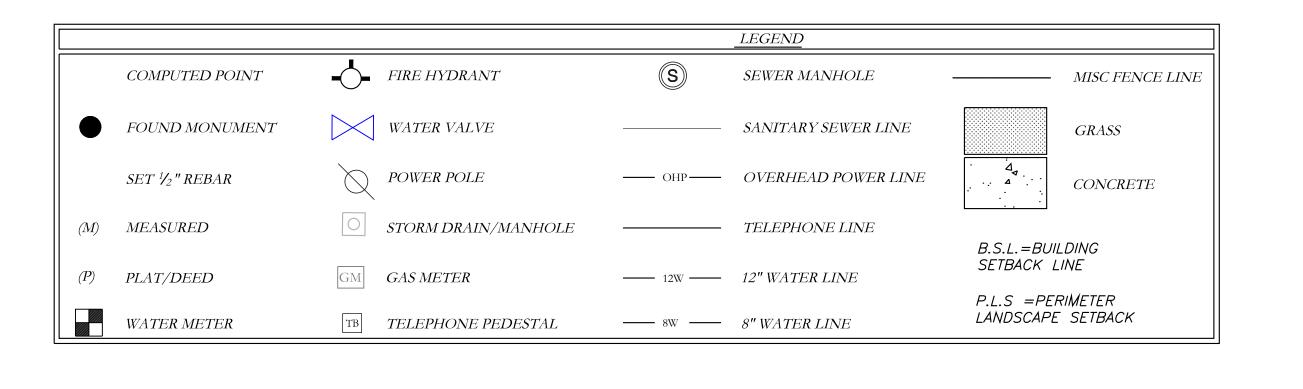


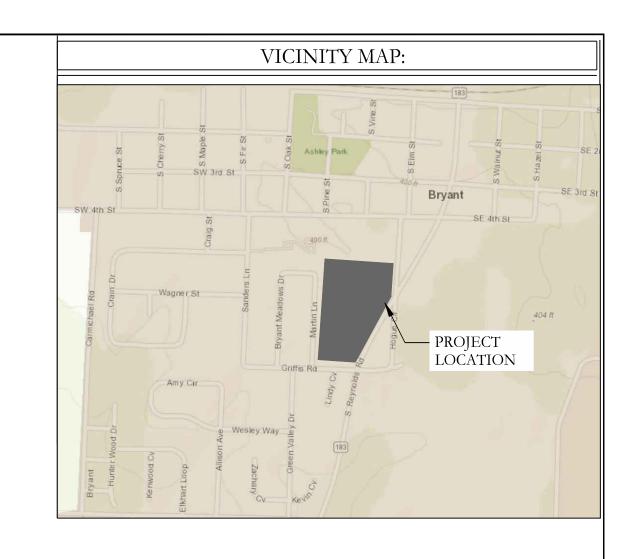
CIVIL ENGINEER HOPE CONSULTING INC 129 N. MAIN STREET BENTON, AR 72015 CONTACT: KAZI TAMZIDUL ISLAM PHONE: 501-315-2626 EMAIL: kazi@hopeconsulting.com

BASIS OF BEARINGS: BASIS OF BEARINGS: GRID NORTH ARKANSAS COORDINATE SYSTEM, SOUTH ZONE BY G.P.S. OBSERVATIONS OBSERVATIONS Nor 155 Nor 155 No		- IO DNSU GINEERS -	P L T I SURVE	N	G	PH. (50 FAX (50	Arkans)1)315-2)1) 315-	as 72015 2626
20 0 40	SA FIR	ST SOUTH		SE AND			CH OF	BRYANT
		CHU	JRCH E)N PHA	SE 1	
–) - Guy Anchor (3) -	- Sewer Manhole	P		SITE PI 5 REYN INE CO	OLDS		8	
	- Landscape Area / DATE: Proposed Sod	09-04-2024	C.A.D	.BY: F	B.JOHN	ISON	DRAWING	G NUMBER:
- Asphalt	REVISE SHEET:	D: 09-25-2024 C-1.0	CHEC SCAL	CKED B E:	Y:		24	-0260
¢¢ Gas Line — T — T — T - Telephone Line	500	01S	14W		34	310	62	1664



NOTE:

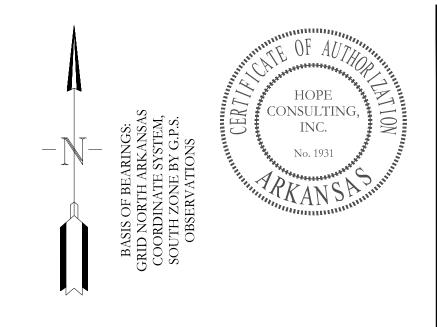




OWNER:	DEVELOPER:
Name: <u>Peter Cunningham</u>	Name: <u>Peter Cunningham</u>
Address: <u>604 S Reynolds Rd, Bryant, Arkansas</u> 72022	Address: <u>604 S Reynolds Rd, Bryant, Arkans</u> as 72022
Email & peter@fsbcbryant.org	Email & peter@fsbcbryant.org
Phone: 501-847-3014	Phone: 501-847-3014

FIRE ALARM , EXISTING & PROPOSED FIRE SEPARATION DETAILS WILL BE PROVIDED AT THE TIME OF BUILDING PERMIT APPLICATION

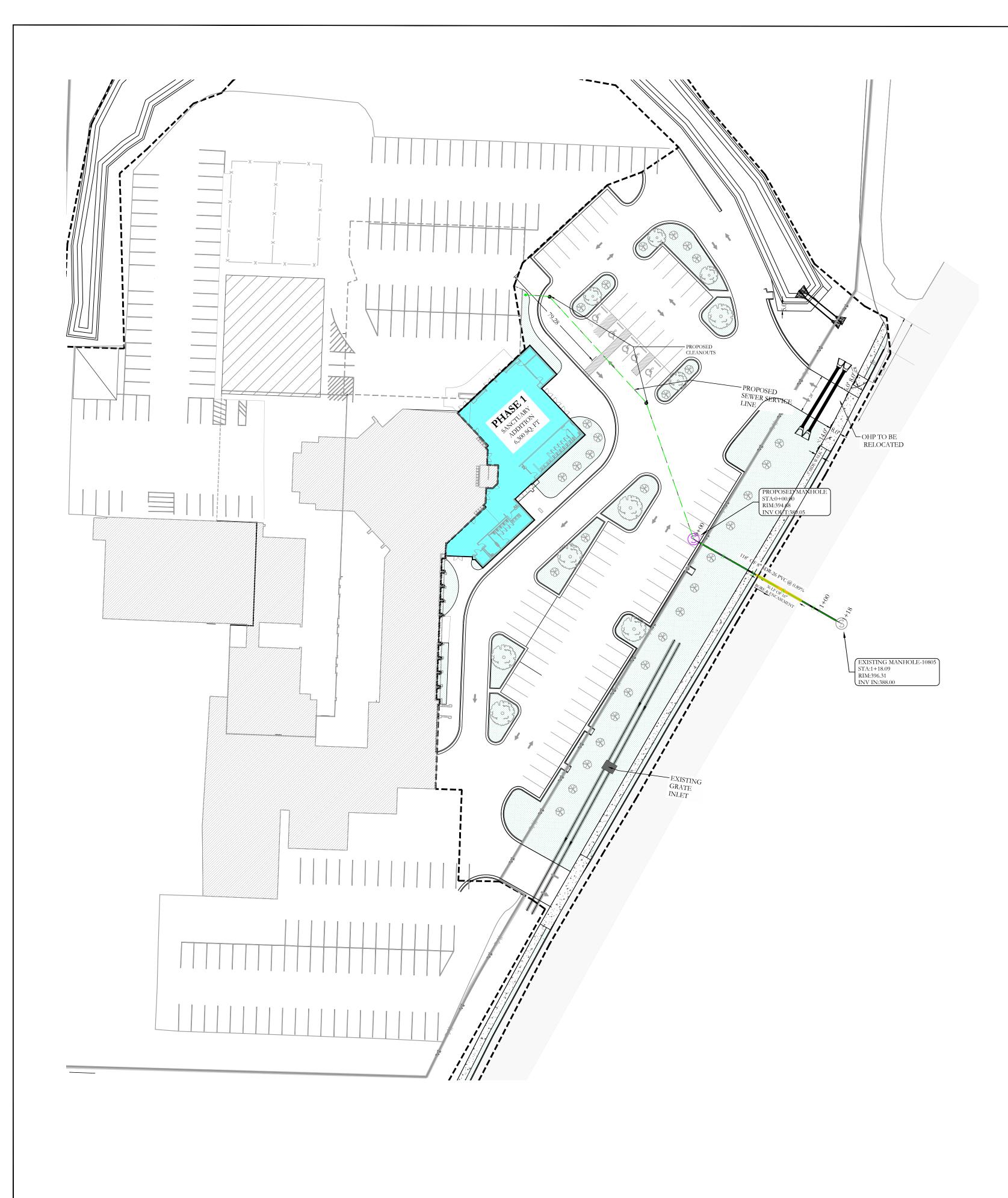
> CIVIL ENGINEER HOPE CONSULTING INC 129 N. MAIN STREET BENTON, AR 72015 CONTACT: KAZI TAMZIDUL ISLAM PHONE: 501-315-2626 EMAIL: kazi@hopeconsulting.com

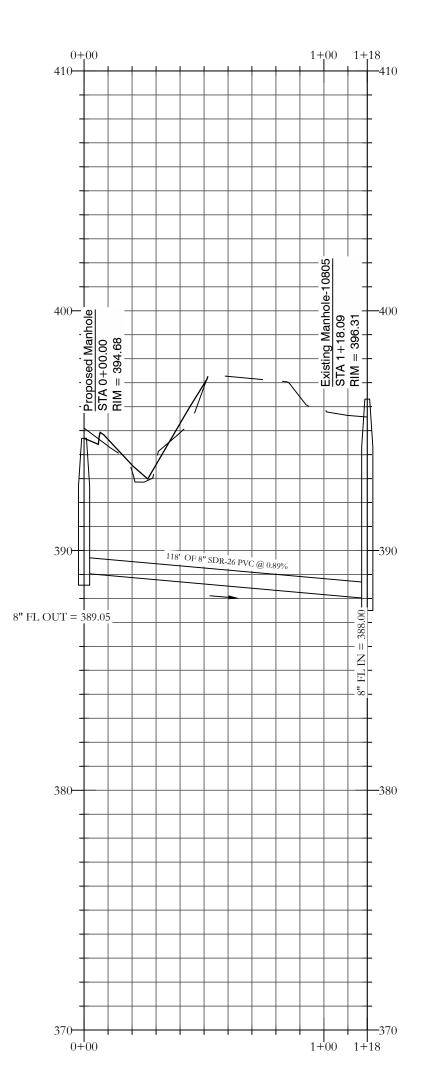


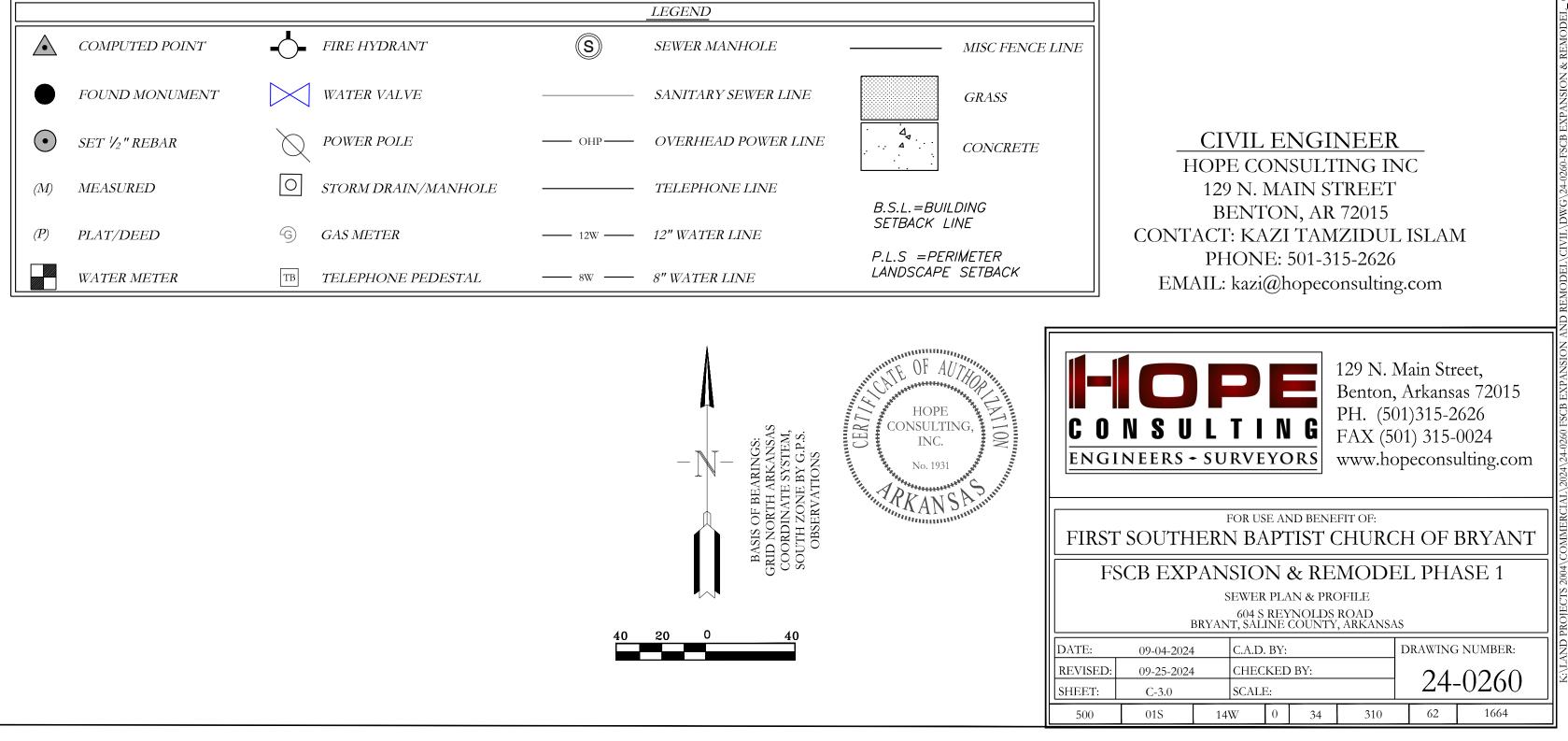
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EN	G	INE	EER	<u>s</u> -	SU	JRV	ΕY	OF	RS	WV

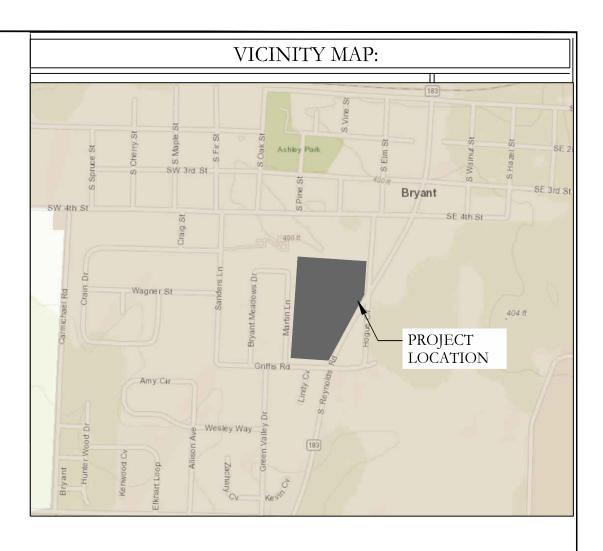
29 N. Main Street, enton, Arkansas 72015 H. (501)315-2626 AX (501) 315-0024 ww.hopeconsulting.com

FOR USE AND BENEFIT OF: FIRST SOUTHERN BAPTIST CHURCH OF BRYANT							
CHURCH EXPANSION PHASE 1							
	UTILITY PLAN						
	604 S REYNOLDS ROAD BRYANT, SALINE COUNTY, ARKANSAS						
DATE:	09-04-2024	- C	A.D. BY:	B.JOH1	NSON	DRAWING	G NUMBER:
REVISED:	09-25-2024	. C	HECKEI	OBY:		21	0260
SHEET:	C-2.0	S	CALE:				-0260
500	01S	14W	0	34	310	62	1664

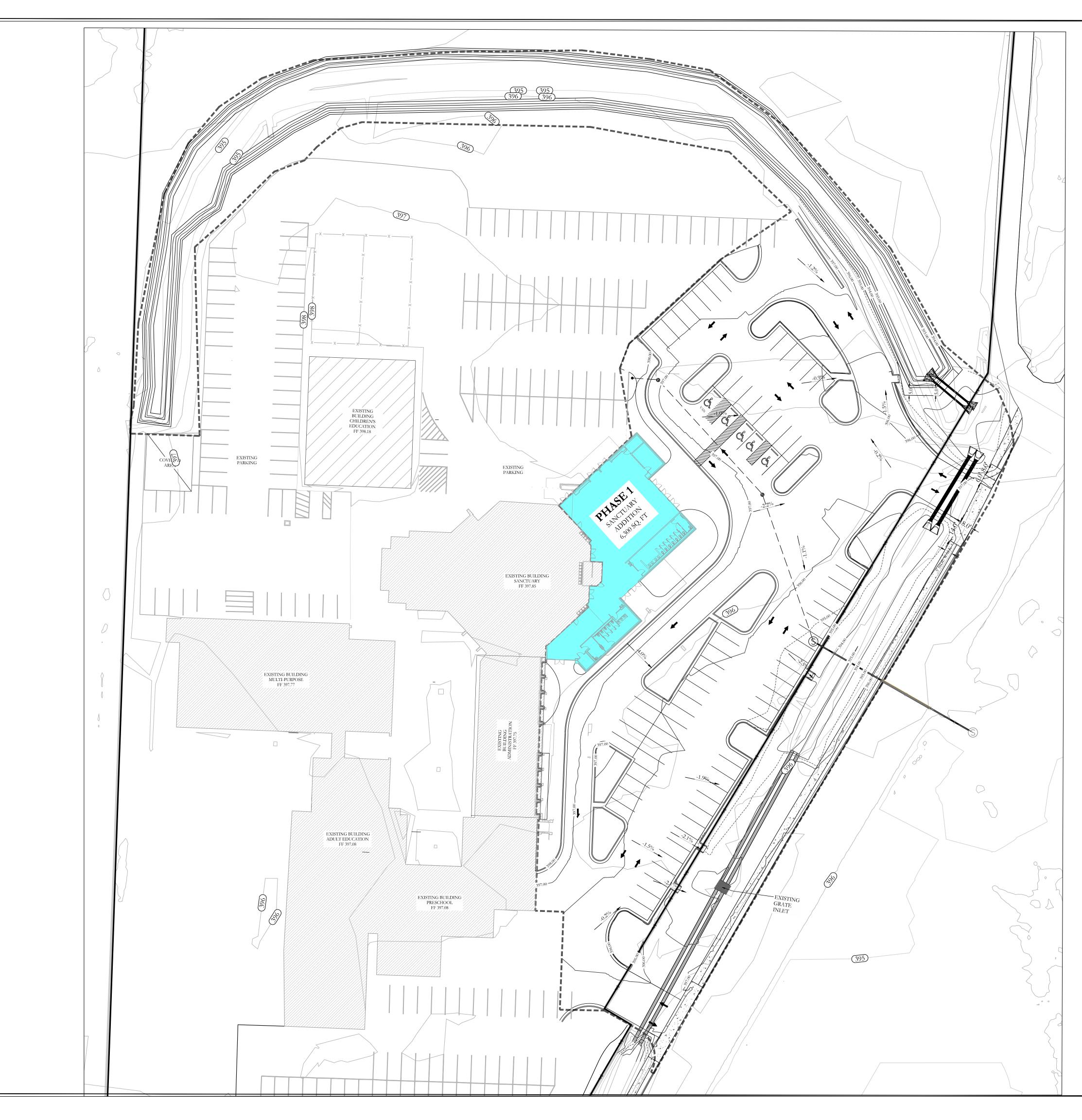






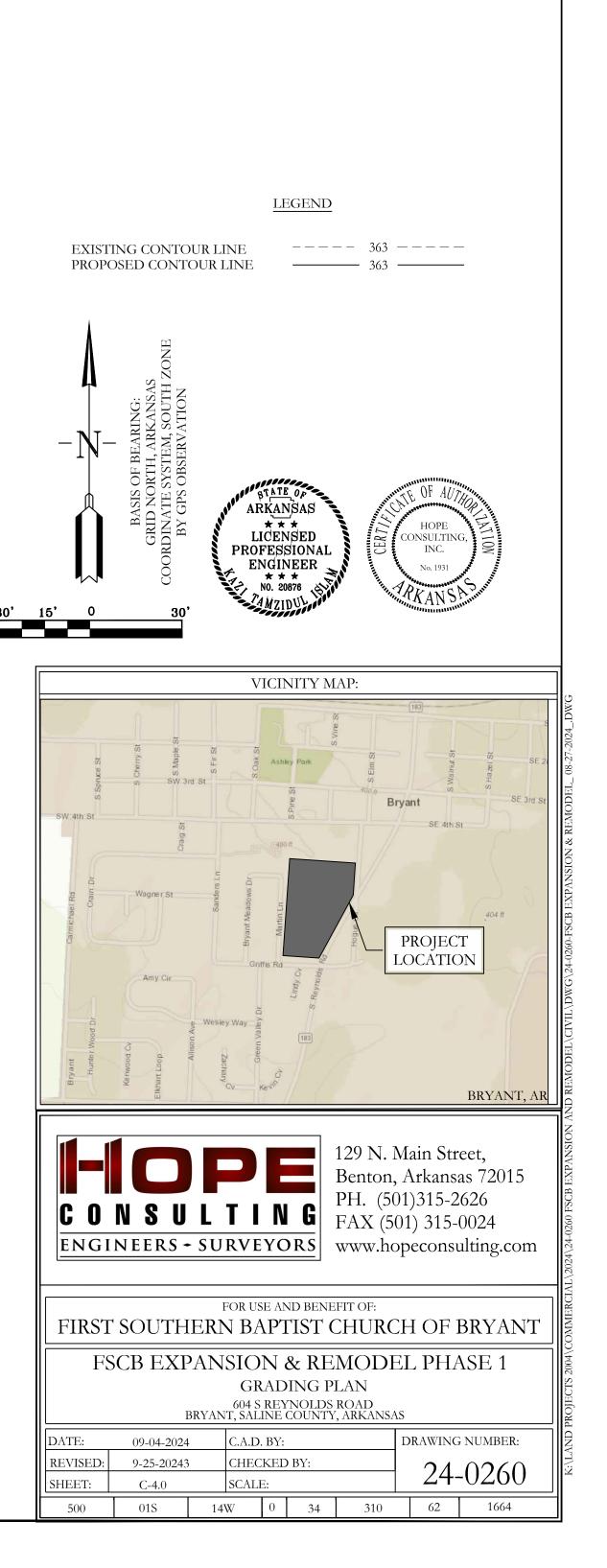


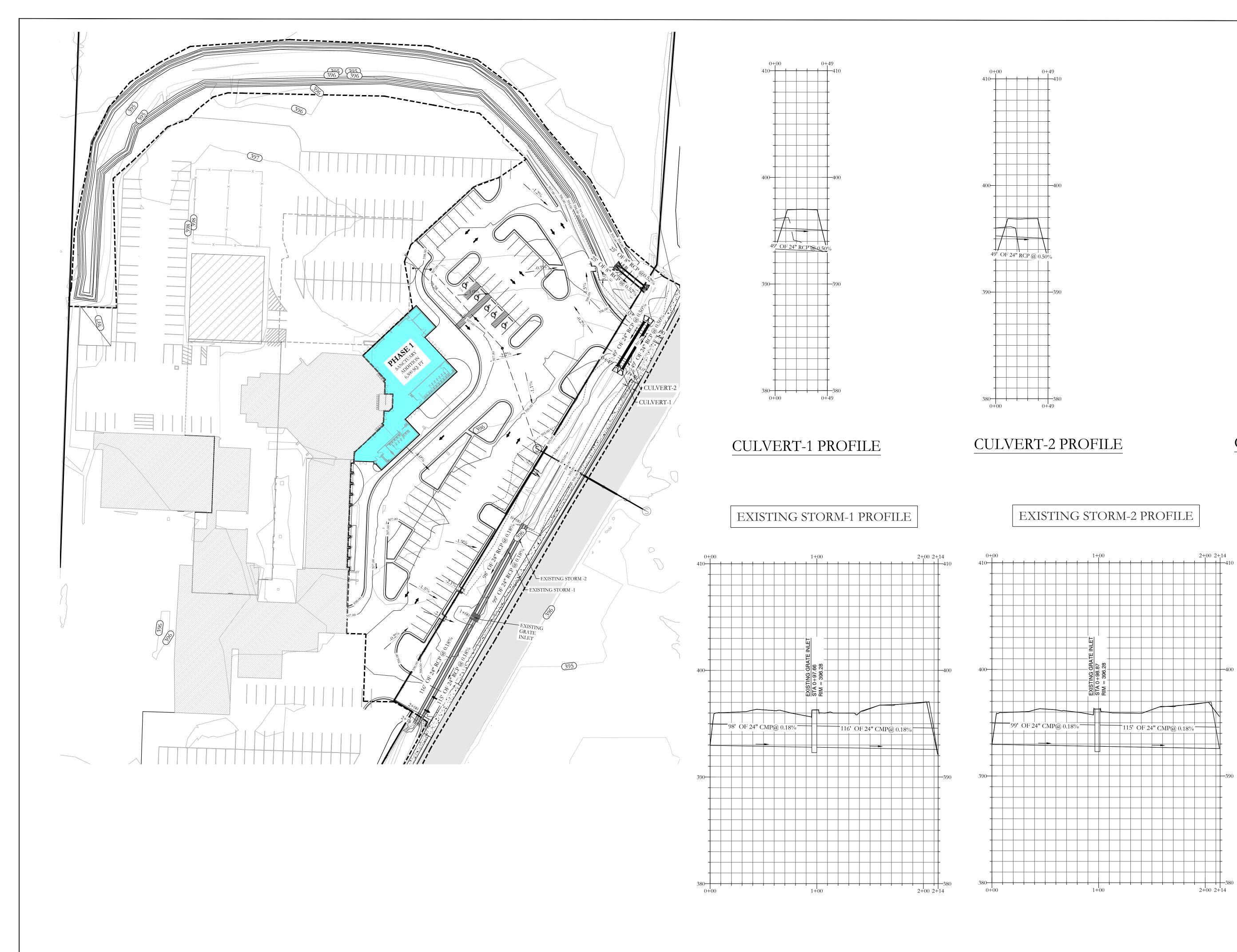
OWNER:	DEVELOPER:
Name: <u>Peter Cunningham</u>	Name: <u>Peter Cunningham</u>
Address:604 S Reynolds Rd, Bryant, Arkansas 72022Email & Phone:peter@fsbcbryant.org 501-847-3014	Address:604 S Reynolds Rd, Bryant, Arkansas72022Email &Phone:501-847-3014

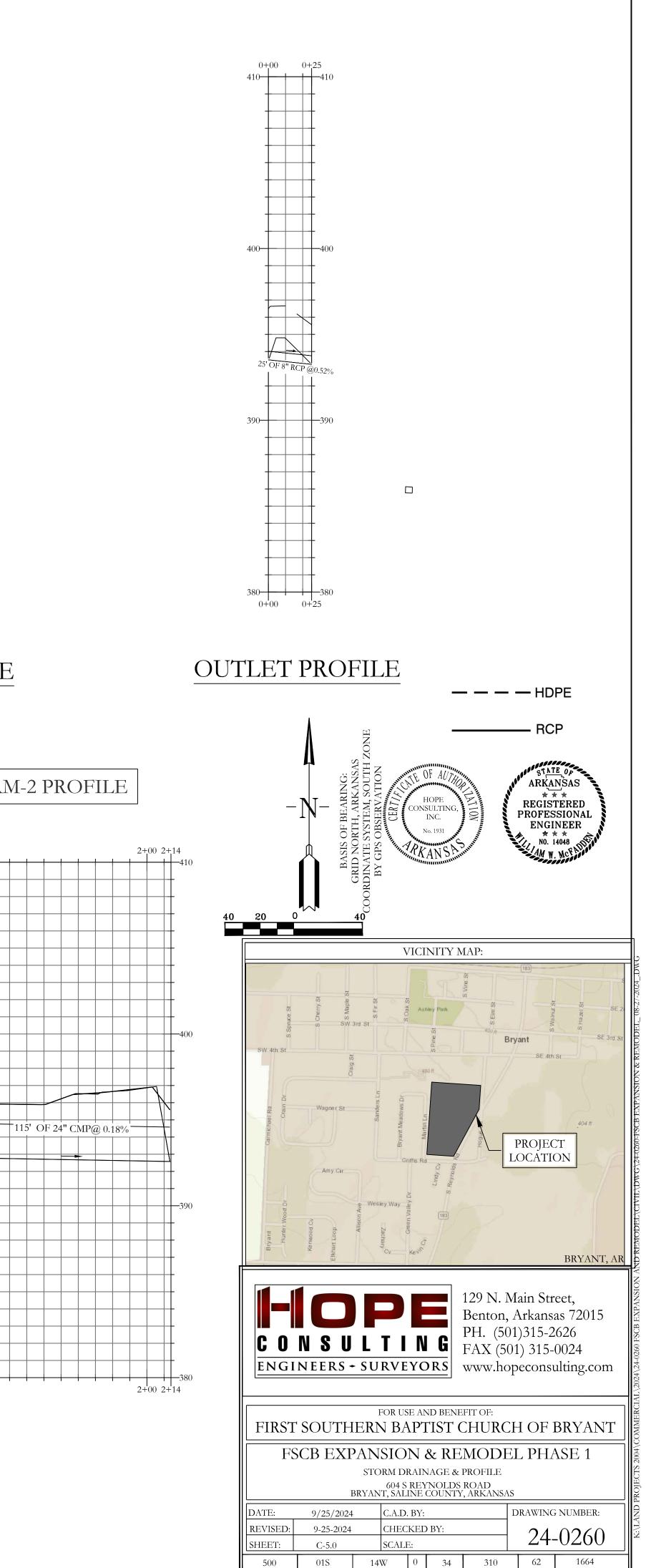


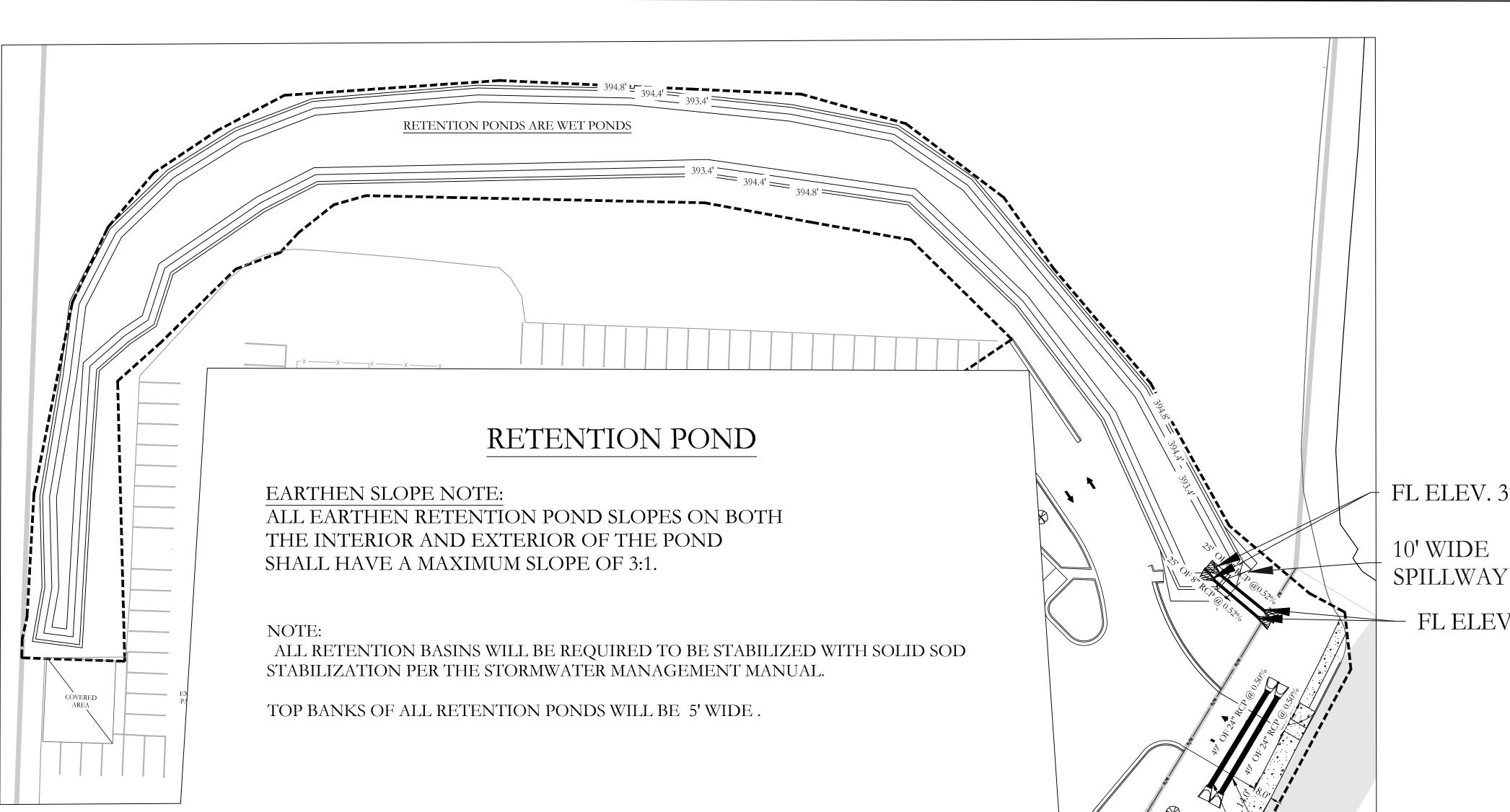


- 1. DESIGN CONTOURS SHOWN ARE FINISHED GRADE.
- 2. SPOT ELEVATIONS SHOWN ARE FINISHED ASPHALT, GROUND OR CONCRETE ELEVATIONS.
- 3. CLEAR AND GRUB AREAS OF THE SITE WHERE CUT OR FILL IS TO OCCUR.
- 4. FILL SHALL BE COMPACTED TO AT LEAST 98% OF THE MATERIAL'S MAXIMUM STANDARD PROCTOR DRY DENSITY.
- 5. THE MOISTURE CONTENT OF FILL MATERIAL SHALL BE WITHIN THE RANGE OF 1% BELOW TO 3% ABOVE THE OPTIMUM MOISTURE CONTENT.
- 6. SUBGRADES SHALL BE PROOF-ROLLED WITH A LOADED DUMP TRUCK TO DETECT ZONES OF UNSUITABLE AND/OR EXCESSIVELY WET SOILS. IF PUMPING BEGINS, COMPACTION SHALL BE STOPPED IMMEDIATELY AND RESUMED ONLY WHEN THE MATERIAL IS SUFFICIENTLY DRY THAT PUMPING DOES NOT OCCUR.
- 7. ALL UNUSABLE SOILS SHALL BE USED ON SITE FOR FILL PURPOSES OUTSIDE THE AREAS OF BUILDING AND PAVEMENT CONSTRUCTION.









DETENTION POND MAINTENANCE PLAN

Background

The Retention ponds are located on the periphery of the subdivision. They are designed to temporarily detain stormwater to meet -Re-growth of trees on or around the pond bank. These should be cut and removed from the pond area. water quantity criteria before discharging off the property.

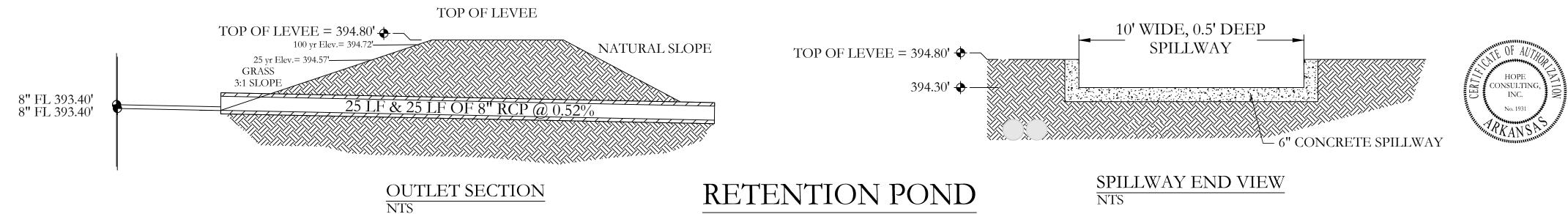
Routine Maintenance:

-Sediment from the site may accumulate in the pond bottom and reduce the pond to below design volume requirements. The The property owners association will maintain the drainage easements . Routine maintenance will include but not be limited to: pond should be excavated if the pond bottom elevation reached a level that allows excessive aquatic growth or reduces the -Mowing of the bank slopes and area around the pond on a monthly basis during the growing season and as needed during the pond efficiency such, that the sediments are passing the discharge structure and release off site. cooler months.

-The outlet pipe from the pond and other areas will be inspected monthly for debris which could inhibit the proper flow of discharge. Any debris will be removed immediately and disposed of or placed in a location to prevent future maintenance and to not cause impact up or downstream of the structure.

-Trash will be removed from around the pond to prevent entering the pond. Generally, the site should be kept free of loose trash which could be carried off site by wind or rain.

-Inspect the pond and outlet pipe for non-routine maintenance need.

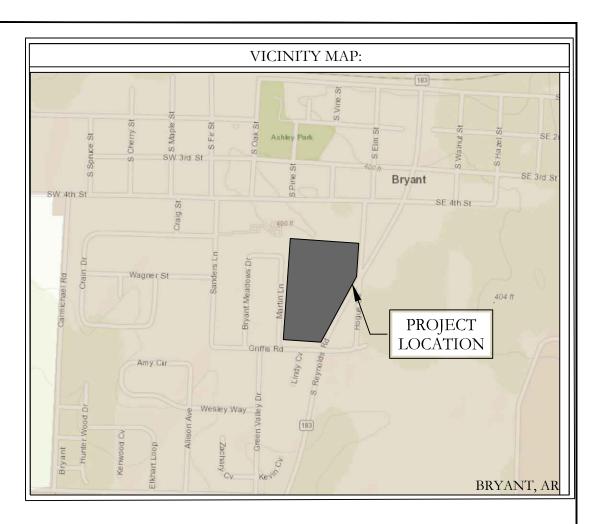


Periodic or Non-Routine Maintenance

The routine inspection of the ponds areas and discharge pipes will identify needed repairs and non-routine maintenance. These items may include but not be limited to:

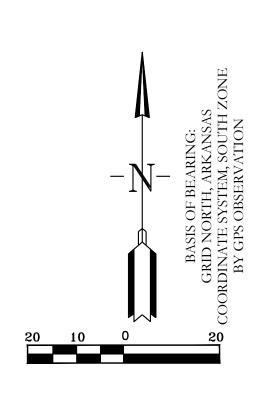
-Stabilization or re-grading of side slopes may be required periodically or after excessive rain events. Any disturbance of slopes should be reseeded or may require installation of erosion control materials until seeding can reestablish adequate grasses to prevent future erosion.

-Any other maintenance or repairs which would minimize other maintenance to the pond or outfall structures.



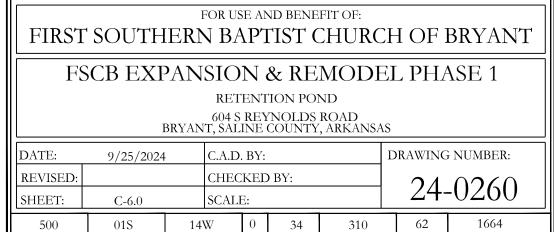
FL ELEV. 393.4'

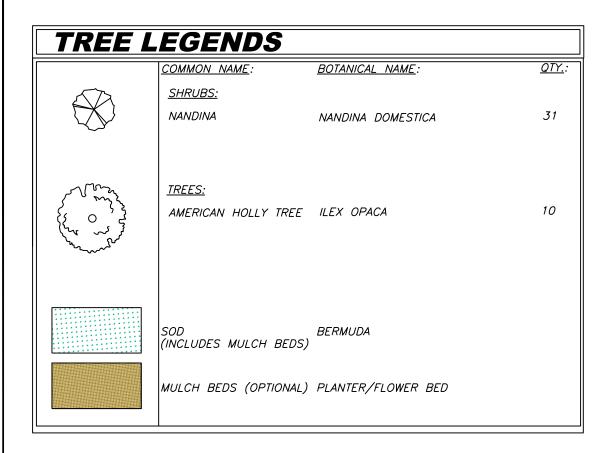
FL ELEV. 393.27'





129 N. Main Street, Benton, Arkansas 72015





CITY PLATING REQUIREMENTS:

SECTION IV: MINIMUM LANDSCAPING CRITERIA

	Residential Subdivision	C-1	C-2	PUD
Trees	N/A	1 each 1/3 acre or Fraction	1 each ½ acre or Fraction	X*
Evergreens	N/A	1/ 2,000 Sq. Ft.	1/ 2,000 Sq. Ft.	X*
Bedding Plants or Ground Cover in Containment	Primary Entrance must be Landscaped	100 Sq. Ft. Minimum	100 Sq. Ft. Minimum	X*
Lawn (Grass)	N/A	Options	Options	X*
Open Space Natural or Landscaping	100 Sq. Ft./Lot	N/A	N/A	X*

* Landscape design must be approved

- No Planting within 5 feet of a fire hydrant.
- Spacing will be 40' between trees.
- Tree must be a minimum 3" in diameter @ the base and 12'+ tall.
- Existing trees meeting the minimum size can be counted to meet the criteria.
- No trees can be planted within thirty-foot (30') of a property comer or driveway.
- Shrubs along street fight-of-way lines cannot exceed thirty inches (30") in height.
- Separations noted in the zoning regulations must be bermed or screened with landscaping and ground cover or grass.

City of Bryant Landscaping Ordinance # 2000-07

Page 4 of 8

The following list of shrubs, are those which have been found to be best	
suited to this area and yet requiring the least amount of maintenance.	
This list, along with the secondary list, are those shrubs which may be	
planted in the required landscape area. Additional selective shrubs may	
be substituted when proven to be hearty in this region.	

1.	Primary List:	
	<u>Соммон Name</u> Evergreen Hollies Nandina	<u>Scientific Name</u> Ilex species Nandina domestica
2.		Secondary List:
	<u>Соммон Name</u> Abelia Boxwood Chinese Photinia Note: Secondary listed shrubs re	<u>Scientific Name</u> Abelia grandiflora Busus sempervirens Photinia serrulata quire increased maintenance
Grass	es	
1.	The following grasses may be us	ed to comply with this ordinance:
	Mayer Z-52 Bermuda Grass Centipede Fescue	Zoysia Bermuda Grass hybrids St. Augustine
2.	The Grasses listed in subsection used grasses adjacent to vehicu	(1) above are the more commonly lar use areas.
Grour	nd Covers	
1.	The following primary list of gro for use to comply with this o	
	<u>Соммон Name</u> DwarfNandina Junipers Liriope Memorial Rose	<u>SCIENTIFIC NAME</u> N. domestica "Harbour Dwarf" Juniperus species Liriope Muscari Rosa Wichuraiana

Ophiopagon japonicus

E. fortunei "Radicans"

Page 6 of 8

Vinca minor

Mondo Grass

Spreading Euonymus

Periwinkle

City of Bryant Landscaping Ordinance # 2000-07

The following list of shrubs, are those which have been found to be best suited to this area and yet requiring the least amount of maintenance. This list, along with the secondary list, are those shrubs which may be planted in the required landscape area. Additional selective shrubs may be substituted when proven to be hearty in this region.

1.	Primary List:	
	<u>Соммон Name</u> Evergreen Hollies Nandina	<u>SCIENTIFIC NAME</u> Ilex species Nandina domestica
2.		Secondary List:
	<u>Common Name</u> Abelia Boxwood Chinese Photinia Note: Secondary listed shrubs rea	<u>Scientific Name</u> Abelia grandiflora Busus sempervirens Photinia serrulata quire increased maintenance
Grasse	es	
1.	The following grasses may be use	ed to comply with this ordinance
	Mayer Z-52 Bermuda Grass Centipede Fescue	Zoysia Bermuda Grass hybrids St. Augustine
2.	The Grasses listed in subsection used grasses adjacent to vehicu	
Groun	d Covers	
1.	The following primary list of grou for use to comply with this or	
	<u>Соммон Name</u> DwarfNandina Junipers Liriope Memorial Rose Mondo Grass Periwinkle Spreading Euonymus	SCIENTIFIC NAME N. domestica "Harbour Dwarf" Juniperus species Liriope Muscari Rosa Wichuraiana Ophiopagon japonicus Vinca minor E. fortunei "Radicans"

City of Bryant Landscaping Ordinance # 2000-07

Secondary List: (This list can be used but must be confined to a bed.)

(
1
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1

SCIENTIFIC NAME Gelsemium sempervirens Arundinaria pygmaea Hedera Helix Lonicera sempervirens

SECTION VI MAINTENANCE

A. The developer, his successor and the property owner shall be responsible for regular weeding, irrigating, fertilizing, pruning and other maintenance of all planting on private property of a development. Plant materials which are installed for compliance with this ordinance, both on private property and the public right-of-way, which exhibit evidence of insect pests, disease and/or damage, shall be appropriately treated and dead plant materials shall be replaced.

- Β. The owner of land abutting a constructed public right-of-way shall be responsible for the tree planting strip lying between the private property line and the curbline or backslope line and shall be required to regularly weed, mow, prune and maintain plantings in compliance with good horticultural practices.
- C. If the owner of land thus situated as in (2) above, neglects or refuses to maintain the areas as proscribed, after having been given ten (10) days notice in writing to maintain by the City, the owner shall be guilty of a misdemeanor.

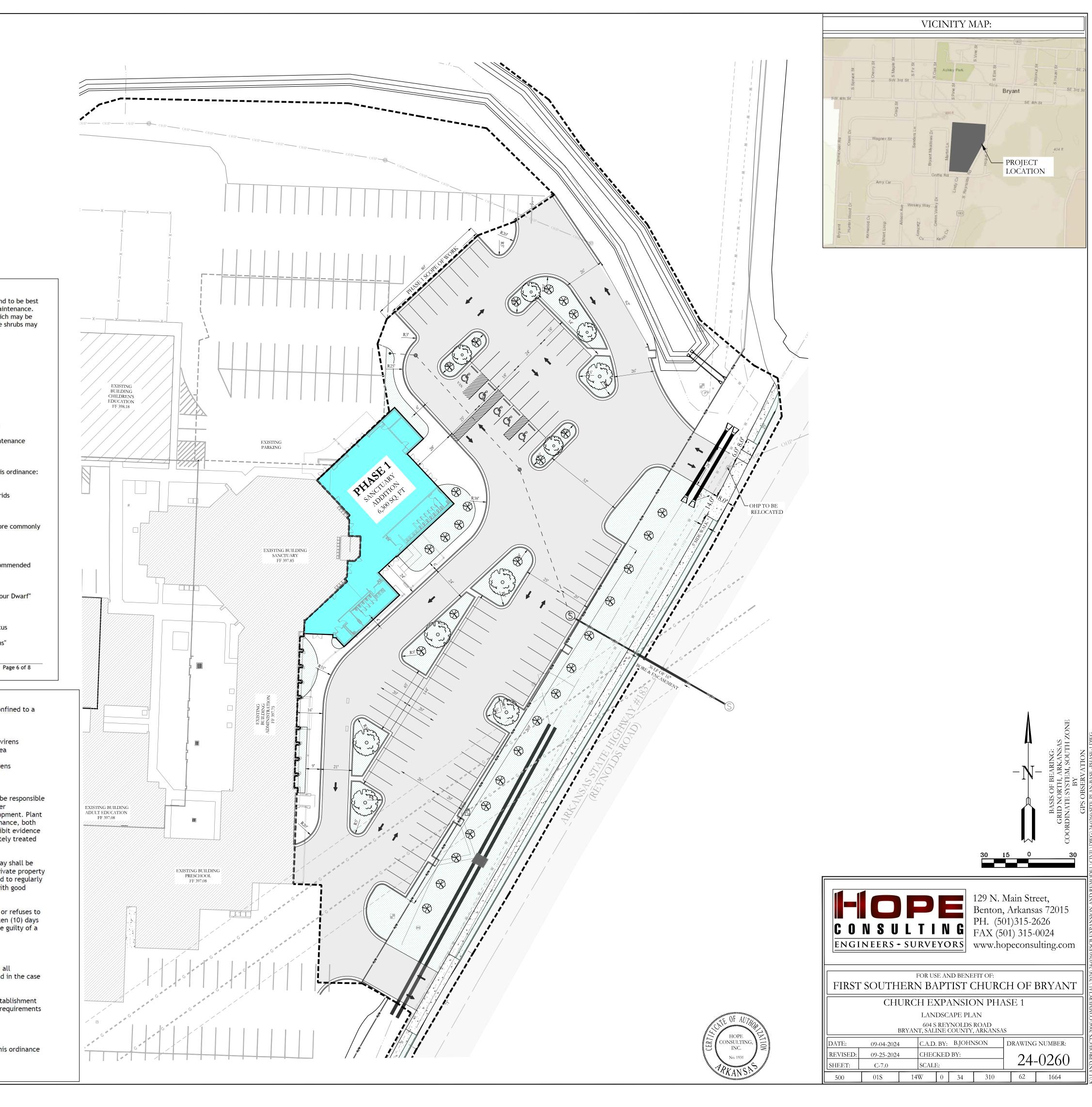
SECTION VII PLANNING COMMISSION APPROVAL

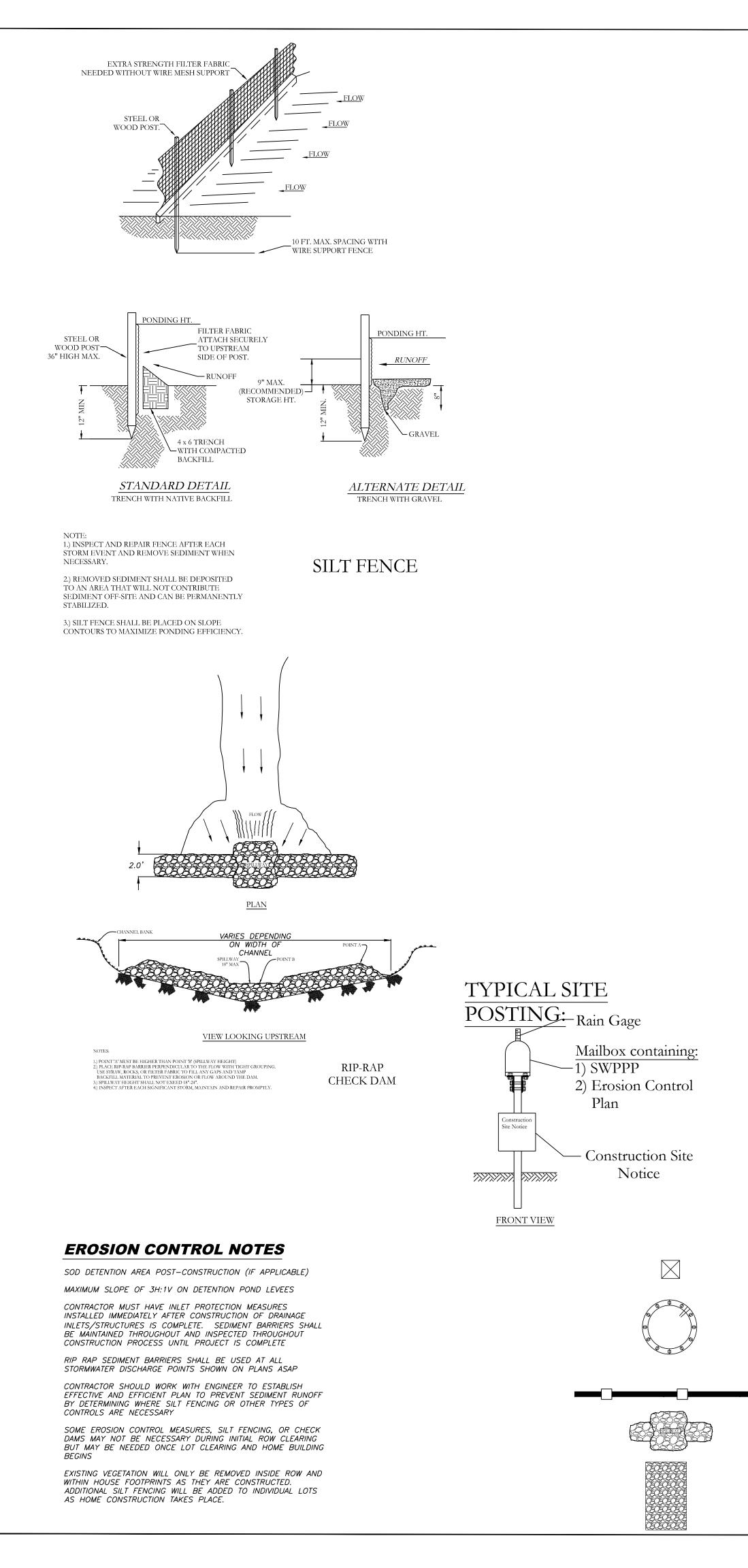
The City of Bryant Planning Commission will review and act on all landscaping proposals at the time building plans are submitted and in the case of subdivision at the preliminary plat submittal.

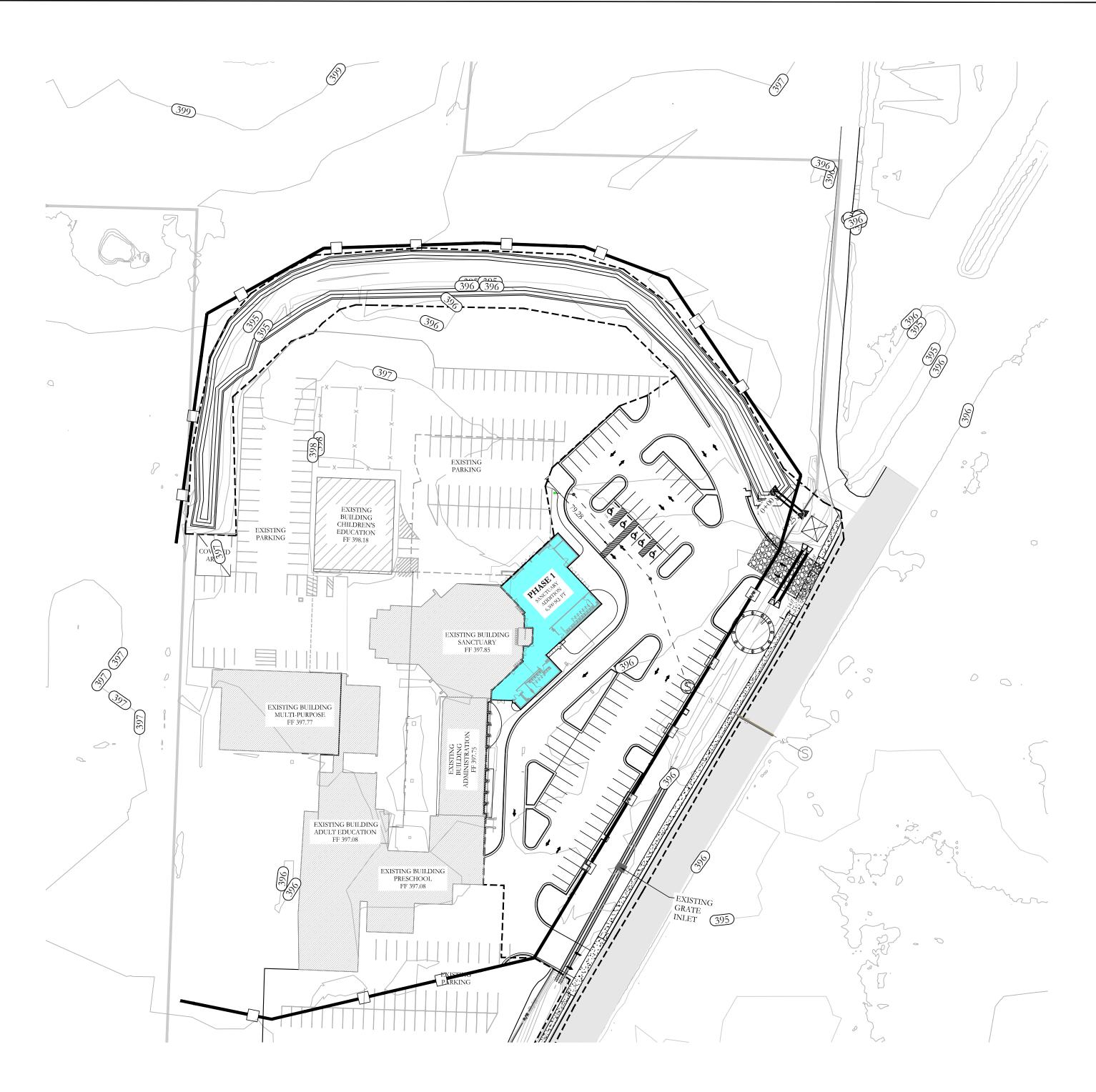
A certificate of occupancy will not be issued for a commercial establishment nor will the final subdivision plat be approved until landscaping requirements are satisfied.

SECTION VIII ENFORCEMENT

The code enforcement officer of the City of Bryant will enforce this ordinance and issue citations as authorized by law.







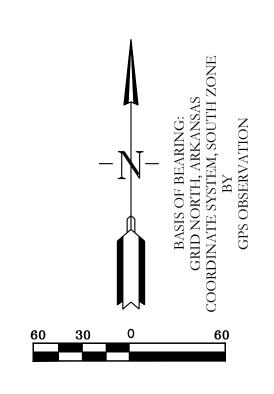
ERC LEGEND

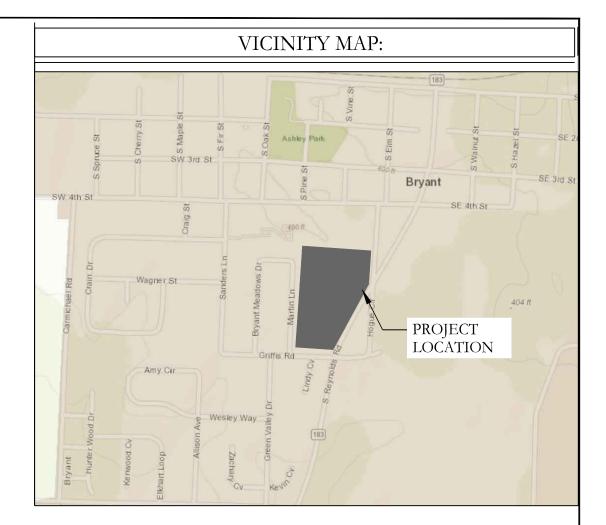
SITE POSTING/ RAIN GUAGE

CONC. WASHOUT DETENTION AREA

SILT FENCE RIP RAP CHECK DAM

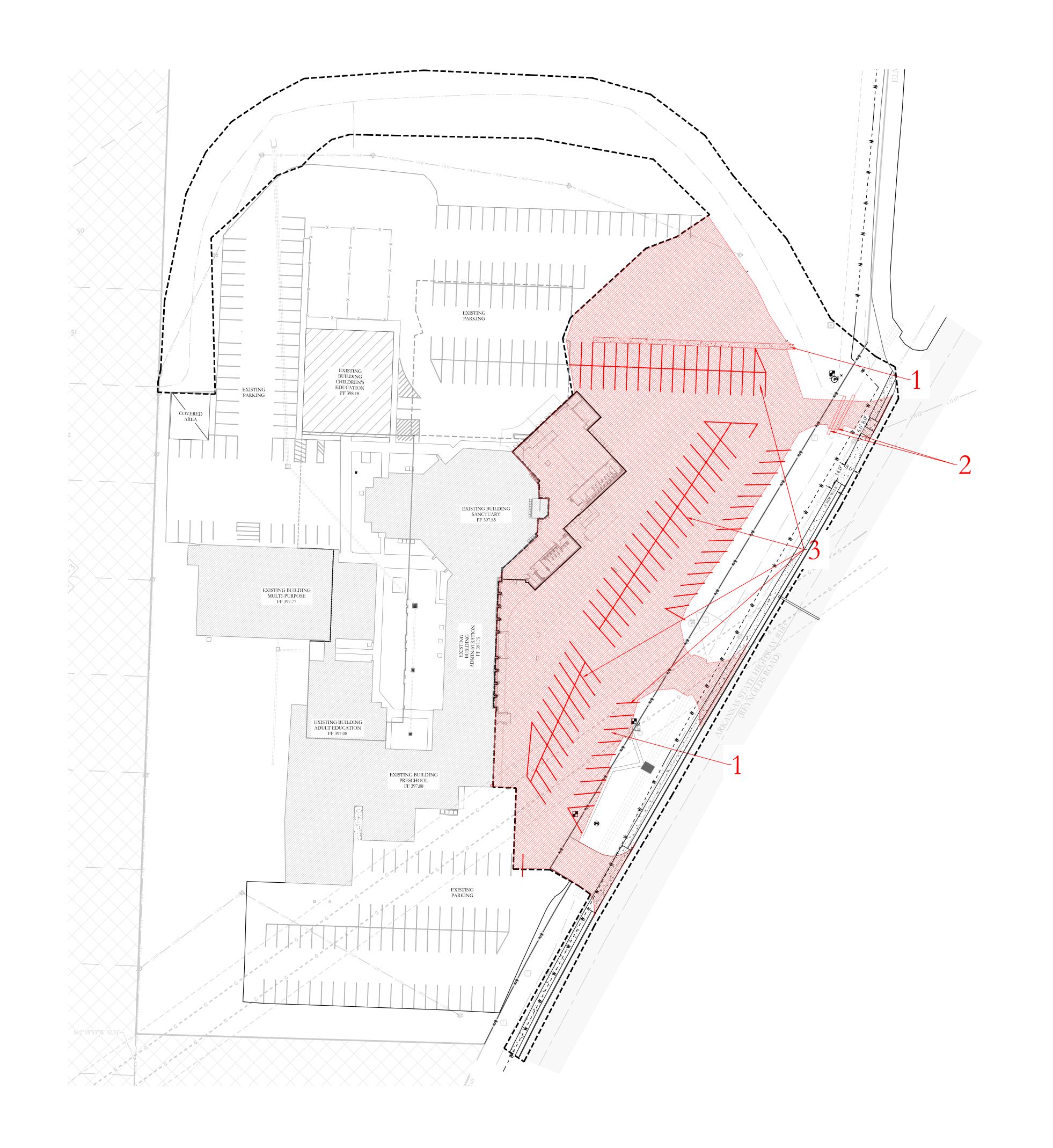
CONSTRUCTION ENTRANCE



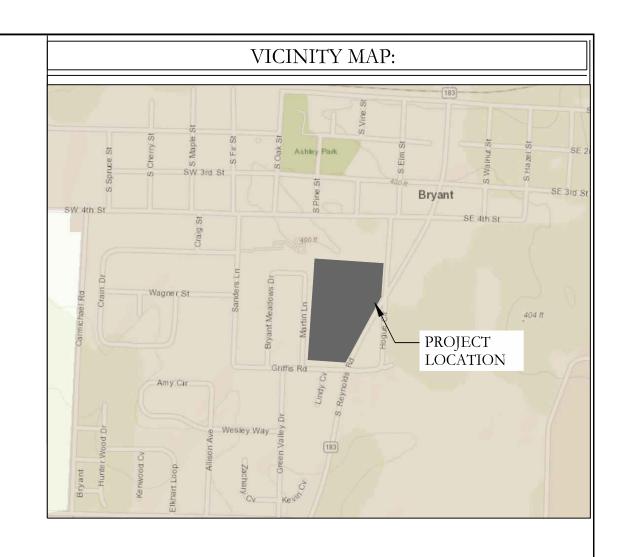




FOR USE AND BENEFIT OF:									
FIRST SOUTHERN BAPTIST CHURCH OF BRYANT									
FSCB EXPANSION & REMODEL PHASE 1									
	EROSION CONTROL PLAN								
	604 S REYNOLDS ROAD BRYANT, SALINE COUNTY, ARKANSAS								
DATE:	09-04-2024	09-04-2024					DRAWING NUMBER:		
REVISED:	9-25-2024		CHEC	KEL) BY:		21	-0260	
SHEET:	C-8.0		SCALE:				Z4·	-0200	
500	01S	14	W	0	34	310	62	1664	



DEMO PLAN NOTES



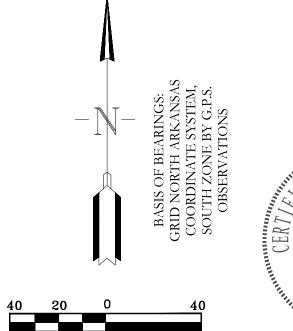
OWNER:	DEVELOPER:
Name: <u>Peter Cunningham</u>	Name: <u>Peter Cunningham</u>
Address: <u>604 S Reynolds Rd, Bryant, Arkansas</u> 72022	Address: <u>604 S Reynolds Rd, Bryant, Arkansas</u> 72022
Email & peter@fsbcbryant.org	Email & peter@fsbcbryant.org
Phone: 501-847-3014	Phone: 501-847-3014

1. DEMOLITION OF ASPHALT & CONCRETE AREA(70,260 SF)

2. DEMOLITION OF EXISTING CULVERTS

3. DEMOLITION OF EXISTING PARKING SPACE







	129 N. Main Street, Benton, Arkansas 72015
CONSULTING	PH. (501)315-2626
ENGINEERS + SURVEYORS	FAX (501) 315-0024 www.hopeconsulting.com

FIRST SOUTHERN BAPTIST CHURCH OF BRYANT								
	CHU	URC	ΗE	XP/	ANSIC)N PH/	ASE 1	
	DEMO PLAN							
604 S REYNOLDS ROAD BRYANT, SALINE COUNTY, ARKANSAS								
DATE:	09-04-2024	-	C.A.D	. BY:	B.JOHN	ISON	DRAWIN	IG NUMBER:
REVISED:	9-25-2024		CHEC	KEL) BY:		-2	
SHEET:	C-9.0		CHECKED BY: 24-0260					
500	01S	14	W	0	34	310	62	1664

City Comment Response Letter

First Southern Baptist Church - 604 Reynolds - Site Plan

Stormwater

1. Stormwater Drainage Calculations

Response: Drainage calculations have been provided. See drainage report.

2. Stormwater In-Lieu Fee

Response: Stormwater In-Lieu fee will be provided

Engineering

1. Submit drainage calculations

Response: Drainage calculations have been provided. See drainage report.

Com Dev

1. Provide Request letter for Site Plan.

Response: Request letter for site plan will be provided.

2. Provide Commercial site plan stormwater review fee \$250

Response: Review fee will be provided.

3. Sidewalk not shown along Reynolds

Response: Sidewalk has been shown along Reynolds Rd (see sheet C-1.0)

4. Is the existing monument sign going to be removed and placed back?

Response: Yes the sign will be removed and placed back.

1. Discuss Fire Alarm

Response: Fire design will be submitted during building permit application submission.

2. Discuss fire separation between new and existing.

Response: Fire separation details will be submitted during building permit application submission.

Fire

First Southern Baptist Church of Bryant 604 S REYNOLDS ROAD, BRYANT, AR 72022 DRAINAGE REPORT

FOR City of Bryant, Saline County, AR

September 2024

Owner & Developer: Peter Cunningham.

By:



TABLE OF CONTENTS

ITEM DESCRIPTION

- 1. Narrative & Summary
- 2. Hydrograph Report

Narrative & Summary

PROJECT TITLE

First Southern Baptist Church of Bryant

PROJECT PROPERTY OWNER

Peter Cunningham

PROJECT LOCATION

604 S Reynolds Road, Bryant, AR

PROJECT DESCRIPTION

The proposed development is on South Reynolds Road, Bryant, AR. Total development site area is 7.58 acres.

DRAINAGE ANALYSIS

On Site Drainage- Rational method was used to determine the existing and proposed flows from proposed site. There will be one retention pond to detain water from this development. Detailed drainage calculations considering the future expected development have been conducted to determine the required detention pond and culvert dimensions. Summary of the calculations are below:

Retention Pond

- Pond is situated on the north-east side of the property.
- Pre-development area 7.36 acres.
- Post-development area 7.34 acres.
- Pre-development runoff cumulative coefficient 0.65.
- Post-development runoff cumulative coefficient 0.72
- Pond has a bottom area of 16,570 sqft with bottom elevation of 393.4'.
- Two 8" RCPs with 0.52% slope is proposed for outflow pipes.

Peak flows for Pre and post development phase of onsite area have been tabulated below-

Period of	Pre-development	Post-dev. Without	Post-dev. With detention
time		detention	
	Peak Flow (cfs)	Peak Flow (cfs)	Peak Flow (cfs)
2-Year	18.69	22.67	1.911
5-Year	20.65	25.15	2.677
10-Year	24.35	29.23	4.569
25-Year	27.93	33.44	6.883
50-Year	31.84	38.07	9.645
100-Year	33.86	40.40	11.06

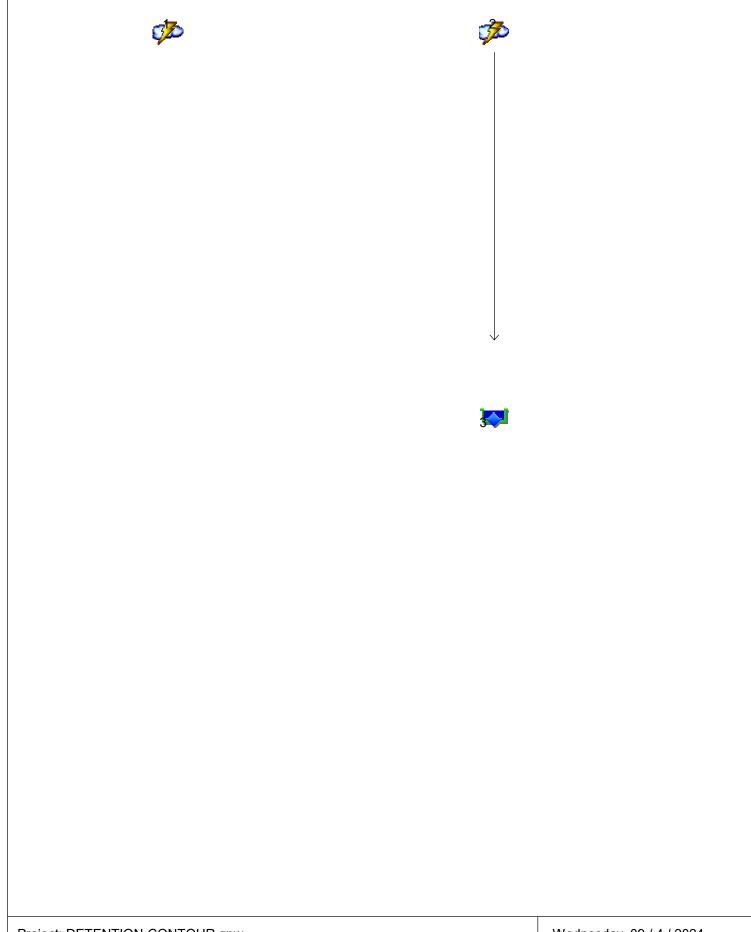
CONCLUSION

From the onsite drainage calculation, it is seen that there is decrease in flow for all storm events due to the proposed retention pond.

Hydrograph Summary Report

Watershed Model Schematic

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

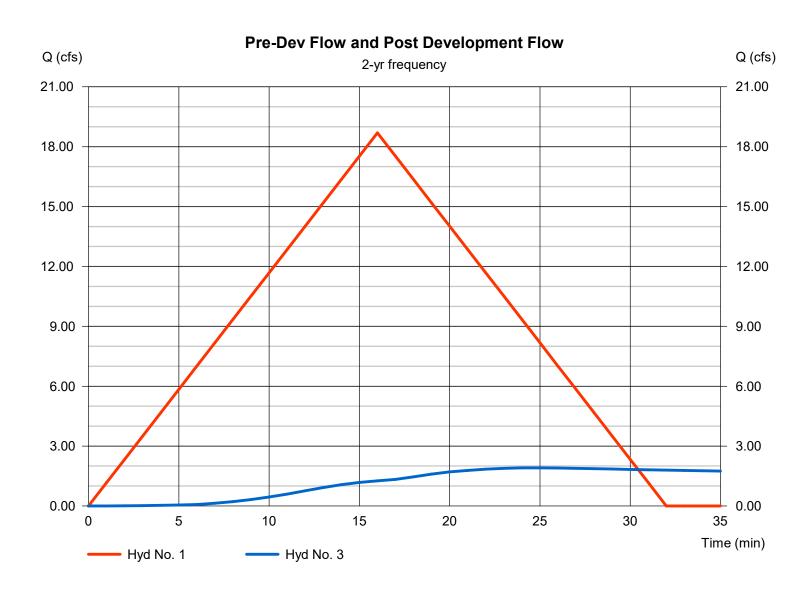
Hyd. No. 1

Pre-Dev Flow

Hydrograph type	= Rational
Peak discharge	= 18.69 cfs
Time to peak	= 16 min
Hyd. Volume	= 17,943 cuft

Hyd. No. 3

Hydrograph type	= Reservoir
Peak discharge	= 1.91 cfs
Time to peak	= 25 min
Hyd. Volume	= 17,652 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

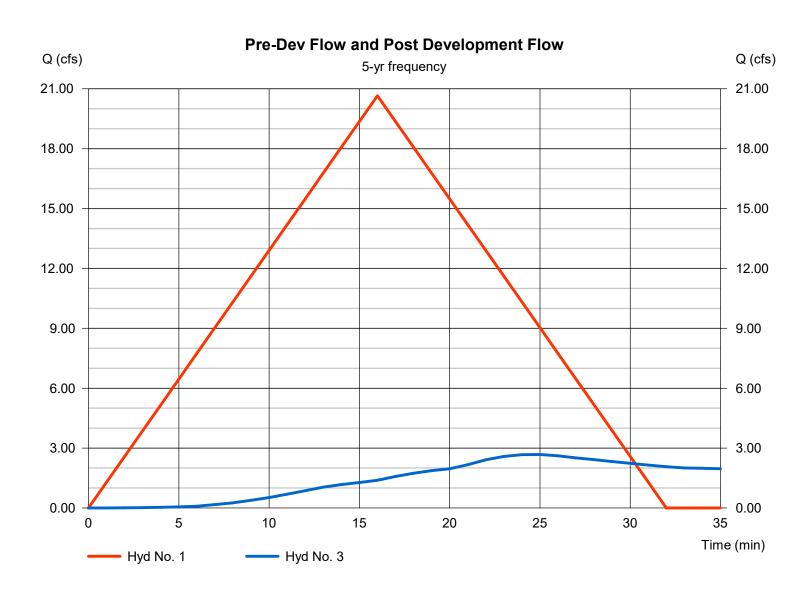
Hyd. No. 1

Pre-Dev Flow

Hydrograph type	= Rational
Peak discharge	= 20.65 cfs
Time to peak	= 16 min
Hyd. Volume	= 19,826 cuft

Hyd. No. 3

Hydrograph type	= Reservoir
Peak discharge	= 2.68 cfs
Time to peak	= 25 min
Hyd. Volume	= 19,588 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

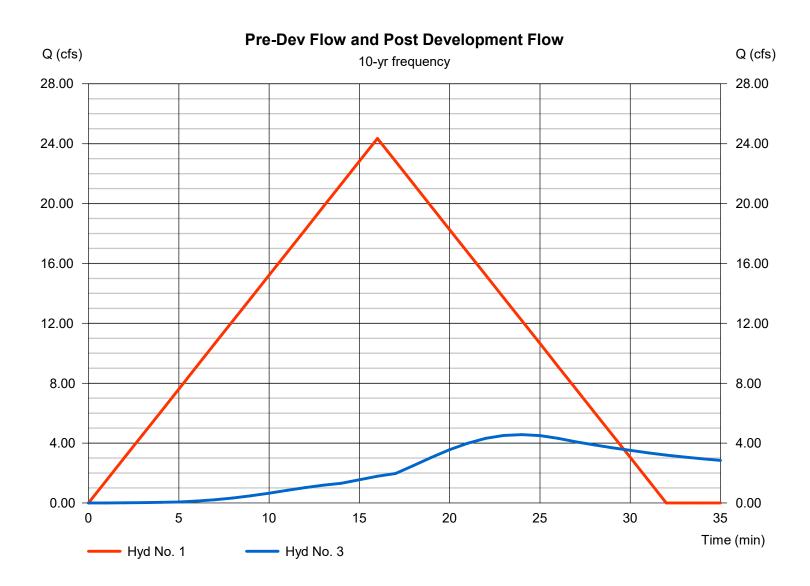
Hyd. No. 1

Pre-Dev Flow

Hydrograph type	= Rational
Peak discharge	= 24.35 cfs
Time to peak	= 16 min
Hyd. Volume	= 23,373 cuft

Hyd. No. 3

Hydrograph type	= Reservoir
Peak discharge	= 4.57 cfs
Time to peak	= 24 min
Hyd. Volume	= 22,771 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

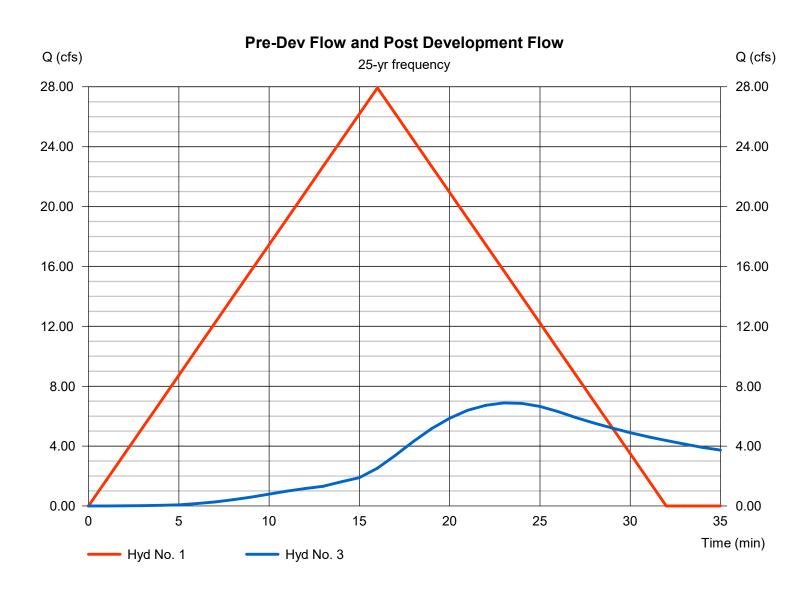
Hyd. No. 1

Pre-Dev Flow

Hydrograph type	= Rational
Peak discharge	= 27.93 cfs
Time to peak	= 16 min
Hyd. Volume	= 26,812 cuft

Hyd. No. 3

Hydrograph type	= Reservoir
Peak discharge	= 6.88 cfs
Time to peak	= 23 min
Hyd. Volume	= 26,060 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

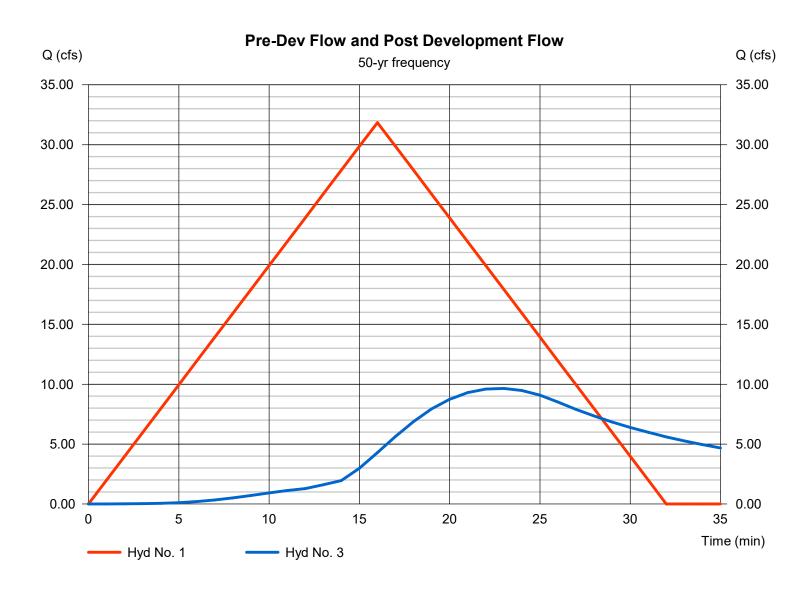
Hyd. No. 1

Pre-Dev Flow

Hydrograph type	= Rational
Peak discharge	= 31.84 cfs
Time to peak	= 16 min
Hyd. Volume	= 30,570 cuft

Hyd. No. 3

Hydrograph type	= Reservoir
Peak discharge	= 9.64 cfs
Time to peak	= 23 min
Hyd. Volume	= 29,672 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

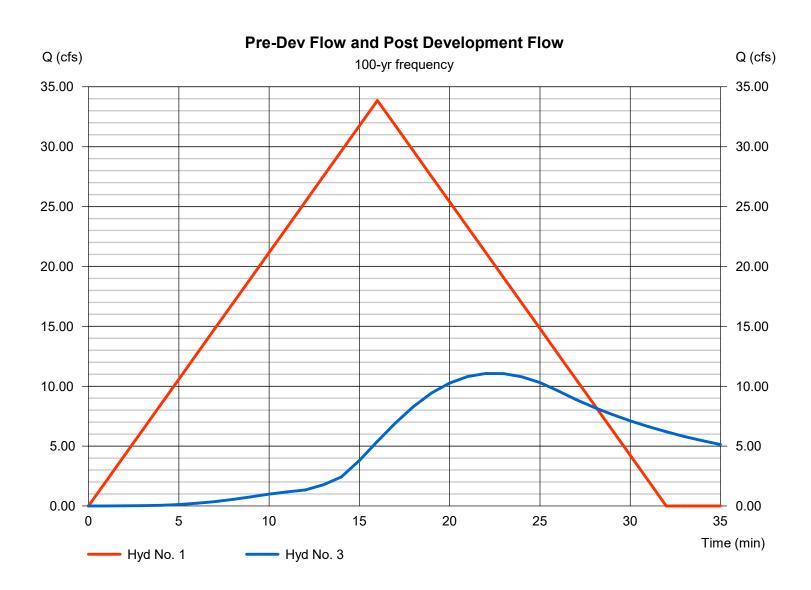
Hyd. No. 1

Pre-Dev Flow

Hydrograph type	= Rational
Peak discharge	= 33.86 cfs
Time to peak	= 16 min
Hyd. Volume	= 32,504 cuft

Hyd. No. 3

Hydrograph type	= Reservoir
Peak discharge	= 11.06 cfs
Time to peak	= 22 min
Hyd. Volume	= 31,482 cuft



Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Pond No. 1 - Retention Pond

Pond Data

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 393.40 ft

Stage / Storage Table

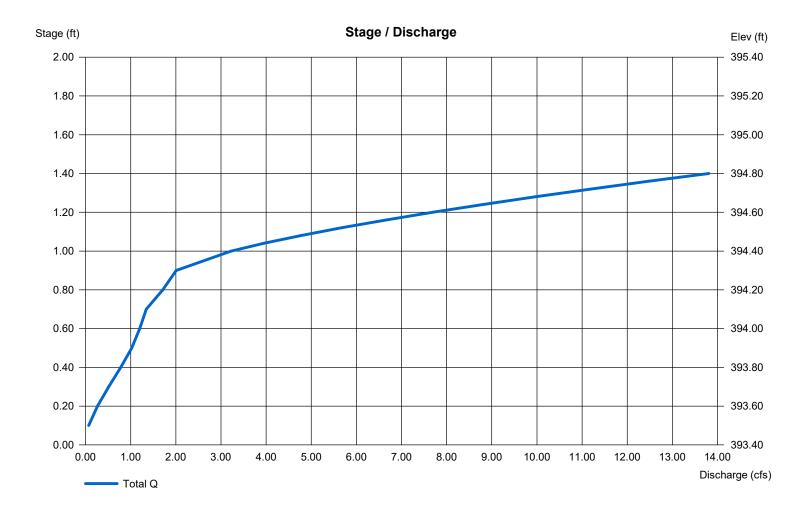
Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	393.40	16,570	0	0
1.00	394.40	21,182	18,827	18,827
1.40	394.80	23,045	8,842	27,669

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 8.00	8.00	Inactive	Inactive	Crest Len (ft)	= 10.00	0.00	0.00	0.00
Span (in)	= 8.00	8.00	0.00	0.00	Crest El. (ft)	= 394.30	0.00	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.03	3.33	3.33	3.33
Invert El. (ft)	= 393.40	393.40	0.00	0.00	Weir Type	= Rect			
Length (ft)	= 25.00	25.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 0.52	0.52	0.00	n/a	-				
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by Contour)			
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00	,		

Weir Structures

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	18.69	1	16	17,943				Pre-Dev Flow
2	Rational	22.67	1	13	17,679				Development Generated Flow
	Rational Reservoir	22.67	1	13 25	17,679	2	394.27	16,333	Development Generated Flow Post Development Flow
DE	TENTION-CO	ONTOUR.	gpw	DETENTION-CONTOUR.gpw				Wednesday	y, 09 / 4 / 2024

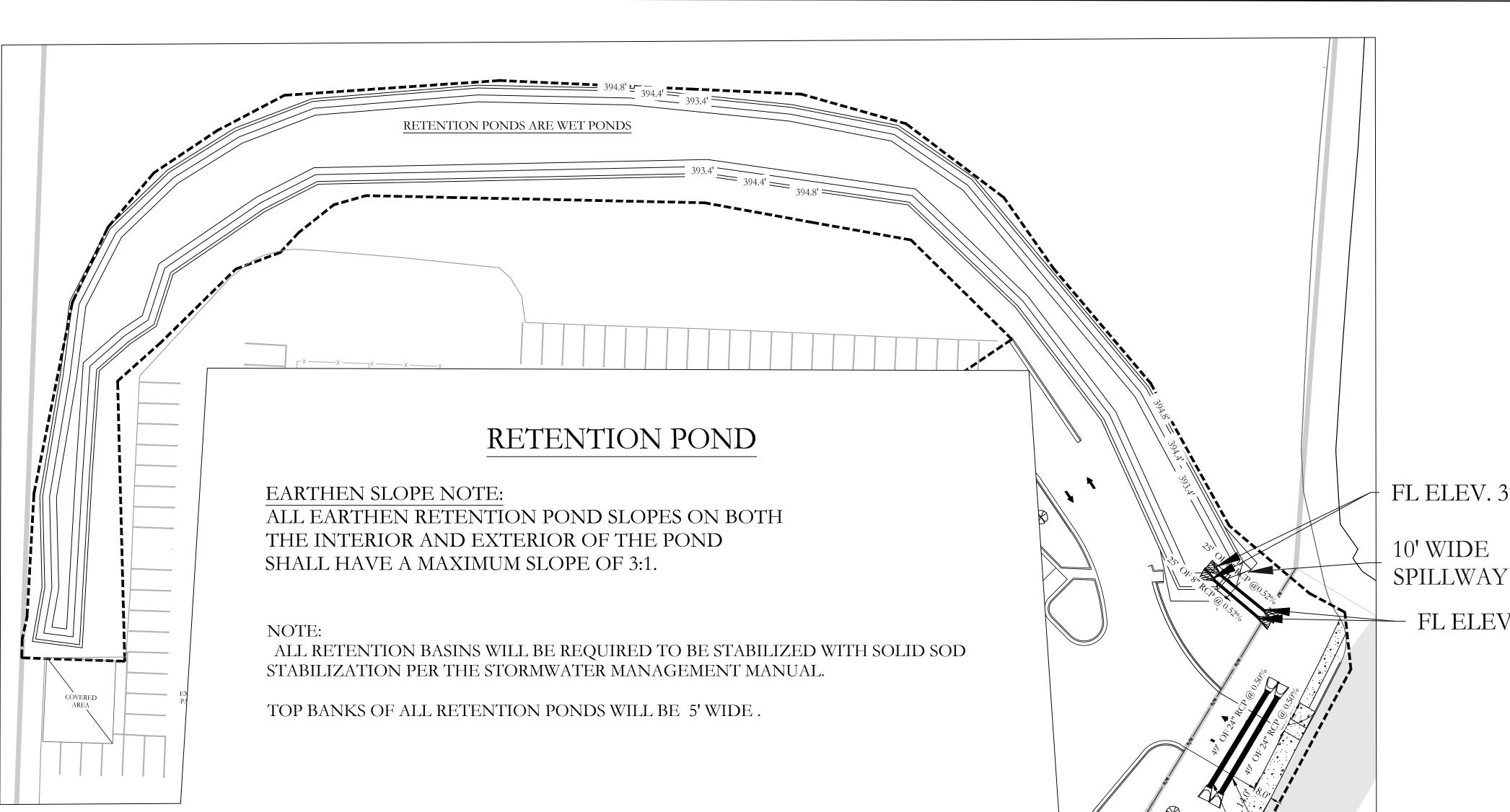
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	20.65	1	16	19,826				Pre-Dev Flow
2	Rational	25.15	1	13	19,614				Development Generated Flow
3	Reservoir	2.677	1	25	19,588	2	394.35	17,979	Post Development Flow
DE	TENTION-CO		.gpw		Return F	Period: 5 Ye	ear	Wednesda	y, 09 / 4 / 2024

łyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	24.35	1	16	23,373				Pre-Dev Flow
2	Rational	29.23	1	13	22,797				Development Generated Flow
	Reservoir	29.23	1	13 24	22,797 22,771	2	394.47	20,378	Development Generated Flow Post Development Flow
F	TENTION-CO		apw		Return F	Period: 10 \	/ear	Wednesda	y, 09 / 4 / 2024

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	27.93	1	16	26,812				Pre-Dev Flow
2	Rational	33.44	1	13	26,086				Development Generated Flow
23	Rational Reservoir	33.44 6.883		13 23	26,086	2	394.57	22,563	Development Generated Flow Post Development Flow
DETENTION-CONTOUR.gpw				Return F	Period: 25 Y	l ′ear	Wednesday	y, 09 / 4 / 2024	

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	31.84	1	16	30,570				Pre-Dev Flow
2	Rational	38.07	1	13	29,698				Development Generated Flow
23	Rational Reservoir	38.07 9.645		13 23	29,698 29,672	2	394.67	24,768	Development Generated Flow Post Development Flow
DE	TENTION-CO		gpw	<u> </u>	Return F	Period: 50 Y	íear	Wednesday	/, 09 / 4 / 2024

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	33.86	1	16	32,504				Pre-Dev Flow
2	Rational	40.40	1	13	31,509				Development Generated Flow
2 3	Reservoir	40.40		13 22	31,509 31,482	2	394.72	25,801	Development Generated Flow Post Development Flow
E	TENTION-CO		apw		Return I	Period: 100	Year	Wednesda	y, 09 / 4 / 2024



DETENTION POND MAINTENANCE PLAN

Background

The Retention ponds are located on the periphery of the subdivision. They are designed to temporarily detain stormwater to meet -Re-growth of trees on or around the pond bank. These should be cut and removed from the pond area. water quantity criteria before discharging off the property.

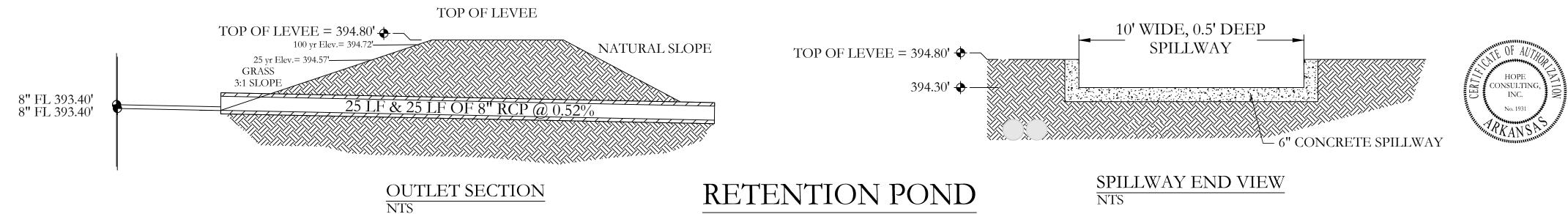
Routine Maintenance:

-Sediment from the site may accumulate in the pond bottom and reduce the pond to below design volume requirements. The The property owners association will maintain the drainage easements . Routine maintenance will include but not be limited to: pond should be excavated if the pond bottom elevation reached a level that allows excessive aquatic growth or reduces the -Mowing of the bank slopes and area around the pond on a monthly basis during the growing season and as needed during the pond efficiency such, that the sediments are passing the discharge structure and release off site. cooler months.

-The outlet pipe from the pond and other areas will be inspected monthly for debris which could inhibit the proper flow of discharge. Any debris will be removed immediately and disposed of or placed in a location to prevent future maintenance and to not cause impact up or downstream of the structure.

-Trash will be removed from around the pond to prevent entering the pond. Generally, the site should be kept free of loose trash which could be carried off site by wind or rain.

-Inspect the pond and outlet pipe for non-routine maintenance need.



Periodic or Non-Routine Maintenance

The routine inspection of the ponds areas and discharge pipes will identify needed repairs and non-routine maintenance. These items may include but not be limited to:

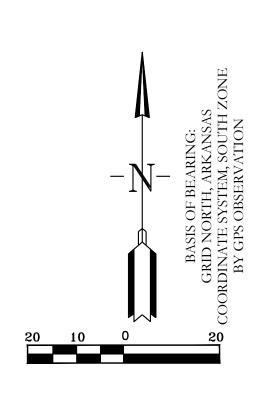
-Stabilization or re-grading of side slopes may be required periodically or after excessive rain events. Any disturbance of slopes should be reseeded or may require installation of erosion control materials until seeding can reestablish adequate grasses to prevent future erosion.

-Any other maintenance or repairs which would minimize other maintenance to the pond or outfall structures.



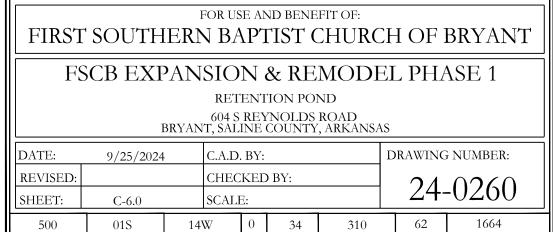
FL ELEV. 393.4'

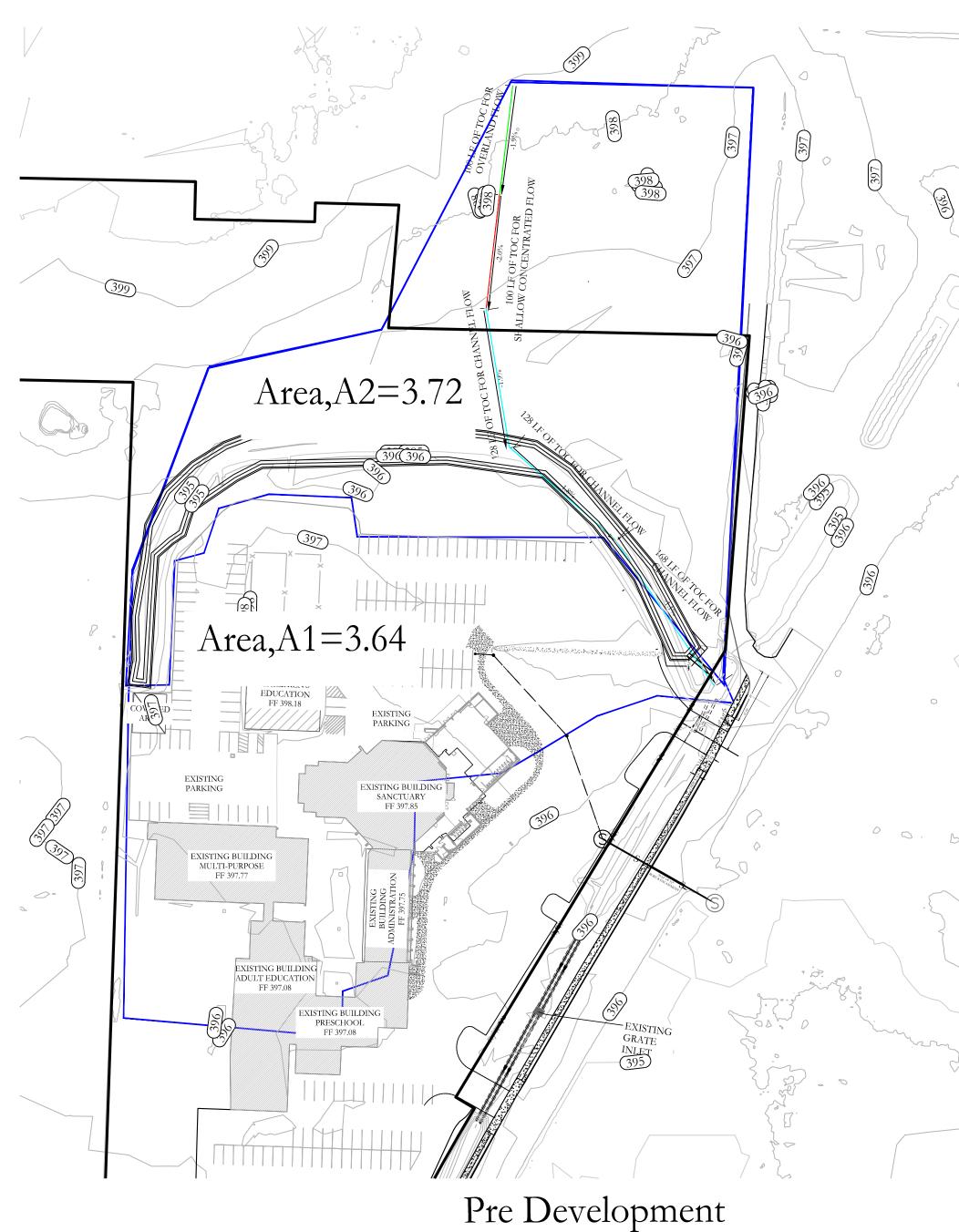
FL ELEV. 393.27'





129 N. Main Street, Benton, Arkansas 72015





Drainage Calculations'. TOC Calculations for 100 yr: Pre- Development: Overland Flow: $t_1 = 0.83 \left[\frac{NL}{50.5} \right] 0.467$ N=0.30 L = 100' 8 =-1.9% = 10.25min Shallow Concentrated How: V= 16.1345 (S) 5; S= 2.0% $tscs = \frac{L}{60V}$ = 2.28L = 100' = 0.73 min Channel Flow: L=420' L1=128; L2=128: L3=164' $tcs = \frac{L}{60V}$ S1 = 1.97.; S2 = 1.8%; ; S3=1-1%. = 5.19 minn = 0.15, R = 0.221: V = 1:49 (2)43 × (S)0.5 Jotal JOC = 16.16 min $V_1 = 0.49$ $V_2 = 0.48$ V = 1.35 $V_3 = 0.38$: Rainfall Intensity, I = 7.4 in/hr Area, A = 7.36 ac Run-Off Co-efficient (Cumulative) = 0.65 [C1 = 0.36, C2 = 0.95] Discharge, Q = CIA = 35.40 cfs

Post - Development: (Without - Detention) Overland Flow: $t_{1} = 10$

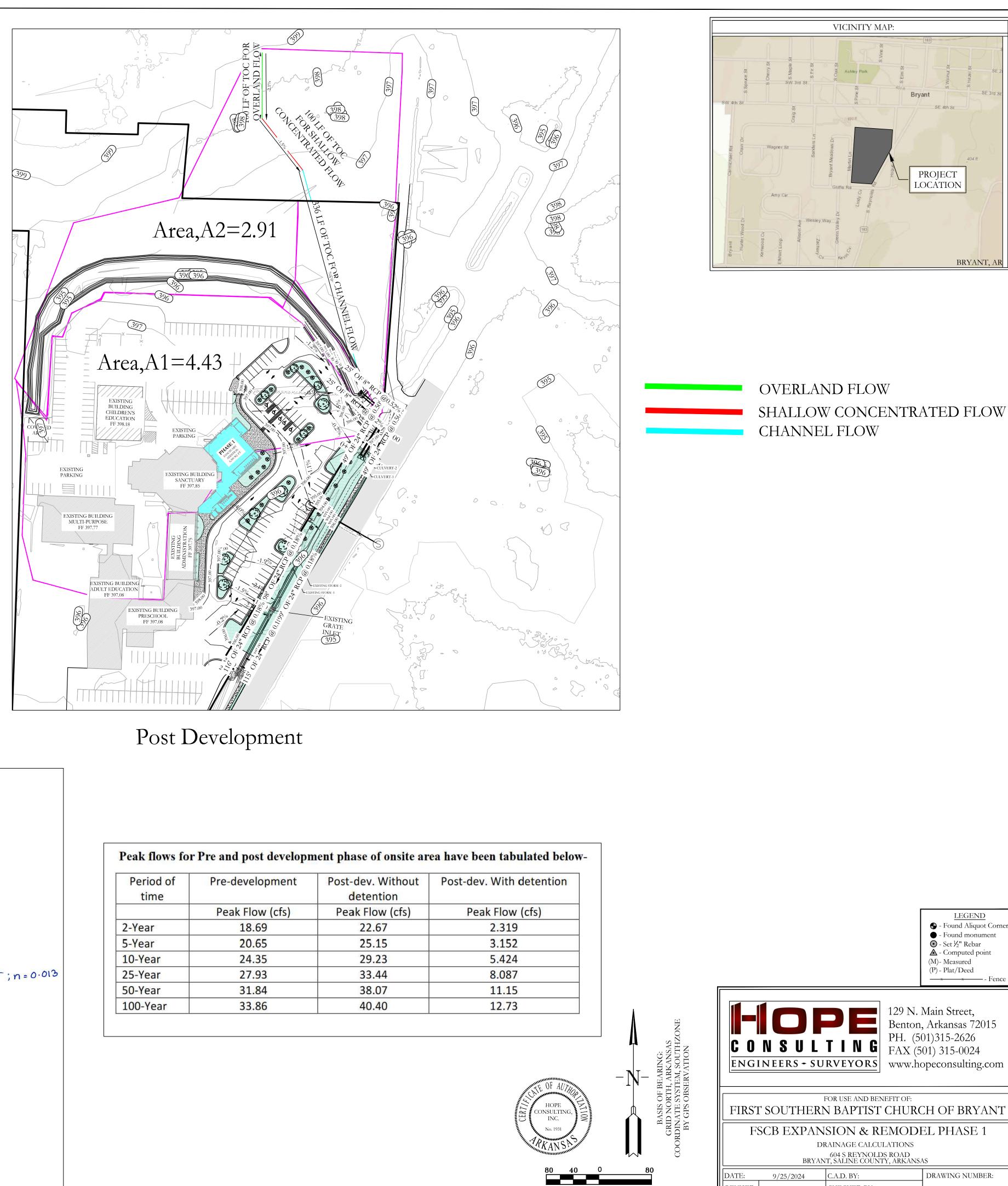
> Shallow Concentrate tscc =

Channel Flow: tsc =

. Total TOC = 12.

: Rainfall Ante Area, .: Cummulative Run-off cost ... Total Discharge, Q = CIA ... Q = 41.22 cts

(398) (398) (398) (398) (395)



0 015min	N = 0.30 L=100'; $S = 2.1$ %
ited Flow: 0.84 min	$V = 16.1345(5)^{5}$ 8 = 1.57. L = 100'
1.89 min	$ \begin{array}{c} L = 336' \\ S = 0.5\% \\ V = \frac{1.49}{n} R^{43} S^{1/5}; n = 0.07 \end{array} $
2.75 min	:N = 2.95
tensity, $I = 7$. A = 7.34 a , lative Run-of	

Period of time	Pre-development	Post-dev. Without detention	Post-de
	Peak Flow (cfs)	Peak Flow (cfs)	Pe
2-Year	18.69	22.67	
5-Year	20.65	25.15	
10-Year	24.35	29.23	
25-Year	27.93	33.44	
50-Year	31.84	38.07	
100-Year	33.86	40.40	

24-0260

C.A.D. BY:

SCALE:

CHECKED BY:

14W 0 34 310 62 1664

9/25/2024

C-5.0

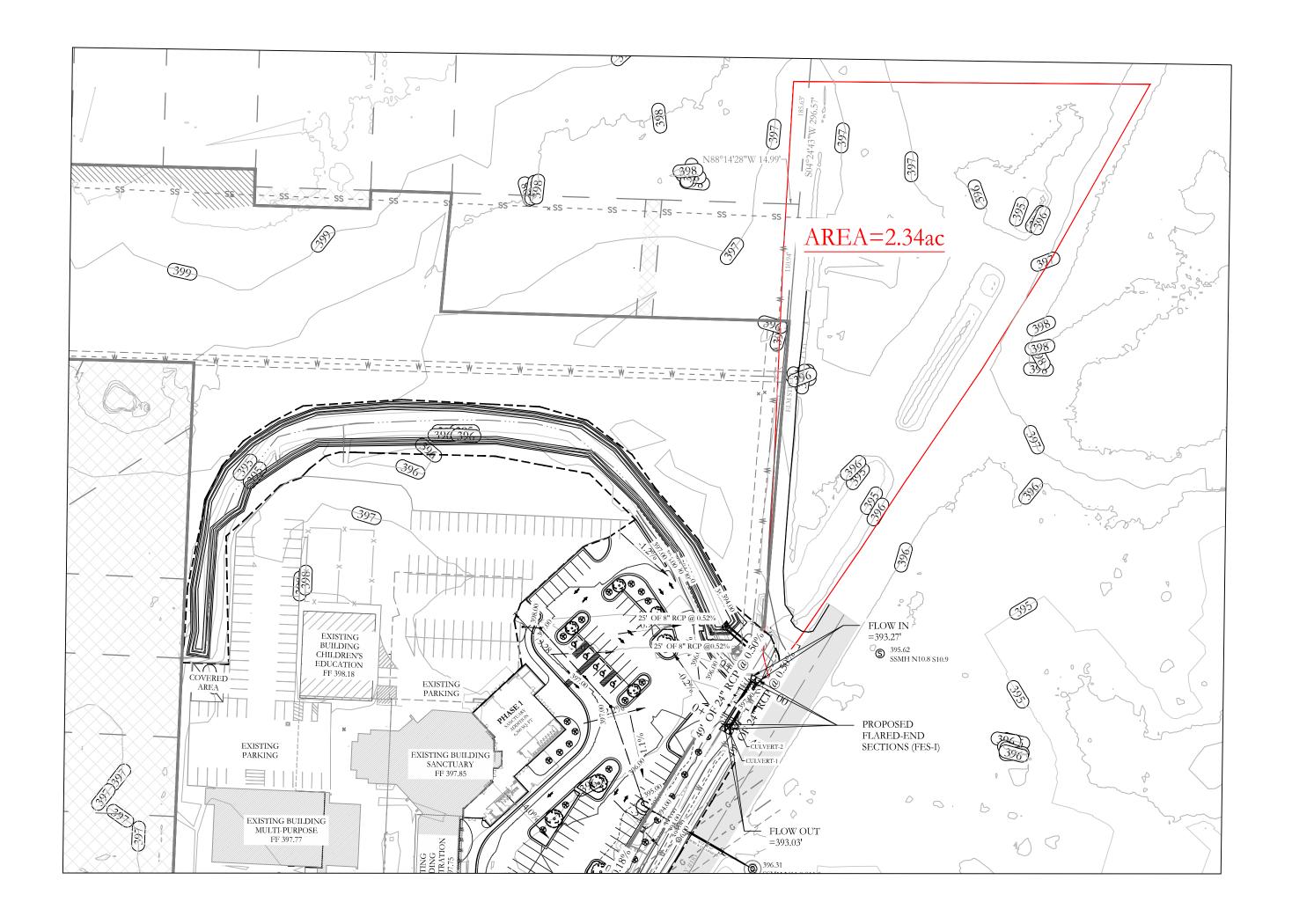
01S

DATE

REVISED:

SHEET:

500



Edge of pavement elev. =397.45' Proposed Driveway Surface elev. =397.00'

10 yr Storm Discharge Elevation=394.97' 50 yr Storm Discharge Elevation =395.28'

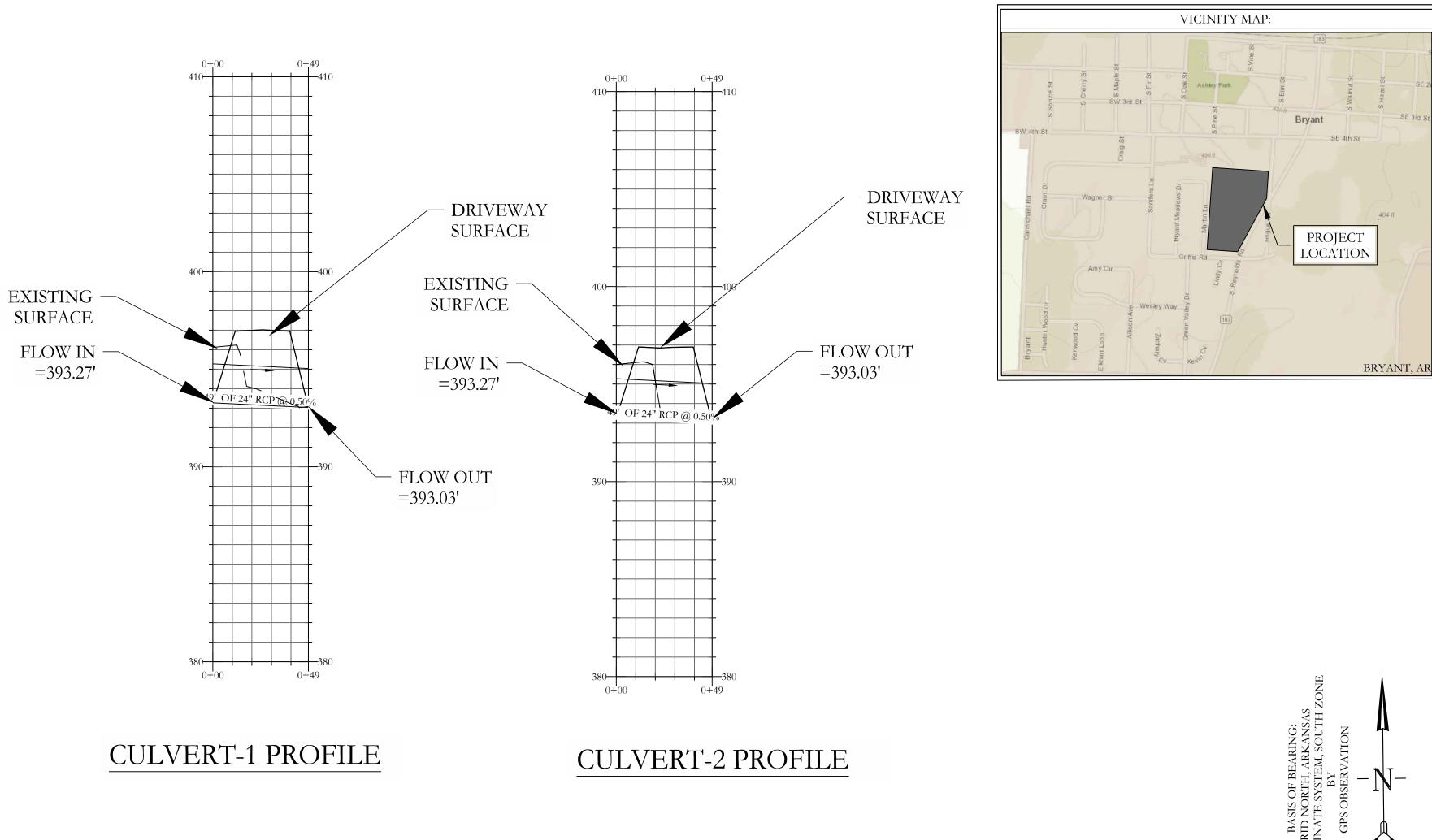
DRAINAGE CALCULATION

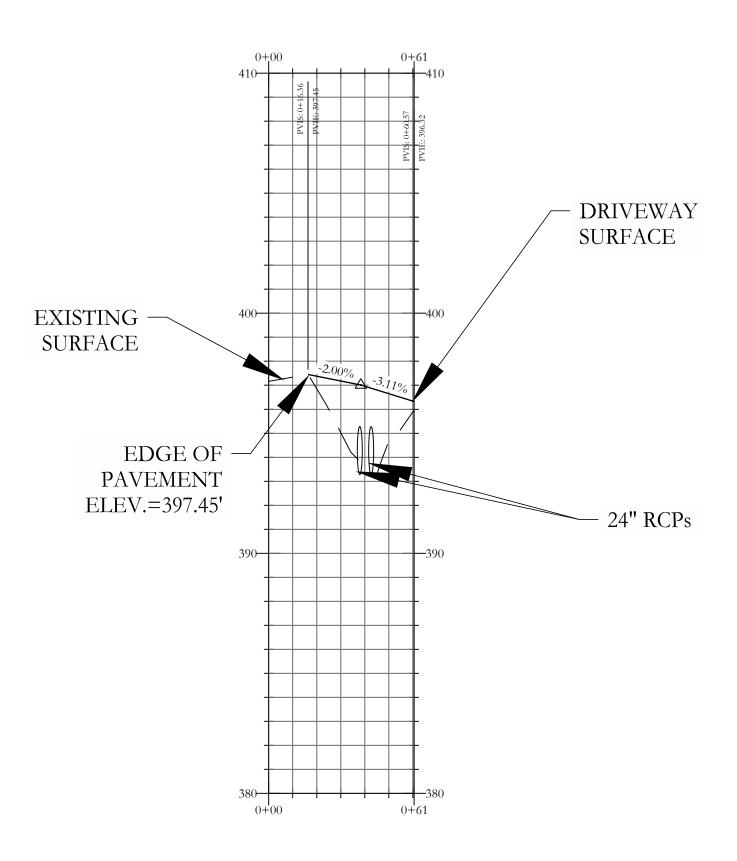
Discharge, Q₁₀= 0.83*6.3*2.34 =12.24 cfs Discharge, Q₅₀= 0.92*7.9*2.34 =17.00 cfs

Discharge from Detention Outlets: Discharge, Q₁₀= 4.569 cfs Discharge, Q₅₀=9.645 cfs

Total Discharge, Q₁₀=16.81 cfs Q₅₀=26.65 cfs

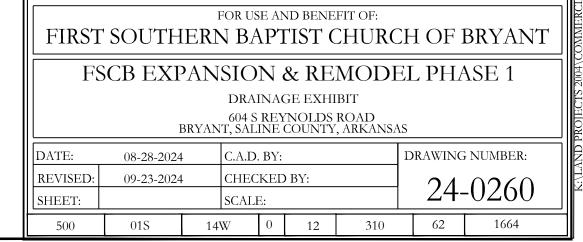
For 24" RCP pipes, 10 yr Storm Discharge Elevation, d₁₀=394.97' 50 yr Storm Discharge Elevation, d₅₀=395.28'





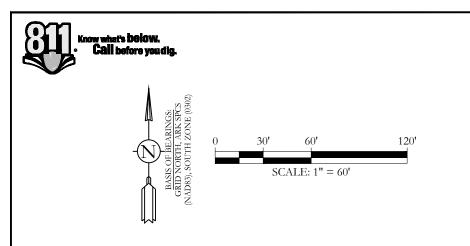
DRIVEWAY PROFILE











REFERENCE DOCUMENTS CITED RECORDED SURVEY PLATS BY: JOHN C. WILLIAMS (PS 1091), FOR MURPHY, DATED 10/5/2000 KERRY D. LANE (PS 1141), FOR W. GRAY, DATED 8/12/2013 FINAL PLAT OF BRYANT MEADOWS SUBD., PHASE 3, AS RECORDED IN BOOK 337, PAGE 452 JOHN A. LANE (PS 1740), FOR B. GRAY, DATED 3/18/2014 PLAT OF FISCHER'S SUBDIVISION, AS RECORDED IN BOOK 35, PAGE 400 SURVEY DETAILS AND NOTES:

OWNER OF RECORD: FIRST SOUTHERN BAPTIST CHURCH PHYSICAL ADDRESS: 604 S. REYNOLDS RD, BRYANT, AR COUNTY PARCEL TAX ID: 840-15972-003, 840-15972-000, 840-15973-000

ALL DIMENSIONS LISTED ARE AS MEASURED BY THIS SURVEYOR UNLESS OTHERWISE NOTED. FOR RECORD DIMENSIONS SEE DOCUMENTS OF RECORD.

OWNERSHIP INFORMATION, IF SHOWN, IS LISTED AS PUBLISHED BY THE LOCAL COUNTY TAX ASSESSOR AND IS LISTED FOR REFERENCE ONLY. NO STATEMENTS OF OWNERSHIP, RIGHTS, OR INTERESTS ARE MADE

THIS SURVEY IS BASED ON PUBLIC RECORDS AND/OR TITLE INVESTIGATIONS FURNISHED BY THIRD PARTIES. NO INDEPENDENT SEARCH OR INVESTIGATION HAS BEEN MADE BY THIS FIRM FOR ANY RECORDS, PUBLIC OR PRIVATE. LISTED REFERENCE DOCUMENTS HEREON WERE USED AND CONSIDERED AS A PART OF THIS SURVEY; HOWEVER OTHER RECORDS, IF ANY, COULD FURTHER AFFECT THIS SURVEY. NO STATEMENT OR GUARANTEES OF OWNERSHIP, RIGHTS, OR OTHER INTERESTS ARE MADE BY THIS SURVEY PLAT.

RECORD PROPERTY DESCRIPTION SALINE COUNTY INSTRUMENT 2000-05544

ALL THAT PART OF THE NORTHEAST QUARTER OF THE SOUTHWEST QUARTER OF SECTION 34, TOWNSHIP SOUTH, RANGE 14 WEST, CITY OF BRYANT, SALINE COUNTY, ARKANSAS, MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCING AT A 5/8 INCH REBAR ACCEPTED AS BEING THE SOUTHWEST CORNER OF SAID NE1/4 OF THE SW 1/4. SECTION 34: THENCE NORTH 89 DEG. 55 MIN. 27 SEC. EAST. ALONG THE SOUTH LINE THEREOF. 451.2 FEET TO A COTTON PICKER SPINDLE IN THE CENTERLINE OF GRIFFIS ROAD AND BEING THE POINT OF BEGINNING OF LANDS HEREIN DESCRIBED; THENCE SOUTH 89 DEG. 52 MIN. 03 SEC. EAST, ALONG THE CENTERLINE OF GRIFFIS ROAD AND THE SOUTH LINE OF SAID NE 1/4 OF THE SW 1/4, 313.90 FEET TO A RAILROAD SPIKE; THENCE SOUTH 89 DEG. 59 MIN. 25 SEC. EAST, ALONG THE CENTERLINE OF GRIFFIS ROAD AND THE SOUTH LINE OF SAID NE 1/4 OF THE SW 1/4, 197.38 FEET TO A COTTON PICKER SPINDLE IN THE WEST RIGHT OF WAY LINE OF ARKANSAS STATE HIGHWAY NO. 183; THENCE ALONG SAID STATE HIGHWAY RIGHT OF WAY LINE AND A CURVE TO THE RIGHT HAVING A RADIUS OF 1472.40 FEET FOR A CHORD OF NORTH 22 DEG. 3 MIN 45 SEC. EAST 359.84 FEET TO A REBAR: THENCE NORTH 60 DEG. 22 MIN. 07 SEC. WEST. ALONG SAID STATE HIGHWAY RIGHT OF WAY LINE, 20.00 FEET TO A REBAR; THENCE NORTH 29 DEG. 37 MIN. 53 SEC. EAST, ALONG SAID STATE HIGHWAY RIGHT OF WAY LINE, 477.50 FEET TO A REBAR IN THE EAST LINE OF SAID NE 1/4 OF THE SW 1/4: THENCE NORTH 02 DEG. 45 MIN. 15 SEC. EAST. ALONG EAST LINE OF SAID NE 1/4 OF THE SW 1/4. 288.55 FEET TO A RAILROAD SPIKE IN THE CENTERLINE OF ELM STREET AND THE SOUTHEAST CORNER OF LAND CONVEYED TO CITY OF BRYANT, ARKANSAS IN WARRANTY DEED DATED JUNE 15, 1961 AND FILED IN SALINE COUNTY DEED RECORD BOOK 103 AT PAGE 119 (SAID POINT BEING LOCATED SOUTH 02 DEG. 46 MIN. 30 SEC. WEST 296.59 FEET FROM THE NORTHEAST CORNER OF SAID NE 1/4 OF SW 1/4, SECTION 34); THENCE LEAVING SAID ELM STREET, SOUTH 89 DEG. 53 MIN. 52 SEC. WEST 130.05 FEET TO A REBAR AND THE SOUTHWEST CORNER OF SAID LAND CONVEYED TO CITY OF BRYANT; THENCE NORTH 02 DEG. 48 MIN. 12 SEC. EAST 111.00 FEET TO A REBAR AND THE NORTHWEST CORNER OF SAID LAND CONVEYED TO CITY OF BRYANT; THENCE SOUTH 89 DEG. 54 MIN. 12 SEC. WEST, ALONG THE SOUTH LINE OF LAND GRANTED TO T. W. COLE, JR. IN DECREE OF CONFIRMATION AND QUIETING OF TITLE DATED OCTOBER 31, 1968 AND FILED IN SALINE COUNTY DEED RECORD BOOK 135 AT PAGE 848, 15.10 FEET: THENCE SOUTH 02 DEG. 48 MIN. 12 SEC. WEST, ALONG THE EAST LINE OF LAND CONVEYED TO T. W. COLE, JR. AND MARY COLE, HUSBAND AND WIFE, IN WARRANTY DEED DATED JULY 29, 1974 AND FILED IN SALINE COUNTY DEED RECORD BOOK 182 AT PAGE 164, 111.50 FEET;

THENCE SOUTH 89 DEG. 34 MIN. 17 SEC. WEST, ALONG THE SOUTH LINE OF LANDS CONVEYED TO T. W. COLE. IR, AND MARY COLE, HUSBAND AND WIFE, IN SAID DEED BOOK 182 AT PAGE 164, 185.00 FEET; THENCE NORTH 02 DEG. 48 MIN. 12 SEC. EAST, ALONG THE WEST LINE OF SAID LANDS CONVEYED IN DEED BOOK 182 AT PAGE 164, 112.98 FEET; THENCE SOUTH 89 DEG. 34 MIN. 17 SEC. WEST, ALONG THE SOUTH LINE OF LANDS CONVEYED TO ROBERT LEE NOWLIN IN WARRANTY DEED DATED MARCH 16, 1971 AND FILED IN SALINE COUNTY DEED RECORD BOCK 146 AT PAGE 487 AND AS CONVEYED TO ROBERT LEE NOWLIN AND MARIE NELL NOWLIN, HUSBAND AND WIFE, IN CORRECTION WARRANTY DEED DATED SEPTEMBER 19, 1974 AND FILED IN SALINE COUNTY DEED RECORD BOOK 194 AT PAGE 104, 187.62 FEET; THENCE NORTH 00 DEG, 25 MIN. 43 SEC. WEST. ALONG THE WEST LINE OF LANDS CONVEYED TO SAID ROBERT LEE NOWLIN AND MARIE NELL NOWLIN HUSBAND AND WIFE, 16.50 FEET TO THE SOUTHEAST CORNER OF LOT 6, FISCHER'S SUBDIVISION; THENCE SOUTH 89 DEG. 34 MIN. 17 SEC. WEST, ALONG THE SOUTH LINE OF SAID FISCHER'S SUBDIVISION, 436.00 FEET TO THE SOUTHWEST CORNER OF LOT 2, FISCHER'S SUBDIVISION; THENCE SOUTH 00 DEG. 25 MIN. 43 SEC. EAST, ALONG THE EAST LINE OF LAND CONVEYED TO PAUL DOUGLAS GATTIN AND ALISA ANN GATTIN. HUSBAND AND WIFE, IN WARRANTY DEED DATED JANUARY 14, 1986 AND FILED IN SALINE COUNTY DEED RECORD BOOK 279 AT PAGE 512, 19.00 FEET; THENCE SOUTH 89 DEG. 34 MIN. 17 SEC. WEST, ALONG THE SOUTH LINE OF LAND CONVEYED TO SAID PAUL DOUGLAS GATTIN AND ALISA ANN GATTIN, HUSBAND AND WIFE, 87.20 FEET; THENCE NORTH 00 DEG. 25 MIN. 43 SEC. WEST, ALONG THE WEST LINE OF LAND CONVEYED TO SAID PAUL DOUGLAS GATTIN AND ALISA ANN GATTIN. HUSBAND AND WIFE, 19.00 FEET TO THE SOUTHWEST CORNER OF LOT 1, FISCHER'S SUBDIVISION; THENCE SOUTH 89 DEG. 35 MIN. 22 SEC. WEST, ALONG THE SOUTH LINE OF LAND CONVEYED TO JOHN L. JACKSON AND ROBIN A. JACKSON, HUSBAND AND WIFE, IN WARRANTY DEED DATED JULY 3, 1986 AND FILED IN SALINE COUNTY DEED RECORD BOOK 284 AT PAGE 118, 269.38 FEET TO A COTTON PICKER SPINDLE IN THE CENTERLINE OF SANDERS ROAD AND THE WEST LINE OF SAID NE 1/4 OF THE SW 1/4 (SAID POINT BEING LOCATED SOUTH 01 DEG. 24 MIN. 00 SEC. WEST 166.5 FEET FROM THE NORTHWEST CORNER THEREOF); THENCE SOUTH 01 DEG. 24 MIN. 00 SEC. WEST, ALONG THE CENTERLINE OF SANDERS ROAD AND THE WEST LINE OF SAID NE 1/4 OF SW 1/4, 33.54 FEET TO A COTTON PICKER SPINDLE; THENCE LEAVING SAID ROAD. SOUTH 8S DEG. 26 MIN. 35 SEC. EAST. ALONG THE NORTH LINE OF LAND

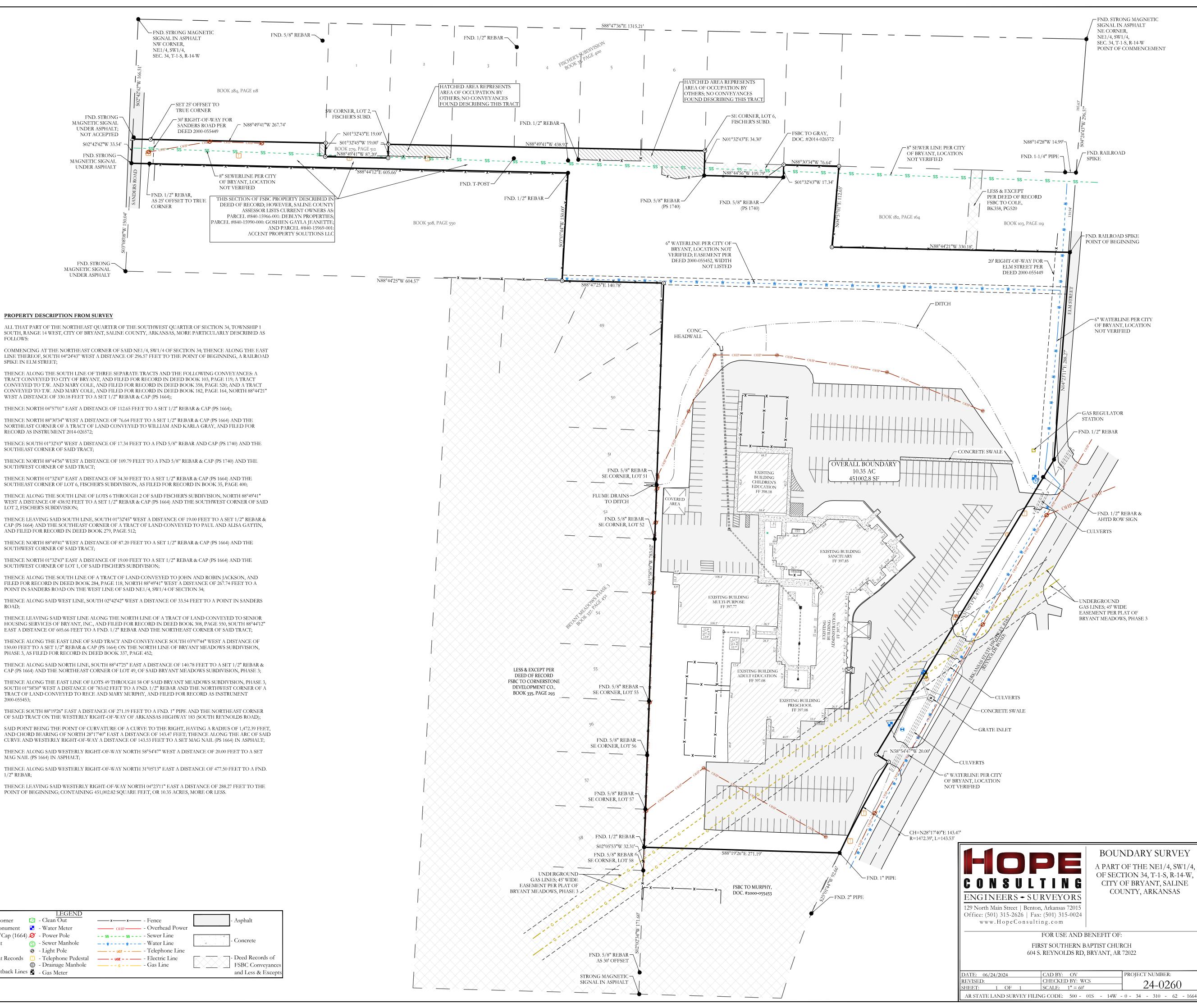
FILED IN SALINE COUNTY DEED RECORD BOOK 308 AT PAGE 550, 606.92 FEET TO A REBAR; THENCE SOUTH 01 DEG. 29 MIN. 25 SEC. WEST, ALONG THE EAST LINE OF LAND CONVEYED TO SAID SENIOR HOUSING SERVICES OF BRYANT, INC., 150.00 FEET TO A REBAR; THENCE NORTH 89 DEG. 26 MIN. 34 SEC. WEST, ALONG THE SOUTH LINE OF LAND CONVEYED TO SAID SENIOR HOUSING SERVICES OF BRYANT, INC., 152.99 FEET TO A REBAR; THENCE SOUTH 01 DEG 33 MIN 04 SEC WEST ALONG THE EAST LINE OF LANDS CONVEYED TO CORNERSTONE DEVELOPMENT CO. IN WARRANTY DEED DATED SEPTEMBER 6, 1989 AND FILED IN SALINE COUNTY DEED RECORD BOOK 322 AT PAGE 285, 877.01 FEET TO THE POINT OF BEGINNING, CONTAINING 18.4662 ACRES, MORE OR LESS. SUBJECT TO A 30 FOOT ROAD RIGHT OF WAY FOR GRIFFIS ROAD. SUBJECT TO A 20 FOOT ROAD RIGHT OF WAY FOR ELM STREET. SUBJECT TO A 30 FOOT ROAD RIGHT OF WAY FOR SANDERS ROAD. SUBJECT TO A WATER LINE EASEMENT TRAVERSING THE NORTHERN PORTION OF LANDS DESCRIBED HEREINABOVE. SUBJECT TO A GAS LINE EASEMENT TRAVERSING SUBJECT PROPERTY.

CONVEYED TO SENIOR HOUSING SERVICES OF BRYANT. INC. IN WARRANTY DEED DATED JULY 11, 1988 AND

LESS AND EXCEPT: THAT PORTION OF THE NORTHEAST QUARTER OF THE SOUTHWEST QUARTER OF SECTION 34, TOWNSHIP 1 SOUTH, RANGE 14 WEST, CITY OF BRYANT, SALINE COUNTY, ARKANSAS, MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGINNING AT THE SOUTHWEST CORNER OF THE NE 1/4 OF SW 1/4, THENCE NORTH 02 DEG. 45 MIN. 10 SEC. EAST (ASTRONOMIC) 982.05 FEET (DEED NORTH 01 DEG. 24 MIN. EAST 982.11 FEET) ALONG THE WEST LINE THEREOF: THENCE SOUTH 89 DEG. 01 MIN. 06 SEC. EAST 452.81 FEET TO THE NORTHEAST CORNER OF LANDS DESCRIBED IN SALINE COUNTY DEED RECORD BOOK 322 AT PAGE 285, SAID POINT BEING THE POINT OF BEGINNING OF LAND HEREIN DESCRIBED; THENCE CONTINUE SOUTH 89 DEG. 01 MIN. 06 SEC. EAST 294.31 FEET; THENCE SOUTH 01 DEG. 41 MIN. 45 SEC. WEST 986.72 FEET TO THE SOUTH LINE OF SAID NE 1/4 OF SW 1/4: THENCE NORTH 88 DEG. 43 MIN. 48 SEC. WEST. ALONG SAID SOUTH LINE. 313.90 FEET TO THE SOUTHEAST CORNER OF LANDS DESCRIBED IN SALINE COUNTY DEED RECORD BOOK 322 AT PAGE 285: THENCE NORTH 02 DEG. 50 MIN. 15 SEC. EAST 984.37 FEET, ALONG THE EAST LINE OF LANDS DESCRIBED IN SAID DEED, TO THE POINT OF BEGINNING.

ALSO, LESS AND EXCEPT: THAT PART OF THE NORTHEAST QUARTER OF THE SOUTHWEST QUARTER OF SECTION 34, TOWNSHIP 1 SOUTH, RANGE 14 WEST, SALINE COUNTY, ARKANSAS, DESCRIBED AS FOLLOWS: BEGINNING AT A POINT THAT IS 185.5 FEET SOUTH OF THE NORTHEAST CORNER OF SAID NE 1/4 OF SW 1/4 AND RUN THENCE WEST FOR 130 FEET FOR THE POINT OF BEGINNING OF LAND HEREIN DESCRIBED; RUN THENCE WEST FOR 15 FEET; THENCE SOUTH 111.5 FEET; THENCE EAST 15 FEET; THENCE NORTH 111.5 FEET TO THE POINT OF BEGINNING



PROPERTY DESCRIPTION FROM SURVEY

FOLLOWS:

SPIKE IN ELM STREET;

WEST A DISTANCE OF 330.18 FEET TO A SET 1/2" REBAR & CAP (PS 1664);

SOUTHEAST CORNER OF SAID TRACT;

SOUTHWEST CORNER OF SAID TRACT;

CAP (PS 1664) AND THE SOUTHEAST CORNER OF A TRACT OF LAND CONV

SOUTHWEST CORNER OF SAID TRACT:

POINT IN SANDERS ROAD ON THE WEST LINE OF SAID NE1/4, SW1/4 OF SECTION 34;

ROAD;

PHASE 3, AS FILED FOR RECORD IN DEED BOOK 337, PAGE 452;

2000-055453:

MAG NAIL (PS 1664) IN ASPHALT;

1/2" REBAR;



NO PORTION OF THE PROPERTY DESCRIBED HEREON LIES WITHIN A SPECIAL FLOOD HAZARD AREA, ACCORDING TO THE FEMA FLOOD INSURANCE RATE MAP LISTED BELOW: PANEL # 05125C0380E , DATED: 06/05/2020

FLOOD ZONE INFORMATION

🕀 - PLSS Aliquot Corner	070	- Cl
 Fnd. Corner Monument 		- W
⊙ - Set 1/2" Rebar/Cap (1664)	Ø	- Po
\triangle - Computed Point	\bigcirc	- Se
(M)- As Measured	ø	- Liş
(P) - Per Deed or Plat Records		- Te
ESMT - Easement	D	- D1
B.S.L Building Setback Lines	c۷	- Gé





September 4, 2024

Colton Leonard City of Bryant 210 S.W. 3rd Street Bryant, Arkansas 72022

RE: First Southern Batist Church of Bryant Expansion (Hope Job#24-0260) 604 S. Reynolds Road Bryant, Arkansas

Dear Mr. Leonard,

I am writing to this item be added to the DRC agenda next week. I have attached construction drawings for the phased expansion of this property. This expanision will increase the building footprint, increase parking, create a new sewer main connection, and modify access to the highway. We will of course follow the drainage manuel in desinging the detention pond and work closely with ArDOT on access to the highway.

Bryant Water and Sewer Service this property currently. The electric is served by Entergy. There is a large gas transmission line that crosses this property. Our proposed sewer main extention will cross this transmission line but precaustions will be taken.

The church is very excited about this expansioin and we are also happy to see the groth in this community. We look forward to discussing this project with you at DRC.

Sincerely,

Jonathan Hope

SKY BLUE DUPLEXES PROPOSED MULTI-FAMILY UNITS

DRAINAGE REPORT

FOR

City of Bryant, AR

DATE

Hurricane Lake Road, Saline County, AR

By:



APPENDIX

- Project Description/Summary
- Detention Discharge Summary, Composite C Values, & time of concentration

Street Drainage Calculation

East Ditch Calculations

Time of Concentration Calculation

Pond Report

Hydrographs

East Ditch Exhibit

<u>Summary</u>

The following calculations pertain to the detention design for the proposed multi family development Located off Hurricane Lake Road in Bryant, AR.

Proposed Development area $= 1$.17 Acres	
	С	tc (min)
Pre-development:	0.49	23
Post-development:	0.69	23

Detention Pre & Post Development Comparisons

Prior to detention routing:

Event (yrs)	Pre-developed Flow Q (cfs)	Post-developed Flow (no pond) Q (cfs)
2	1.79	2.52
10	2.48	3.50
25	2.87	4.04
50	3.26	4.59
100	3.49	4.92

After routing to detention:

Event (yrs)	Pre-developed Q (cfs)	Post-developed (with pond) Q (cfs)	Water El. (ft)
2	1.79	1.23	402.37
10	2.48	1.52	402.66
25	2.87	1.64	402.80
50	3.26	1.75	402.96
100	3.49	1.82	403.06

Therefore, the development will not create any additional flow in the downstream area.

East Channel

The following calculations pertain to the existing east ditch, and are based on proposed re-design and excavation of the existing channel in order to have the needed vertical room necessary for detention and 2.0 feet of freeboard for the finished floor elevations of proposed structures.

time of concentration, tc (min)	REGION 3 IDF			
Pre				
Channel Dimension	s and Time of Con	centration, to	:	
Area (ft2)	1998592.29			
Area (Acre)	46			
Length, L (ft)	2217.0			
Change in Elevation (ft)	60.27			
Slope, S (ft/ft)	0.027			
N (asphalt,grass,etc)	0.400		h (ft)	S
L(overland, ft)	200		4	0.020
L(channel 1, ft)	2017		56.27	0.028
L(channel 2, ft)	0.0		0	0.000
ti	45.4	v		
t _{t1}	5.6	6.007023		
t _{t2}	0.0	0		
time of concentration, tc (min)	51.0	use 50 mi	n	

	Design Peak Runoff Rates, Qp (cfs)									
	Intensity, I (in/hr)	Flow (cfs)								
	1		С	Q						
ear		4.19	0.53	101.89						
	Qp,max (max flow) cfs			102						

100ye

V-Bottom Ditch (Analysis)

Side Slope	Q	n	Slope, m	Depth	Depth	Area	Velocity	Width
	cfs		ft/ft	ft	in	ft²	ft/sec	ft
1: 3	103.0	0.023	0.005	2.53	30.4	19.26	5.35	15.20
STATION '	1+68							
Elev. + 2.0'	Y + depth	Dist to	outlet El. (@ Outlet	Low Point			
freeboard		x	y=	=mx+b	b			
403.31	400.78	3 168	3.4 39	98.242	397.4			
V-Bottom	Ditch (Ana	lysis)						
Side Slope	Q	n	Slope, m	Depth	Depth	Area	Velocity	Width
	cfs		ft/ft	ft	in	ft²	ft/sec	ft
1: 2	103.0	0.023	0.005	2.95	35.4	17.40	5.92	11.80
STATION	1+00							
El. + 2.0	Y + dept	h Re-gr	ade Dist	El. @ x	Low Point			
freeboard			x	y=mx+b	b			
403.80	400.85	1	00	397.9	397.4			

PRE DEVELOPMENT TOC:

Time of Concentration, tc (min)	Bryant IDF				
Channel Dimensions	and Time of Co	oncentratio	n, tc		
Area (ft2)	40262.9				
Area (Acre)	0.92				
Length, L (ft)	837.0				
Change in Elevation (ft)	32				
Slope, S (ft/ft)	0.038				
N (Coeff. Of roughness, Table 400-3)	0.100		h (ft)	S	
L(overland/sheet flow, ft)	75		1		0.013
L(channel 1, ft)	601		25.00		0.04
L(channel 2, ft)	161.0		1		0.006
ti	18.4	v			
t _{t1}	3.3	3.0241			
t _{t2}	0.9	2.909438			
time of concentration, tc (min)	22.7			us	e 23

POST DEVELOPMENT TOC:

time of concentration, tc (min)	Bryant IDF				
Channel Dimensions	and Time of Co	oncentratio	n, tc		
Area (ft2)	40262.9				
Area (Acre)	0.92				
Length, L (ft)	888.0				
Change in Elevation (ft)	32				
Slope, S (ft/ft)	0.036				
N (Coeff. Of roughness, Table 400-3)	0.100		h (ft)	S	
L(overland/sheet flow, ft)	75		1		0.013
L(channel 1, ft)	659		25.00		0.04
L(channel 2, ft)	154.0		3		0.017
t _i	18.4	v			
t _{t1}	3.8	2.887956			
t _{t2}	0.5	4.77828			
time of concentration, tc (min)	22.8			use	23

^a Watershed Model Schematic

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

1

<u>Legend</u>

<u>Hyd.</u>	<u>Origin</u>	Description
1	Rational	PRE DEV FLOW
2	Rational	DEVELOPMENT CREATED FLOW
3	Reservoir	POST DEV. FLOW

Project: 19-0066 Bessent Duplexes _09-25-2024.gpw

Hydrograph Return Period Recap Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

lyd. Io.	Hydrograph type	Inflow hyd(s)		Peak Outflow (cfs)							Hydrograph Description
	(origin)		1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	2000.19101
	Rational			1.786			2.482	2.872	3.262	3.493	PRE DEV FLOW
2	Rational			2.515			3.495	4.044	4.593	4.919	DEVELOPMENT CREATED FLOW
3	Reservoir	2		1.232			1.524	1.643	1.752	1.815	POST DEV. FLOW
^{>} rc	oj. file: 19-006	66 Besser	nt Duplex	es_09-2	25-2024.	gpw			We	ednesdav	y, 09 / 25 / 2024

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	1.786	1	23	2,464				PRE DEV FLOW
2	Rational	2.515	1	23	3,470				DEVELOPMENT CREATED FLOW
3	Reservoir	1.232	1	35	3,464	2	402.37	1,896	POST DEV. FLOW

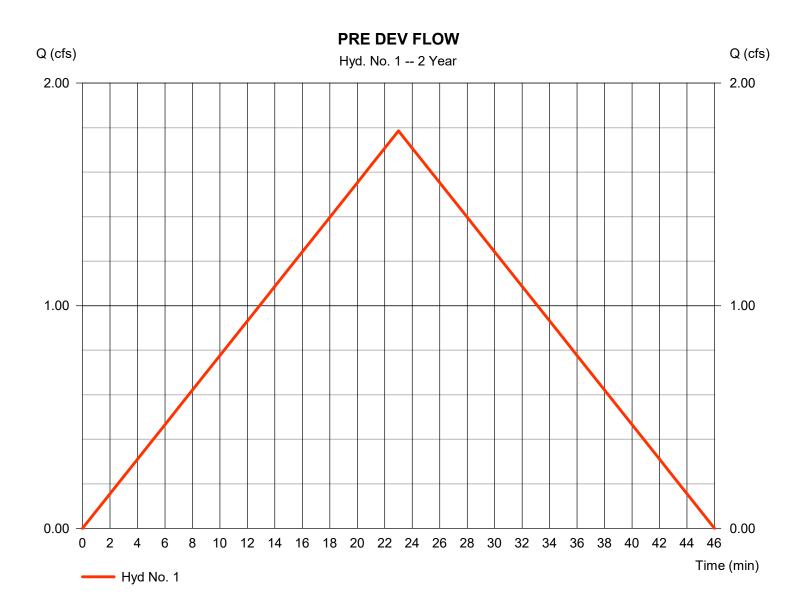
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Wednesday, 09 / 25 / 2024

Hyd. No. 1

PRE DEV FLOW

Hydrograph type	= Rational	Peak discharge	= 1.786 cfs
Storm frequency	= 2 yrs	Time to peak	= 23 min
Time interval	= 1 min	Hyd. volume	= 2,464 cuft
Drainage area	= 1.170 ac	Runoff coeff.	= 0.49
Intensity	= 3.115 in/hr	Tc by User	= 23.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1
	-		

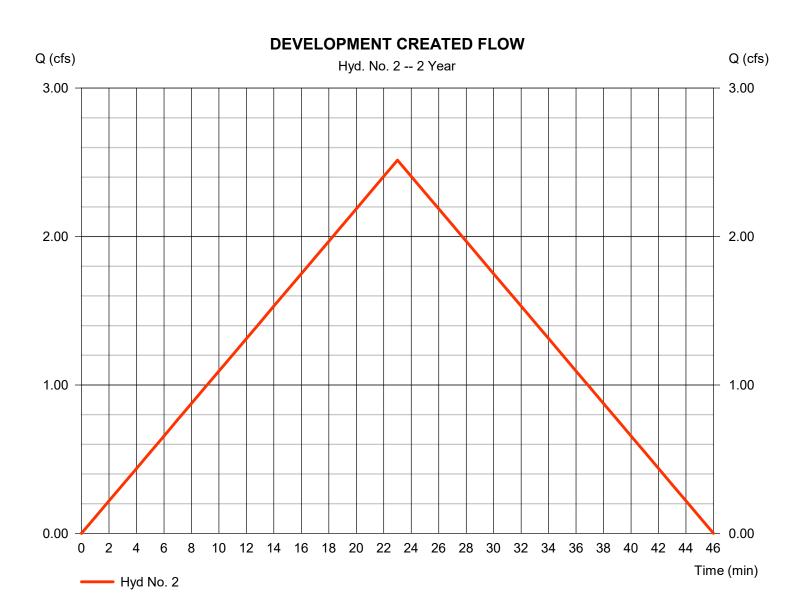


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Hyd. No. 2

DEVELOPMENT CREATED FLOW

Hydrograph type	 Rational 2 yrs 1 min 1.170 ac 3.115 in/hr 	Peak discharge	= 2.515 cfs
Storm frequency		Time to peak	= 23 min
Time interval		Hyd. volume	= 3,470 cuft
Drainage area		Runoff coeff.	= 0.69
Intensity		Tc by User	= 23.00 min
Intensity	= 3.115 in/hr	Tc by User	= 23.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1



5

Wednesday, 09 / 25 / 2024

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

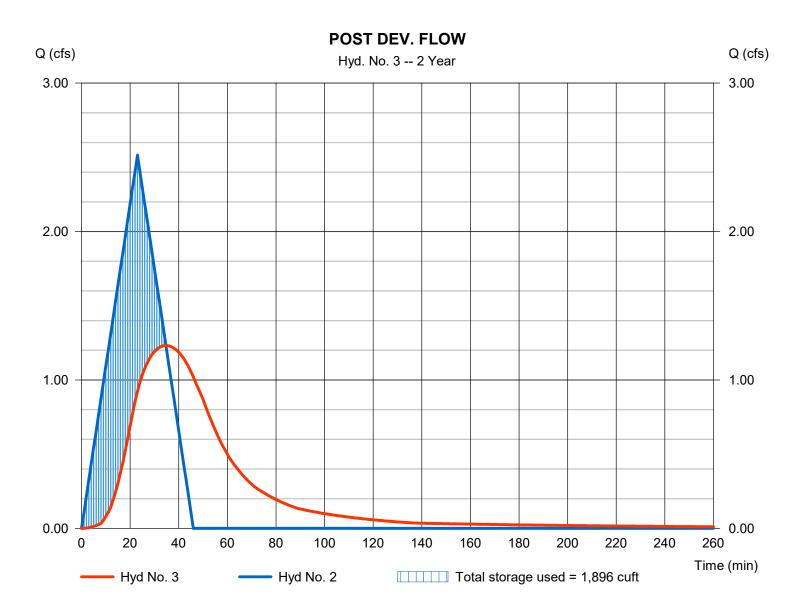
Wednesday, 09 / 25 / 2024

Hyd. No. 3

POST DEV. FLOW

Hydrograph type	= Reservoir	Peak discharge	= 1.232 cfs
Storm frequency	= 2 yrs	Time to peak	= 35 min
Time interval	= 1 min	Hyd. volume	= 3,464 cuft
Inflow hyd. No.	= 2 - DEVELOPMEN	CREATEDME CEMevation	= 402.37 ft
Reservoir name	= DETENTION	Max. Storage	= 1,896 cuft

Storage Indication method used.



Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Pond No. 1 - DETENTION

Pond Data

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 401.50 ft

Stage / Storage Table

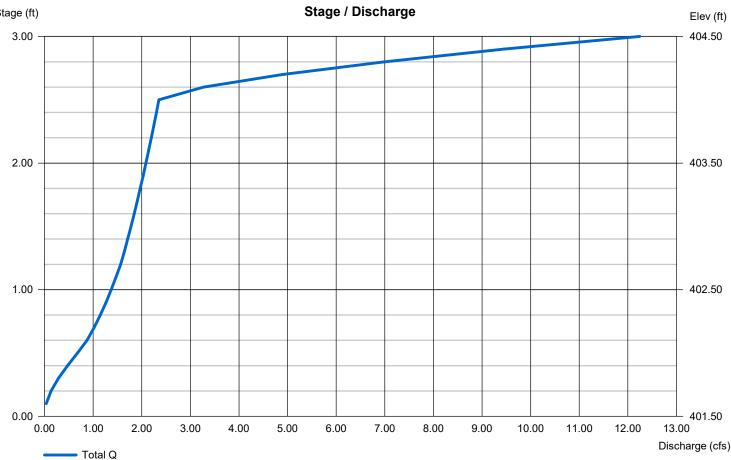
Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	401.50	1,616	0	0
1.00	402.50	2,786	2,174	2,174
2.00	403.50	4,028	3,388	5,562
3.00	404.50	5,328	4,662	10,224

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 8.00	Inactive	Inactive	0.00	Crest Len (ft)	= 10.50	0.00	0.00	0.00
Span (in)	= 8.00	0.00	0.00	0.00	Crest El. (ft)	= 404.00	0.00	0.00	0.00
No. Barrels	= 1	0	0	0	Weir Coeff.	= 2.60	3.33	3.33	3.33
Invert EI. (ft)	= 401.50	0.00	0.00	0.00	Weir Type	= Broad			
Length (ft)	= 34.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 1.47	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Wet area)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Weir Structures



7

Stage (ft)

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	2.482	1	23	3,425				PRE DEV FLOW
2	Rational	3.495	1	23	4,823				DEVELOPMENT CREATED FLOW
3	Reservoir	1.524	1	36	4,817	2	402.66	2,704	POST DEV. FLOW
19-	0066 Bessen	t Duplexe	es_09-28	5-2024.gr	w Return	Period: 10 `	Year	Wednesda	ay, 09 / 25 / 2024

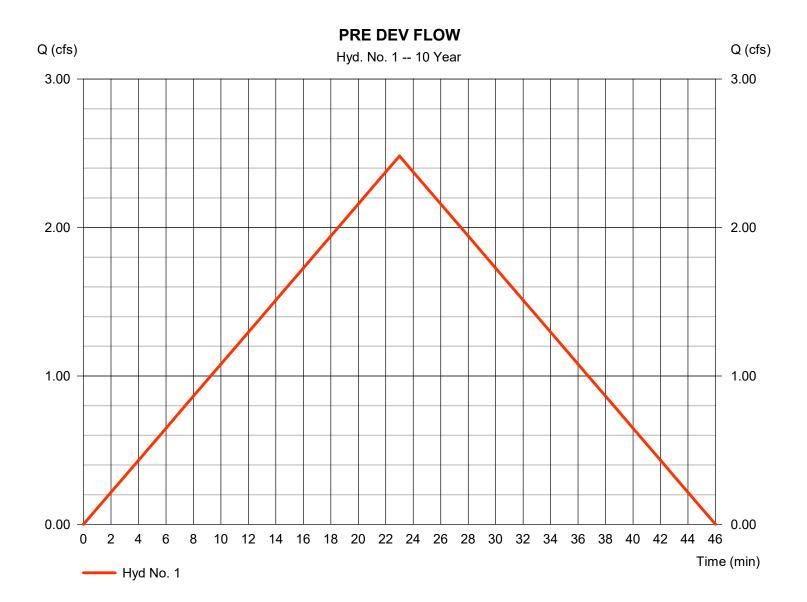
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Wednesday, 09 / 25 / 2024

Hyd. No. 1

PRE DEV FLOW

Hydrograph type	= Rational	Peak discharge	= 2.482 cfs
Storm frequency	= 10 yrs	Time to peak	= 23 min
Time interval	= 1 min	Hyd. volume	= 3,425 cuft
Drainage area	= 1.170 ac	Runoff coeff.	= 0.49
Intensity	= 4.330 in/hr	Tc by User	= 23.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1
	-		

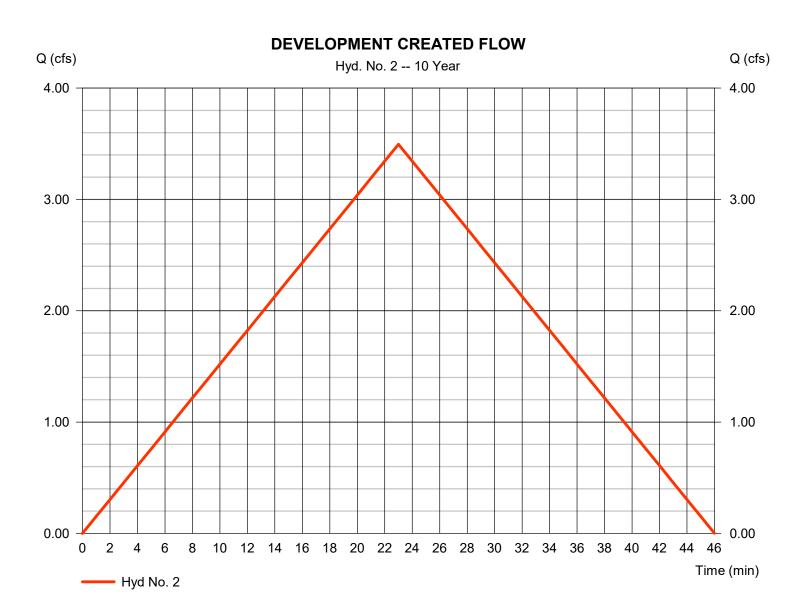


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Hyd. No. 2

DEVELOPMENT CREATED FLOW

Hydrograph type Storm frequency Time interval Drainage area Intensity IDF Curve	 Rational 10 yrs 1 min 1.170 ac 4.330 in/hr Bryant 50.IDF 	Time to peak Hyd. volume Runoff coeff.	= 3.495 cfs = 23 min = 4,823 cuft = 0.69 = 23.00 min = 1/1
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1



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Wednesday, 09 / 25 / 2024

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

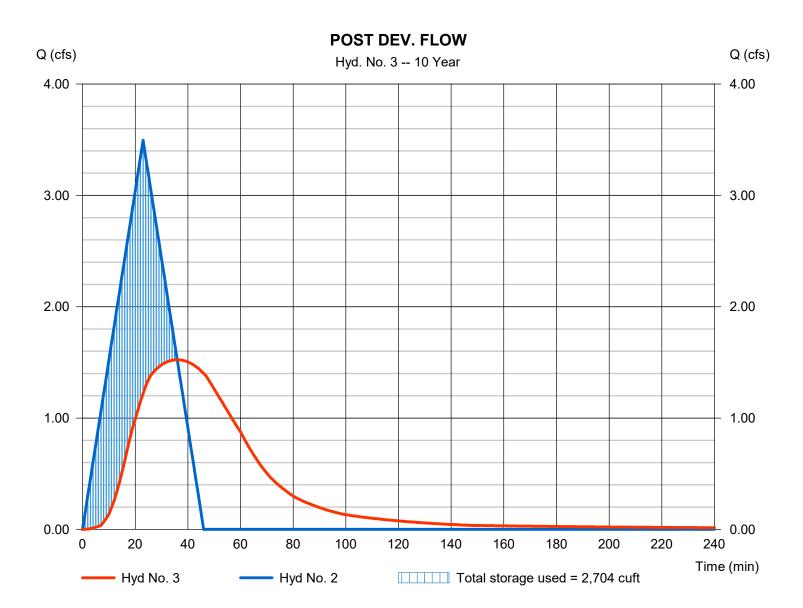
Wednesday, 09 / 25 / 2024

Hyd. No. 3

POST DEV. FLOW

Hydrograph type	= Reservoir	Peak discharge	= 1.524 cfs
Storm frequency	= 10 yrs	Time to peak	= 36 min
Time interval	= 1 min	Hyd. volume	= 4,817 cuft
Inflow hyd. No.	= 2 - DEVELOPMENT	CREATE Mak OEMevation	= 402.66 ft
Reservoir name	= DETENTION	Max. Storage	= 2,704 cuft

Storage Indication method used.



1 Ratonal 2.872 1 23 3.963 PRE DEV FLOW 2 Ratonal 4.044 1 23 5.581 DEVELOPMENT CREATER 3 Reservoir 1.643 1 37 5.575 2 402.80 3.207 POST DEV. FLOW	Hyd. Hydrograph No. type (origin)	n Peak Time flow interva (cfs) (min)	erval Peak volume	Inflow Maximum hyd(s) elevation (ft)	Total strge used (cuft)	Hydrograph Description
	1 Rational	2.872 1	1 23 3,963			PRE DEV FLOW
3 Reservoir 1.643 1 37 5.575 2 402.80 3.207 POST DEV. FLOW	2 Rational	4.044 1	1 23 5,581			DEVELOPMENT CREATED FLOW
	3 Reservoir	1.643 1	1 37 5,575	2 402.80	3,207	POST DEV. FLOW

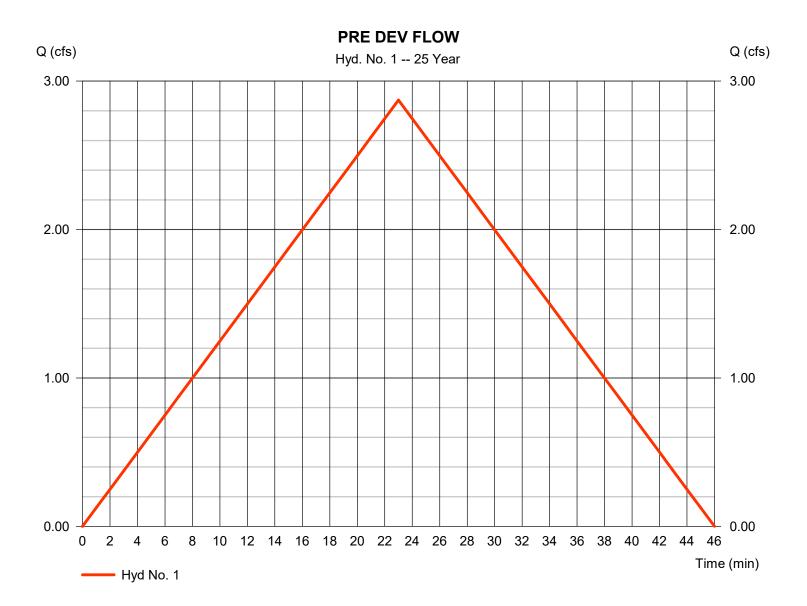
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Wednesday, 09 / 25 / 2024

Hyd. No. 1

PRE DEV FLOW

Hydrograph type	= Rational	Peak discharge	= 2.872 cfs
Storm frequency	= 25 yrs	Time to peak	= 23 min
Time interval	= 1 min	Hyd. volume	= 3,963 cuft
Drainage area	= 1.170 ac	Runoff coeff.	= 0.49
Intensity	= 5.010 in/hr	Tc by User	= 23.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1
	-		

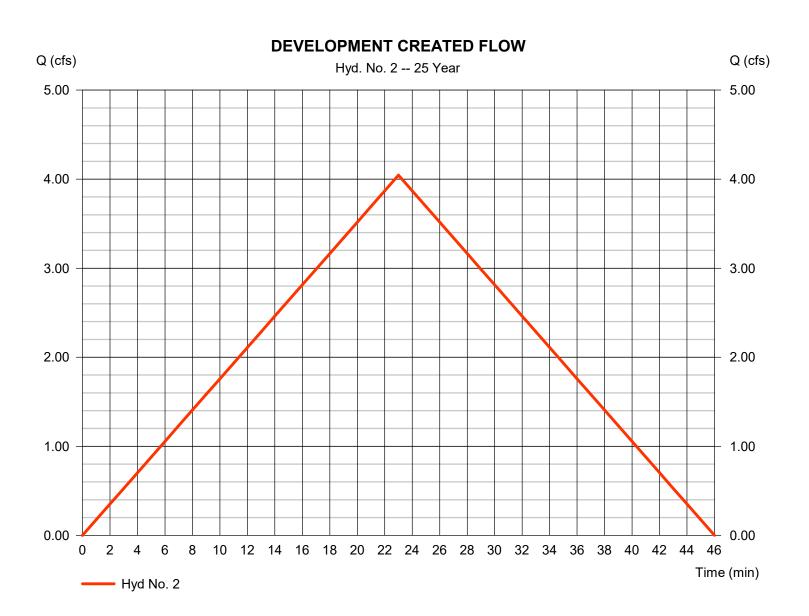


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Hyd. No. 2

DEVELOPMENT CREATED FLOW

IDF Curve= 5.010 In/InTC by Oser= 23.00 IninIDF Curve= Bryant 50.IDFAsc/Rec limb fact= 1/1	Hydrograph type Storm frequency Time interval Drainage area Intensity IDF Curve	 Rational 25 yrs 1 min 1.170 ac 5.010 in/hr Bryant 50.IDF 	Peak discharge Time to peak Hyd. volume Runoff coeff. Tc by User Asc/Rec limb fact	= 4.044 cfs = 23 min = 5,581 cuft = 0.69 = 23.00 min = 1/1
--	--	---	---	---



Wednesday, 09 / 25 / 2024

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

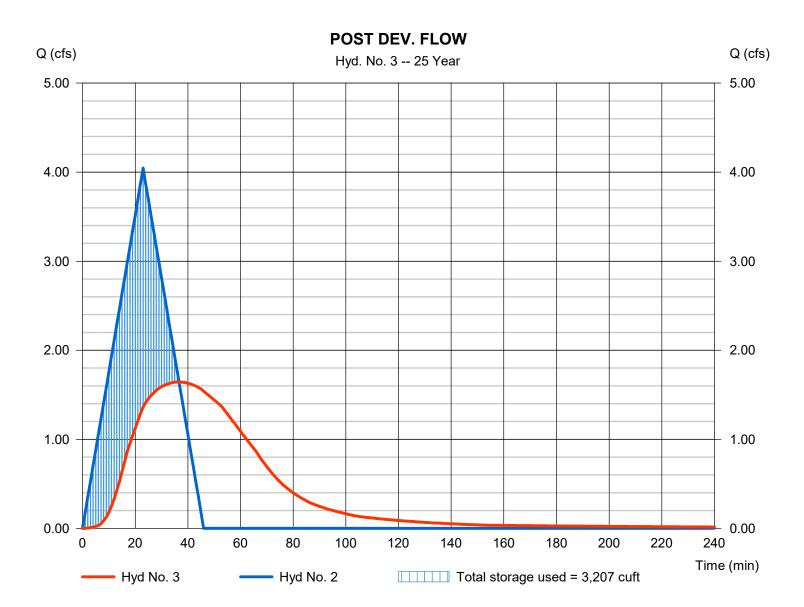
Wednesday, 09 / 25 / 2024

Hyd. No. 3

POST DEV. FLOW

Hydrograph type	= Reservoir	Peak discharge	= 1.643 cfs
Storm frequency	= 25 yrs	Time to peak	= 37 min
Time interval	= 1 min	Hyd. volume	= 5,575 cuft
Inflow hyd. No.	= 2 - DEVELOPMENT	CREATE Mak OEMevation	= 402.80 ft
Reservoir name	= DETENTION	Max. Storage	= 3,207 cuft

Storage Indication method used.



Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	3.262	1	23	4,501				PRE DEV FLOW
2	Rational	4.593	1	23	6,339				DEVELOPMENT CREATED FLOW
3	Reservoir	1.752	1	37	6,332	2	402.96	3,737	POST DEV. FLOW
19-	0066 Besser	nt Duplexe	es 09-25	5-2024.gp	w Return	Period: 50 `	Year	Wednesda	ay, 09 / 25 / 2024

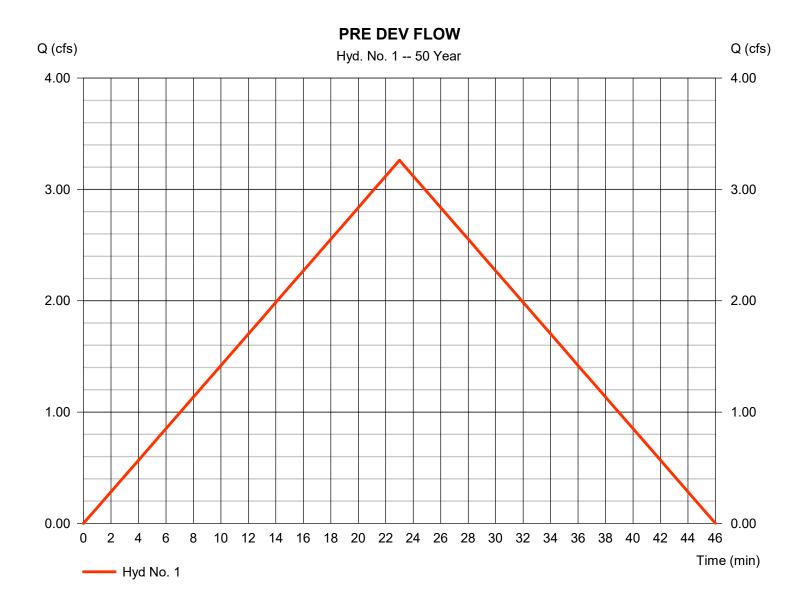
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Wednesday, 09 / 25 / 2024

Hyd. No. 1

PRE DEV FLOW

Hydrograph type	= Rational	Peak discharge	= 3.262 cfs
Storm frequency	= 50 yrs	Time to peak	= 23 min
Time interval	= 1 min	Hyd. volume	= 4,501 cuft
Drainage area	= 1.170 ac	Runoff coeff.	= 0.49
Intensity	= 5.690 in/hr	Tc by User	= 23.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1
	-		

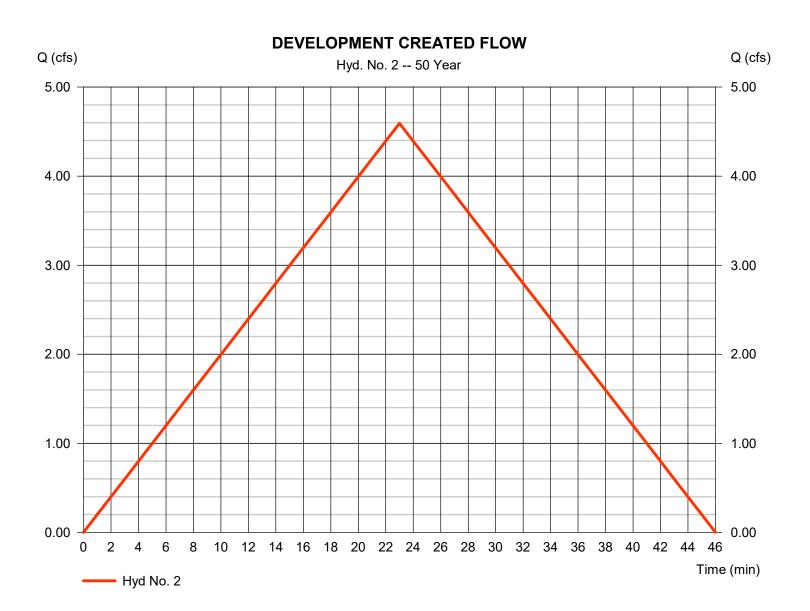


Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Hyd. No. 2

DEVELOPMENT CREATED FLOW

Hydrograph type Storm frequency Time interval Drainage area Intensity	 Rational 50 yrs 1 min 1.170 ac 5.690 in/hr Bryant 50 IDE 	Peak discharge Time to peak Hyd. volume Runoff coeff. Tc by User Asc/Rec limb fact	 = 4.593 cfs = 23 min = 6,339 cuft = 0.69 = 23.00 min = 1/1
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1



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Wednesday, 09 / 25 / 2024

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

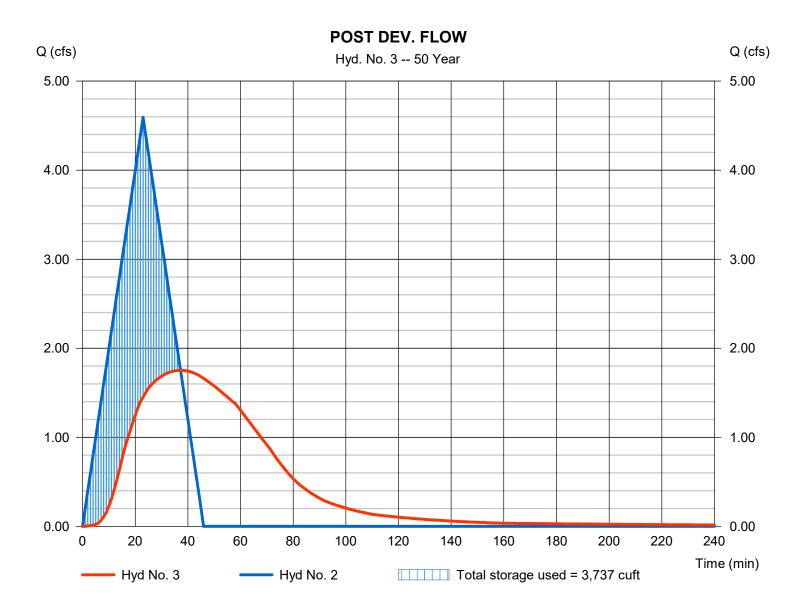
Wednesday, 09 / 25 / 2024

Hyd. No. 3

POST DEV. FLOW

Hydrograph type	= Reservoir	Peak discharge	= 1.752 cfs
Storm frequency	= 50 yrs	Time to peak	= 37 min
Time interval	= 1 min	Hyd. volume	= 6,332 cuft
Inflow hyd. No.	= 2 - DEVELOPMENT	CREATE Mak OM evation	= 402.96 ft
Reservoir name	= DETENTION	Max. Storage	= 3,737 cuft

Storage Indication method used.



lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	3.493	1	23	4,821				PRE DEV FLOW
2	Rational	4.919	1	23	6,788				DEVELOPMENT CREATED FLOW
3	Reservoir	1.815	1	38	6,782	2	403.06	4,058	POST DEV. FLOW

Hydrograph Report

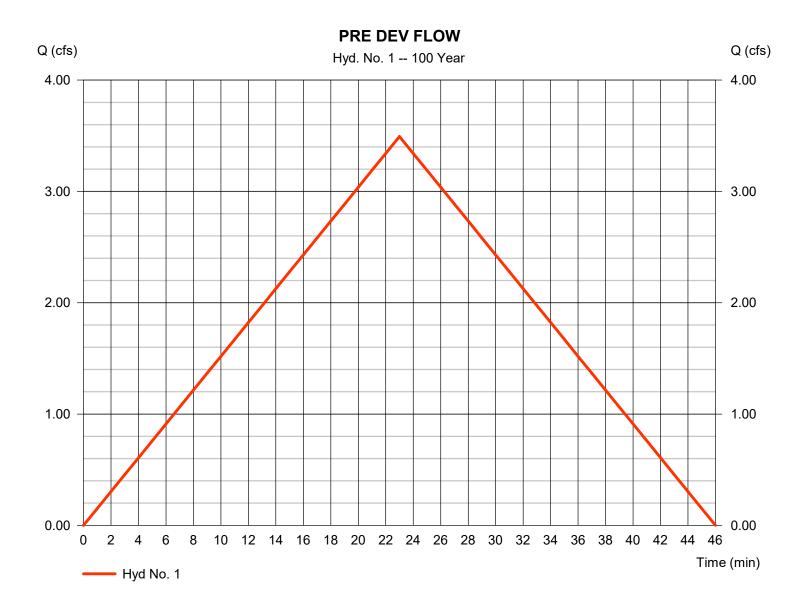
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Wednesday, 09 / 25 / 2024

Hyd. No. 1

PRE DEV FLOW

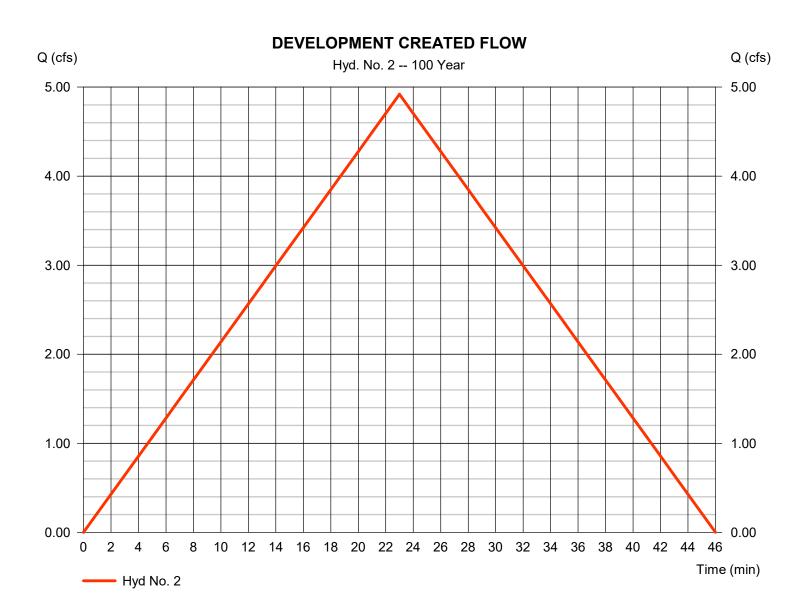
Hydrograph type	= Rational	Peak discharge	= 3.493 cfs
Storm frequency	= 100 yrs	Time to peak	= 23 min
Time interval	= 1 min	Hyd. volume	= 4,821 cuft
Drainage area	= 1.170 ac	Runoff coeff.	= 0.49
Intensity	= 6.093 in/hr	Tc by User	= 23.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1
	-		



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Hyd. No. 2

DEVELOPMENT CREATED FLOW



Wednesday, 09 / 25 / 2024

Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

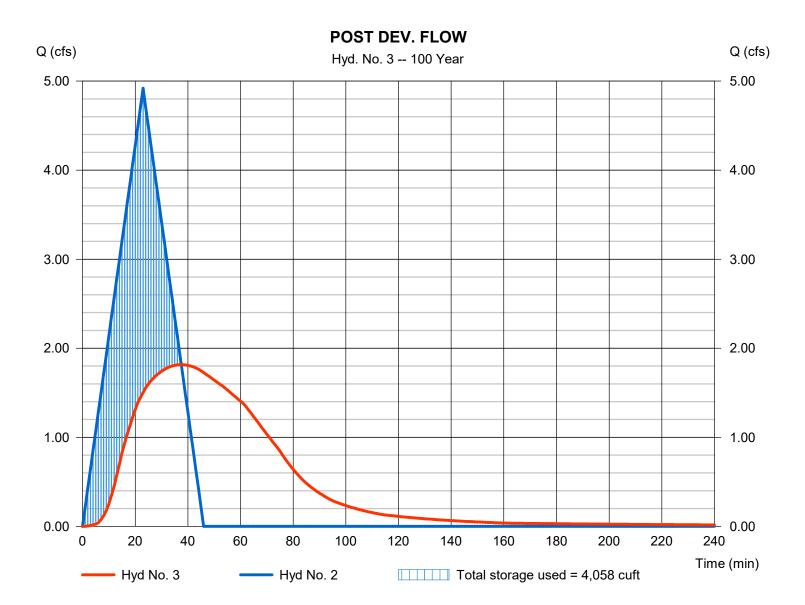
Wednesday, 09 / 25 / 2024

Hyd. No. 3

POST DEV. FLOW

Hydrograph type	= Reservoir	Peak discharge	= 1.815 cfs
Storm frequency	= 100 yrs	Time to peak	= 38 min
Time interval	= 1 min	Hyd. volume	= 6,782 cuft
Inflow hyd. No.	= 2 - DEVELOPMENT	CREATE DME & OEMevation	= 403.06 ft
Reservoir name	= DETENTION	Max. Storage	= 4,058 cuft

Storage Indication method used.



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Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2025

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)							
(Yrs)	В	D	E	(N/A)					
1	0.0000	0.0000	0.0000						
2	32.2253	7.2000	0.6856						
3	0.0000	0.0000	0.0000						
5	0.0000	0.0000	0.0000						
10	46.3641	10.0000	0.6781						
25	61.8249	11.8000	0.7079						
50	79.0516	13.3000	0.7326						
100	54.7483	10.0000	0.6279						
	1	1	1	1					

File name: Bryant 50.IDF

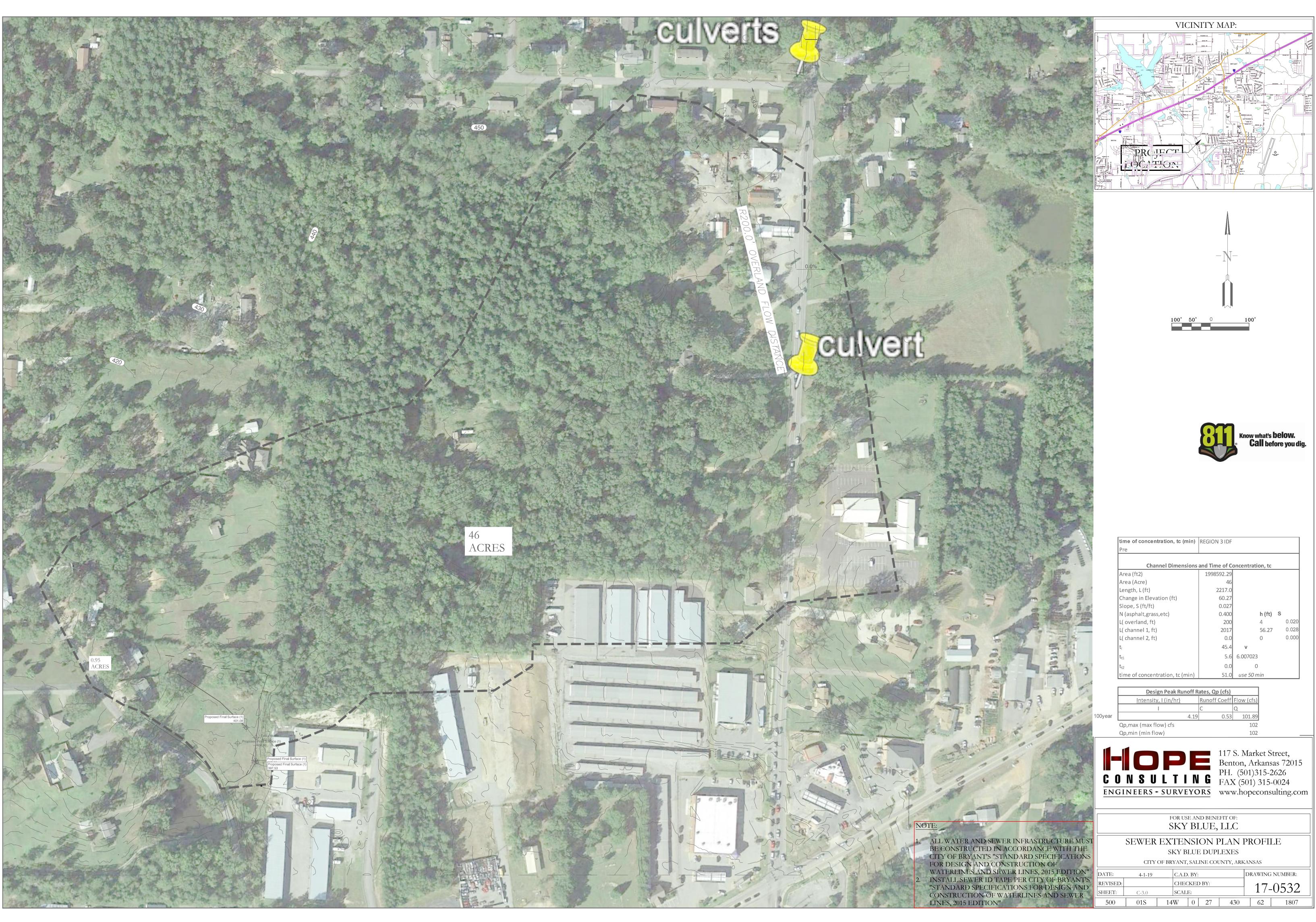
Intensity = B / (Tc + D)^E

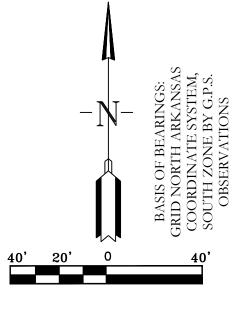
Return	Intensity Values (in/hr)											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.80	4.58	3.85	3.35	2.98	2.70	2.48	2.29	2.14	2.01	1.90	1.80
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
10	7.39	6.08	5.23	4.62	4.16	3.80	3.51	3.27	3.06	2.89	2.73	2.60
25	8.39	6.98	6.03	5.34	4.82	4.40	4.06	3.78	3.54	3.34	3.16	3.00
50	9.40	7.87	6.83	6.06	5.47	5.00	4.62	4.29	4.02	3.79	3.58	3.40
100	10.00	8.34	7.25	6.47	5.87	5.40	5.02	4.69	4.42	4.19	3.98	3.80

Tc = time in minutes. Values may exceed 60.

		Rainfall Precipitation Table (in)						
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Precip. file name: C:\Documents and Settings\Will\Desktop\Fleming\fleming.pcp





DETENTION POND MAINTENANCE PLAN

Background

The detention pond is located just beyond the northeast corner of the property. The modifications are designed to temporarily detain stormwater to meet the City of Bryant's water quantity criteria before discharging from the pond.

Routine Maintenance

Routine maintenance will include but not be limited to:

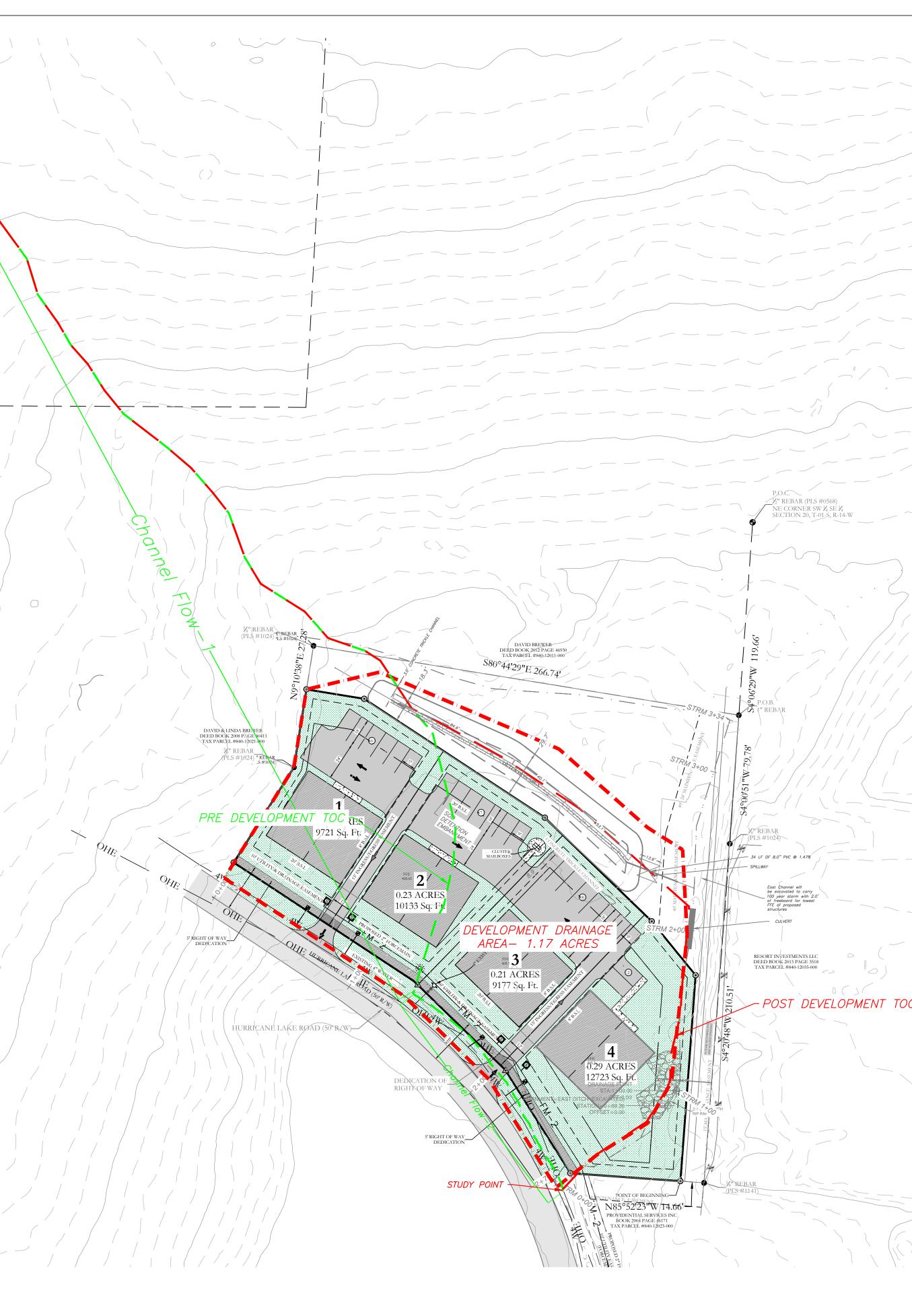
- -The primary discharge (v-notch weir) from the pond and other areas will be inspected monthly for debris which could inhibit the proper flow of discharge. Any debris will be removed immediately and disposed of or placed in a location to prevent future maintenance and to not cause impact up or downstream of the structure.
- -Trash will be removed from around the pond to prevent entering the pond. Generally, the site should be kept free of loose trash which could be carried off site by wind or rain.
- -Inspect the pond and discharge weir for non-routine maintenance need.

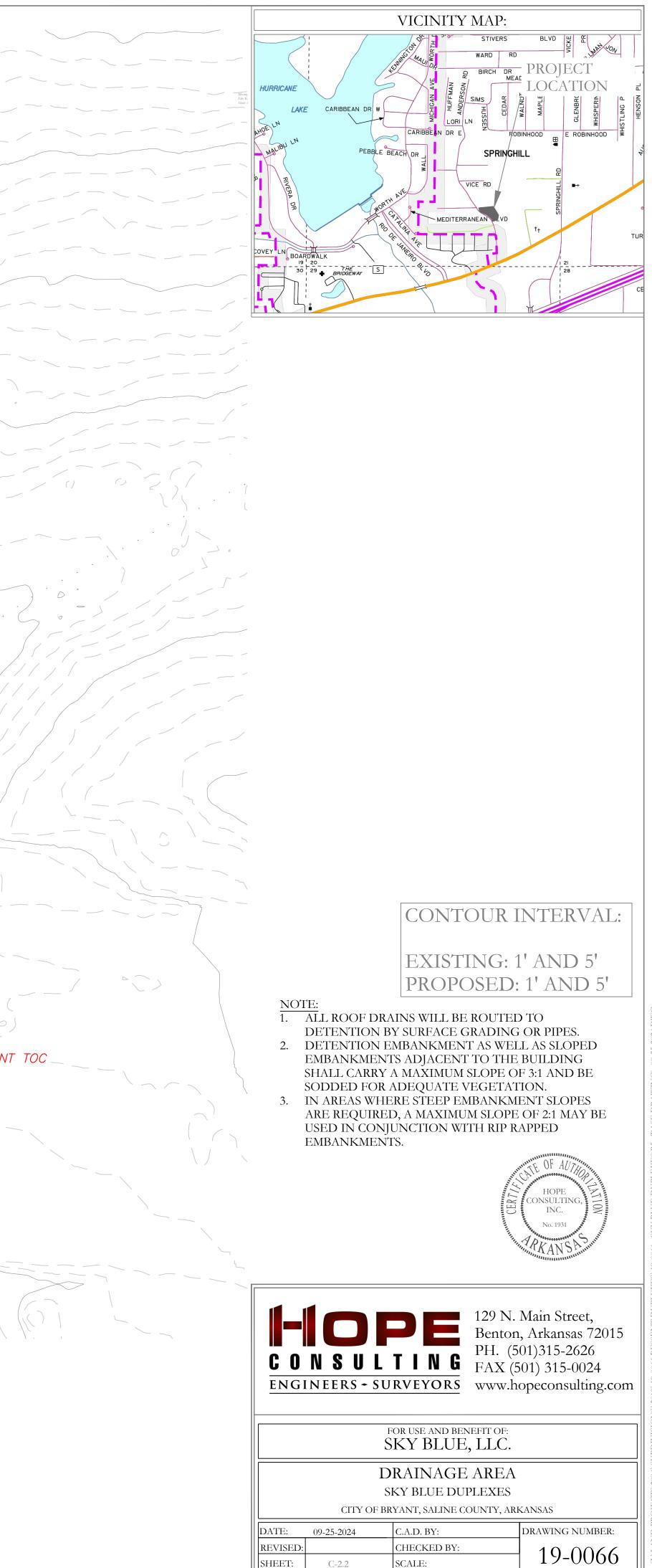
Periodic or Non-Routine Maintenance

The routine inspection of the pond area and discharge weir will identify needed repairs and non-routine maintenance. These items may include but not be limited to:

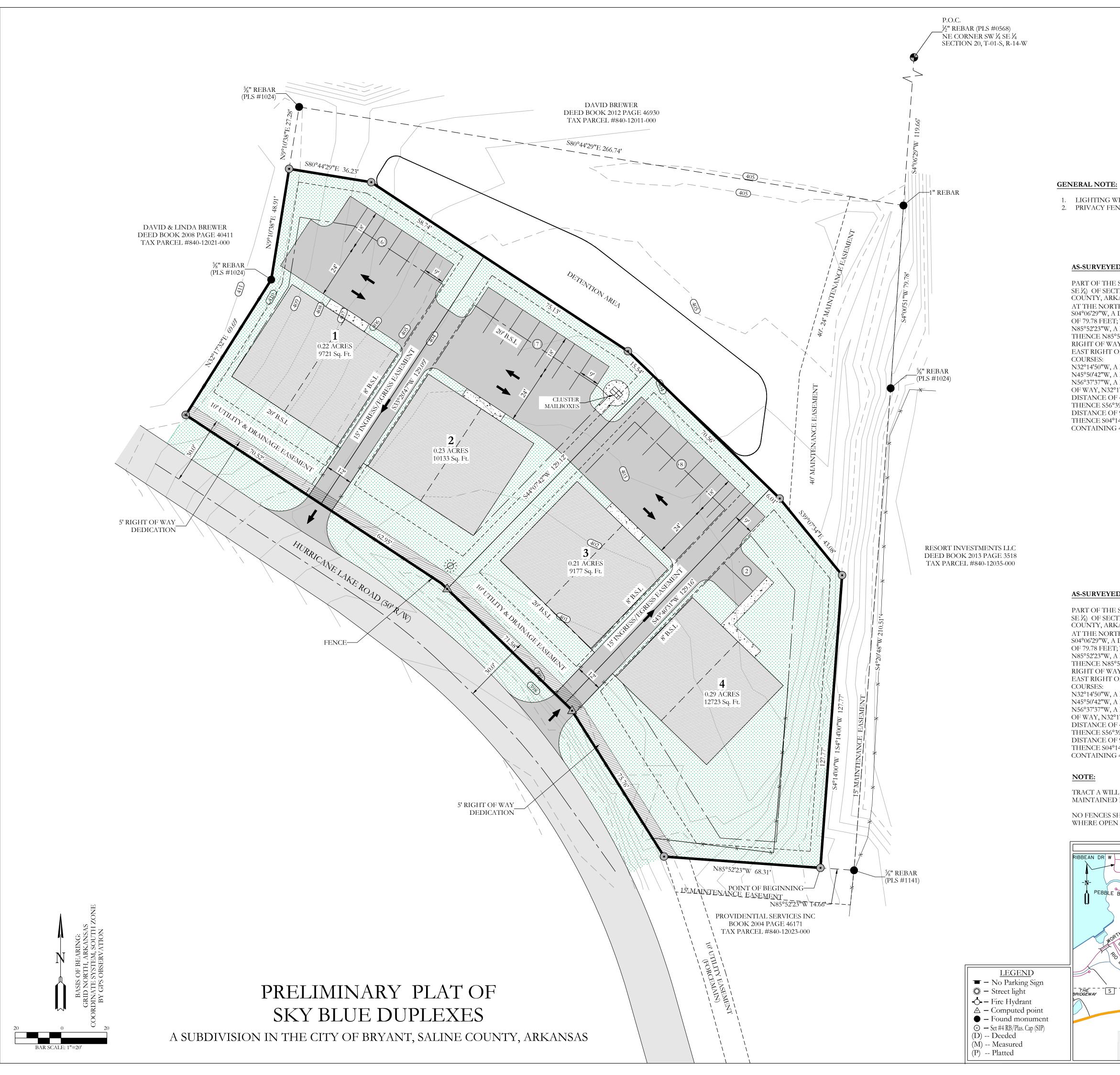
- -Bottom of pond will be sodded (except where trickle channel is located). -Embankments sloped 2:1 will be rip rapped, 3:1 slopes shall be sodded
- -Re-growth of trees on or around the pond bank. These should be cut and removed from the pond area.
- -Stabilization of slopes may be required periodically or after excessive rain events. Any disturbance of slopes should be reseeded or may require installation of erosion control materials until seeding can reestablish adequate grasses to prevent future erosion. -Any other maintenance or repairs which would minimize other maintenance to the pond or outfall structures.

For questions or concerns about the pond, contact _____ at 501-315-2626.

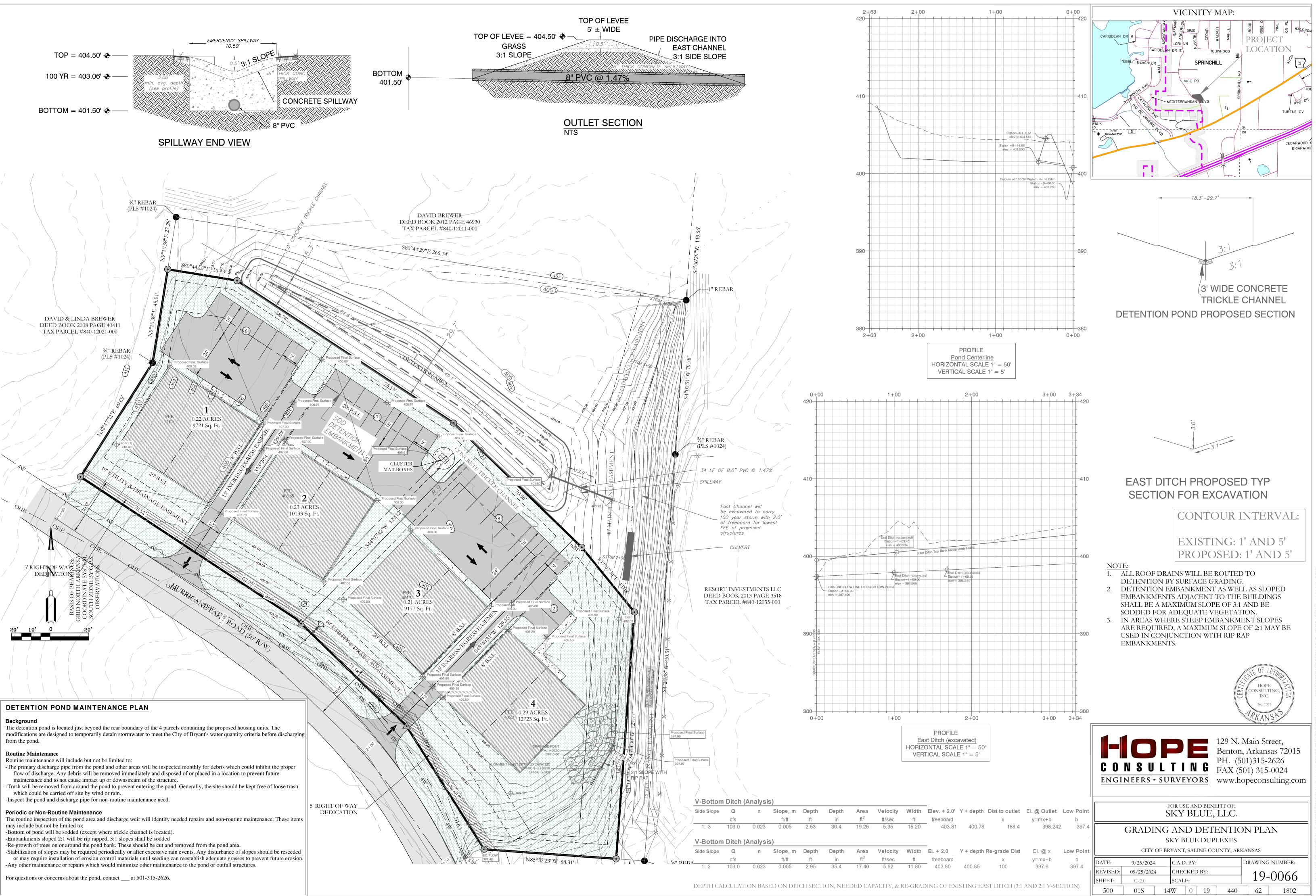


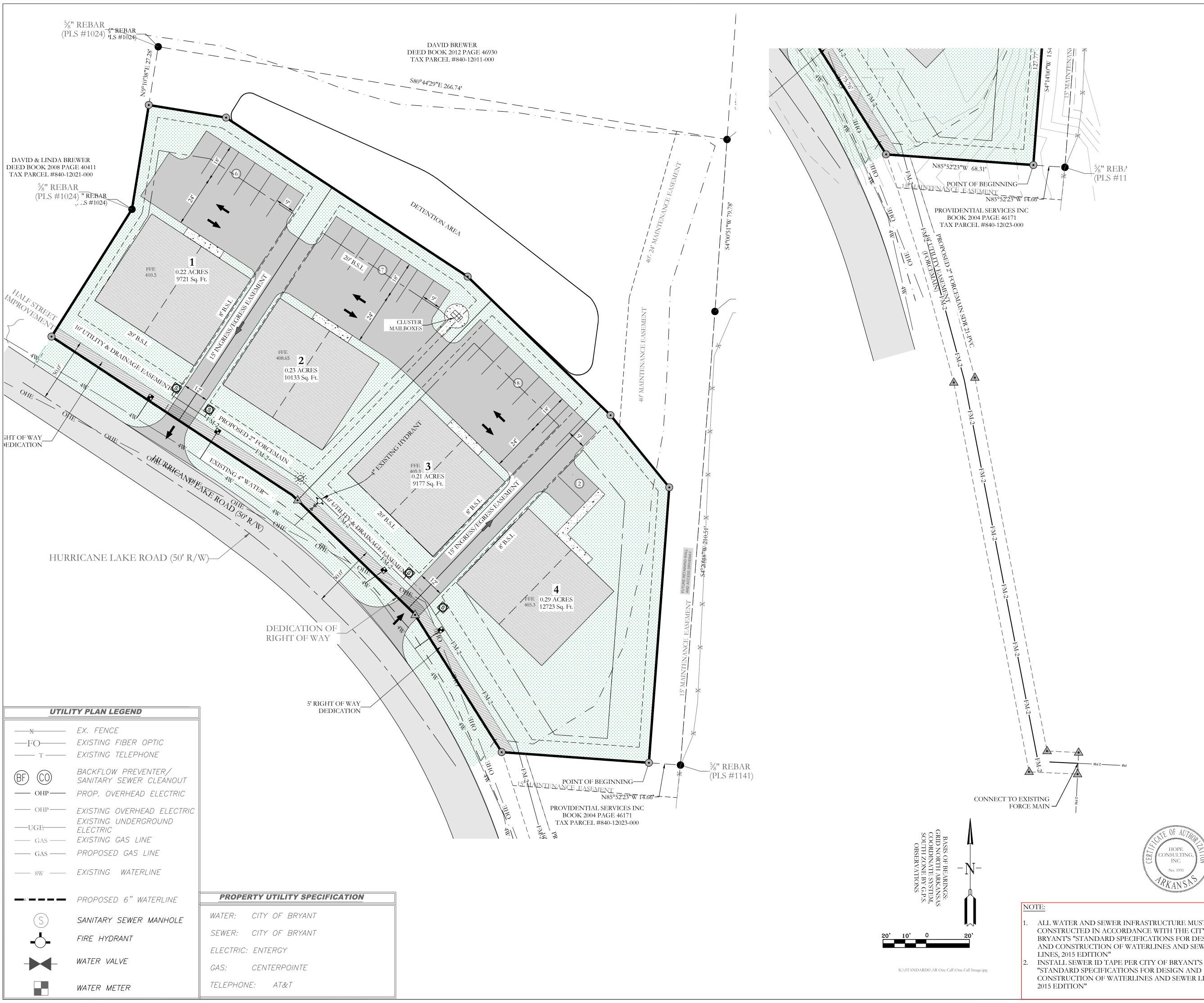


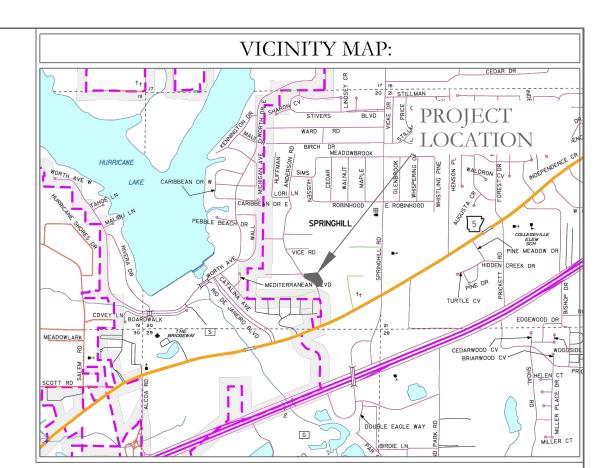
 500
 01S
 14W
 0
 19
 440
 62
 1802



	CERTIFICATIONS:
	OWNER: DEVELOPER: Name: SKY BLUE, LLC Address: 3621 INDEPEDENCE DRIVE DEVELOPER: Name: SKY BLUE, LLC Address: 3621 INDEPEDENCE DRIVE
	BRYANT, AR 72022 BRYANT, AR 72022 CERTIFICATE OF OWNER: We, the undersigned, owners of the real estate shown and described herein do hereby certify that we have caused to be laid off, platted and subdivided, and to hereby lay off, plat and subdivide said real estate in accordance with the plat.
	Date of Execution Name: Source of Tile: D.R. BOOK 2015 PAGE 7766
ILL BE ATTACHED TO THE BUILDING ON THE REAR SIDE. ICE WILL BE INSTALLED.	CERTIFICATE OF PROPERTY OWNERSHIP: I,, hereby certify that the deed records in the office of Circuit Clerk and Ex-Officio recorder of Saline County, Arkansas, reflect that is the record title owner of real property more particularly described herein on plat.
	Dated: Certified Title Insurance Agent or Abstractor
D DESCRIPTION: SOUTHWEST QUARTER OF THE SOUTHEAST QUARTER (SW ¼ ION 20, TOWNSHIP 01 SOUTH, RANGE 14 WEST, SALINE ANSAS; MORE PARTICULARLY DESCRIBED AS COMMENCING HEAST CORNER OF SAID SW ¼ SE ¼ OF SECTION 20; THENCE DISTANCE OF 119.66 FEET ; THENCE S04°00'51"W, A DISTANCE THENCE S04°20'48"W, A DISTANCE OF 210.51 FEET; THENCE DISTANCE OF 14.66 FEET TO THE POINT OF BEGINNING;	CERTIFICATE OF PRELIMINARY SURVEYING ACCURACY: I, William Corbitt R. Shoffner, hereby certify that this proposed prelminary plat correctly represents a survey completed by me or under my supervision on, 20; that the boundary lines show hereon correspond with the description in the deeds cited in the above Source of Title; and that all monuments which were found or placed on the property are correctly described and located.
52'23"W, A DISTANCE OF 68.31 FEET TO A POINT ON THE EAST Y LINE OF HURRICANE LAKE ROAD; THENCE ALONG SAID OF WAY LINE OF HURRICANE LAKE ROAD THE FOLLOWING	Date of Execution William Corbitt R. Shoffner Registered Professional Land Surveyor No. 1664 Arkansas
DISTANCE OF 75.76 FEET; THENCE DISTANCE OF 78.70 FEET; THENCE DISTANCE OF 133.47 FEET; THENCE LEAVING SAID RIGHT 7'32"E, A DISTANCE OF 64.69 FEET; THENCE N09°10'38"E, A 48.91 FEET; THENCE S80°44'29"E, A DISTANCE OF 36.23 FEET; 9'13"E, A DISTANCE OF 133.87 FEET; THENCE S45°52'18"E, A 92.11 FEET; THENCE S39°07'34"E, A DISTANCE OF 43.08 FEET; 4'00"W A DISTANCE OF 127.77TO THE POINT OF BEGINNING. 41,754 SQUARE FEET, OR 0.96 ACRES, MORE OR LESS.	CERTIFICATE OF PRELIMINARY ENGINEERING ACCURACY: I, Kazi Islam, hereby certify that this plat correctly represents a plan made by me, and that the engineering requirements of the City of Bryant Subdivision Rules and Regulations have been complied with.
	Date of Execution Kazi Islam Registered Professional Engineer, No. 20876 Arkansas
	CERTIFICATE OF PRELIMINARY PLAT APPROVAL: Pursuant to the City of Bryant Subdivision Rules and Regulations, and all of the conditions of approval having been completed, this document is hereby accepted. This certificate is hereby executed under the authority of said rules and regulations.
	Date of Execution NAME, CHAIRMAN BRYANT PLANNING COMMISSION
DESCRIPTION: SOUTHWEST QUARTER OF THE SOUTHEAST QUARTER (SW ¼ ION 20, TOWNSHIP 01 SOUTH, RANGE 14 WEST, SALINE ANSAS; MORE PARTICULARLY DESCRIBED AS COMMENCING HEAST CORNER OF SAID SW ¼ SE ¼ OF SECTION 20; THENCE DISTANCE OF 119.66 FEET ; THENCE S04°00'51"W, A DISTANCE THENCE S04°20'48"W, A DISTANCE OF 210.51 FEET; THENCE DISTANCE OF 14.66 FEET TO THE <u>POINT OF BEGINNING</u> ; 52'23"W, A DISTANCE OF 68.31 FEET TO A POINT ON THE EAST Y LINE OF HURRICANE LAKE ROAD; THENCE ALONG SAID OF WAY LINE OF HURRICANE LAKE ROAD THE FOLLOWING DISTANCE OF 75.76 FEET; THENCE DISTANCE OF 78.70 FEET; THENCE DISTANCE OF 133.47 FEET; THENCE LEAVING SAID RIGHT	By affixing my seal and signature, I Kazi Islam PE No, 20876, hereby certify that this drawing correctly depicts a survey compiled under my supervision.
7'32"E, A DISTANCE OF 64.69 FEET; THENCE N09°10'38"E, A 48.91 FEET; THENCE S80°44'29"E, A DISTANCE OF 36.23 FEET; 9'13"E, A DISTANCE OF 133.87 FEET; THENCE S45°52'18"E, A 92.11 FEET; THENCE S39°07'34"E, A DISTANCE OF 43.08 FEET; 4'00"W A DISTANCE OF 127.77TO THE POINT OF BEGINNING.	search. No portion of the property described hereon lies within the 100 year floodplain, according to the Federal Insurance Rate Map, panel # <u>05125C0360E</u> , Dated: <u>06/05/2020</u> .
41,754 SQUARE FEET, OR 0.96 ACRES, MORE OR LESS. , BE UTILIZED AS DRAINAGE AND UTILITY EASEMENTS BY THE PROPERTY OWNERS ASSOCIATION. HALL BE CONSTRUCTED IN THE DRAINAGE EASEMENT DITCHES ARE CONSTRUCTED.	PROPERTY SPECIFICATIONS: OWNER: SKY BLUE, LLC 3261 INDEPEDENCE DRIVE AVERAGE LOT SIZE: 0.19 ACRES (38,437 SQ. FT.) MINIMUM LOT SIZE: (7,200SQ. FT) NUMBER OF LOTS: 4 SOURCE OF WATER: WATER USERS SUBDIVIDER 3621 INDEPEDENCE DRIVE BRYANT, AR 72022 SOURCE OF WATER: WATER USERS SUBDIVIDER 3621 INDEPEDENCE DRIVE BRYANT, AR 72022 SOURCE OF SEWER: CITY OF BRYANT SOURCE OF ELECTRIC: ENTERGY BUILDING SETBACKS: FRONT-20' OR AS SHOWN REAR-20' OR AS SHOWN NAME OF SUBDIVISION: SKY BLUE DUPLEXES UTILITY & DRAINAGE EASEMENTS: INSTRUMENT # 2015-7766 UTILITY & DRAINAGE EASEMENTS: ZONING: R-X SIDE - 5' OR AS SHOWN
VICINITY MAP	
BEACH DR SPRINGHE PROJECT LOCATION	K:\STANDARDS\Hope Logo\HOPE-dwg.png 129 N. Main Street, Benton, Arkansas 72015 K:\STANDARDS\Hope Logo\HOPE-dwg.png PH. (501)315-2626 FAX (501) 315-0024 www.hopeconsulting.com
	FOR USE AND BENEFIT OF: SKY BLUE, LLC
	PRELIMINARY PLAT SKY BLUE DUPLEXES A SUBDIVISION IN THE CITY OF BRYANT, SALINE COUNTY, ARKANSAS DATE: 09/16/2024 C.A.D. BY: B.JOHNSON DRAWING NUMBER:
	REVISED: CHECKED BY: 19-0066 SHEET: C-1.0 SCALE: 1"=20' 500 01S 14W 0 20 230 62 1807







ALL WATER AND SEWER INFRASTRUCTURE MUST B CONSTRUCTED IN ACCORDANCE WITH THE CITY OF BRYANT'S "STANDARD SPECIFICATIONS FOR DESIGN AND CONSTRUCTION OF WATERLINES AND SEWER

INSTALL SEWER ID TAPE PER CITY OF BRYANT'S "STANDARD SPECIFICATIONS FOR DESIGN AND "STANDARD SPECIFICATIONS FOR DESIGN AND CONSTRUCTION OF WATERLINES AND SEWER LINES, SHEE



FOR USE AND BENEFIT OF: SKY BLUE, LLC.										
UTILITY PLAN SKY BLUE DUPLEXES city of bryant, saline county, arkansas										
ſE:	06/26/2024	1	C.A.D. BY:				DR	AWING	NUMBER:	٦
/ISED:	09/25/2024	024 CHECKED BY:					10	0066		
CHECKED BY: 19-0066 EET: C-3.0 SCALE: 19-0066					0000					
500	01S	14	W	0	27	430		62	1807	



September 12, 2024

Colton Leonard City of Bryant 210 Southwest Third St., Bryant, AR 72022

RE: Request for Modification from Code and Request for CUP (Parcel #:840-12022-000)

Dear Mr. Leonard,

We are proposing duplexes on the 4 lots of this proposed subdivision. I am also requesting a modification from the Walk Bike Drive Code for no curb improvements on this proposed development. We are also asking for a waiver on sidewalks and half-street improvements.

It is our goal to be included on the October 14, 2024 Planning Commission agenda.

Please feel free to contact me with any questions or concerns or if I can be of any further assistance. Sincerely,

onathan

Jonathan Hope ' Hope Consulting, Inc.

129 N. Main St. Benton, Arkansas 72015 501-315-2626 www.hopeconsulting.com



September 25, 2024

Tim Fournier Director of Public Works 210 SW 3rd St Bryant, AR 72022

Re: Midtown Phase 3 Detailed Cost Estimate for Water/Sewer, Streets, and Storm water Bonds

ł

Dear Tim,

Hope Consulting has reviewed the project with the owner and the cost associated costs with the Utility Construction, Street Construction, and Storm water Construction.

1. Streets (1 Year Bond: 25% of the Total Cost)

- Total Street Costs on this project was \$398,994.4
- Bond Cost \$99,748.60

2. Sewer (2 Year Bond: 50% of the Total Cost)

- Total Utility Costs on this project was \$27,620
- Bond Cost \$\$13,810.00

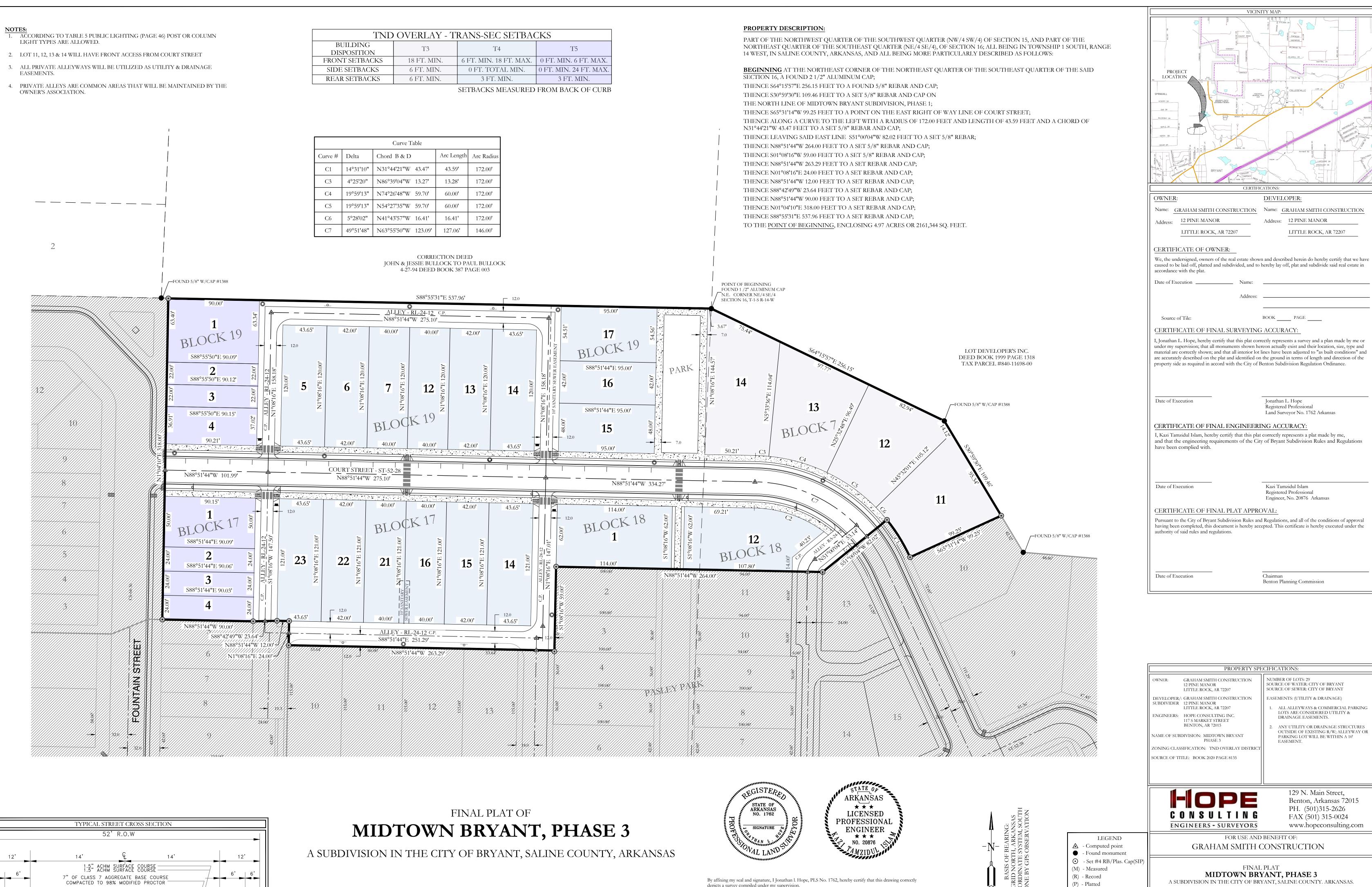
3. Storm water (1 Year Bond: 100% of the Total Cost)

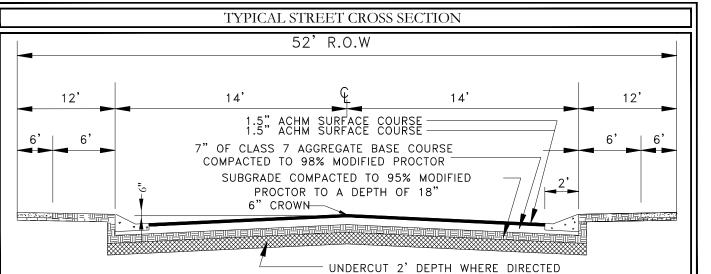
- Total Storm water Costs on this project was \$170,997.60
- Bond Cost \$\$170,997.60

Please do not hesitate to contact us if you have any questions or require additional information.

Sincerely,

Jonathan Hope











depicts a survey compiled under my supervision.

NOTE: This survey was based on legal descriptions and title work furnished by others and does not represent a title search.

According the the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) for Saline County unincorporated areas, panel # $\underline{05125C0225D}$, dated $\underline{06/19/2012}$, NO portion of the property described hereon does lie within the 100 year flood hazard boundary.

• - Fire Hydrant d - No Parking Sign Stop Sign C.P. - Common Place

🕉 - Street Light

C.A.D. BY: B.JOHNSON DRAWING NUMBER: 09-20-2024 DATE: CHECKED BY: 07-0032 REVISED SCALE: 1"=40' 14W 0 15/16 210/340 62 1762 500 1S



September 25, 2024

Colton Leonard Colton Leonard City of Bryant 210 Southwest Third St., Bryant, AR 72022

RE: Midtown Phase 3 Final Plat Hope Job #22-0497

Dear Mr. Leonard:

Please find the attached Final Plat of Midtown Phase 3 for review. We are currently working through the construction numbers with the contractor finalizing the bond amounts for the roads and utilities. We should have those letters prepared for Ted Taylor to review soon.

Please feel free to contact me with any questions or concerns or if I can be of any further assistance.

Respectfully Submitted,

HOPE CONSULTING

onathan

Jonathan Hope, PS \ President

Use table below to enter information regarding each sign for approval. Please use each letter to reference each sign rendering in packet.

SIGN	Type (Façade, Pole, Monument, other)	Dimensions (Height, Length, Width)	Sqft (Measurement standards found on	Façade Width (Linear Ft of building façade where wall		eight
			Pg.7 of Sign Code)	sign is being installed)	То Тор	To Bottom
A	WALL	42" + 151"	44	50'	15'	11.5'
В						
с						
D						
E						
F				1		

Existing Sign ON BUILDING IS 4'x14'



1231 Central Avenue Hot Springs, AR 71901 (© (501) 623-3181 () seizsigns.com

Job Info

Date8/20/24Job #Ronny SkipperSalespersonRonny SkipperDesignerScott Telferemailscott@seizsigns.com

Client/file name

Rookh Rookh_fusion food sign_PROOF3

Spe	cifica	tions
Quantity	1 ea	SFX_DF
Substrate		
Material		
Color(s)		
Laminate		
Equipment		

Notes



 The client is responsible for content accuracy. Please review the text, dimensions, and layout carefully.
 Colors are representative only. There are variations in color between computer monitors, desktop printers, and sign prints.
 All designs presented and represented in this drawing (except registered trademarks) are the sole property of Seiz Sign Company, and may not be reproduced in part or whole without written permission from Seiz Sign Company. The rights thereof are protected by law. 151 in

Italian+Indian Restaurant+Bar

Digitally printed banner with grommets



DATE

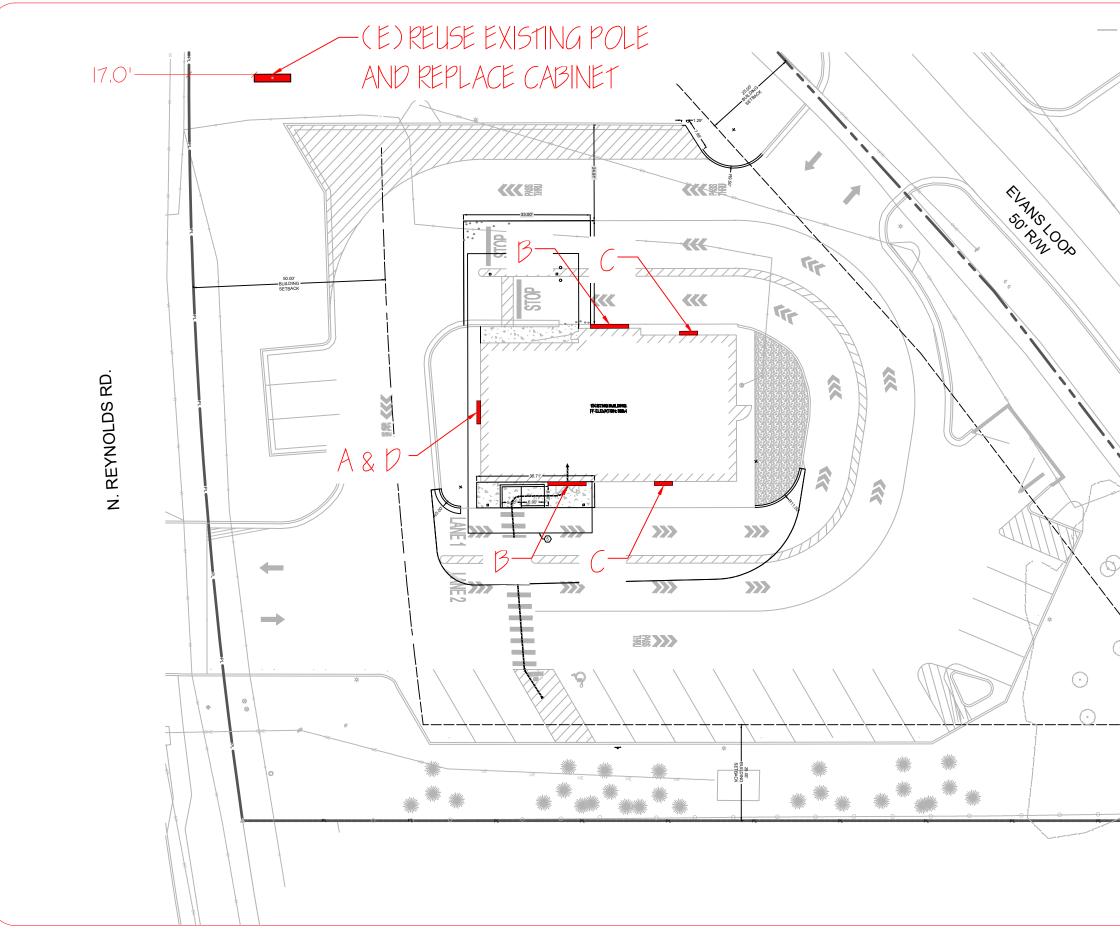
.⊇

42

By signing you agree that all artwork is correct and give Seiz Sign Company permission to begin production.





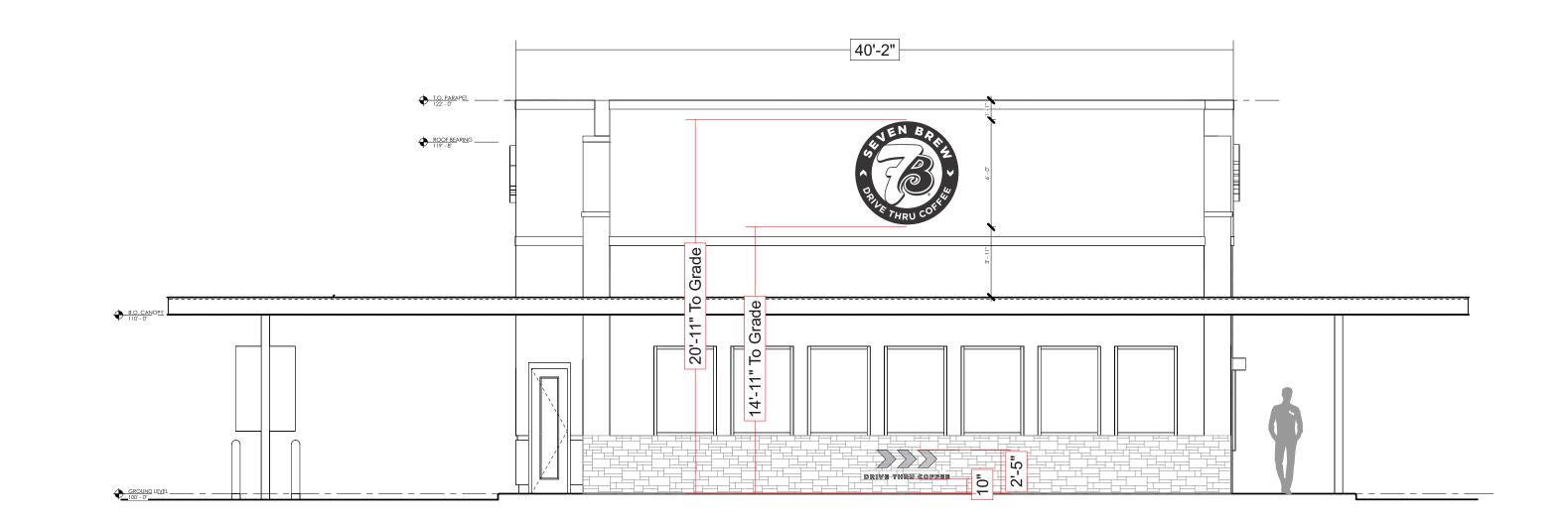


Incruting and the second	SPRINGFIELD SIGN 4825 E. Kearney St. Springfield, MO 65803 (417) 862-2454
	Project Number: 5525 7 BREW Dawn By: CLH Reviewed By: MW Date: 8-16-24 BRYANT, AR. 72022 Steet Number: 10F 1 SIGN PLACEMENT PLAN
SCALE 1" =25'	Project Number Drawn By: Reviewed By: Date: 8 Sheet Number:





SCALE: 3/16"=1'



SPRINGFIELDSIGN.com

AUTHORIZED SIGNATURE:

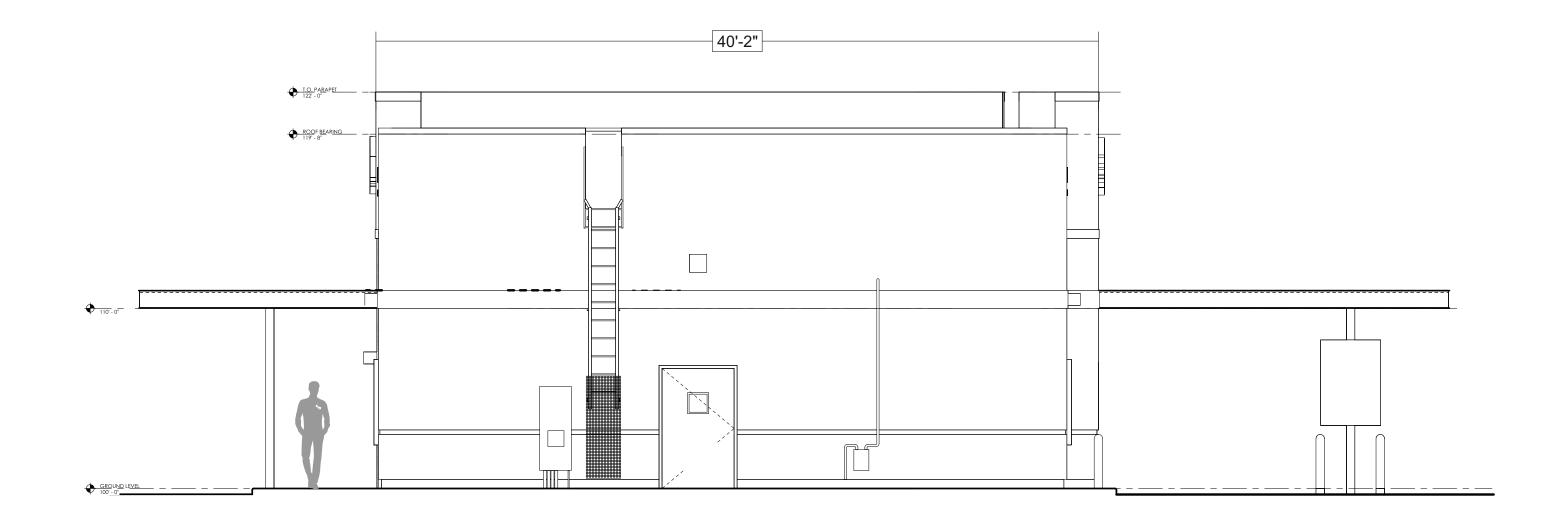
This drawing is copyrighted material. It remains the property of Springfield Sign unless otherwise agreed upon in writing. It is unlawful to use this drawing for bidding purposes, nor can it be reproduced, copied, or used in the production of a sign without the written permission from Springfield Sign, Inc. This is an artistic rendition and final colors/sizes may vary from that depicted herein.







SCALE: 3/16"=1'



SPRINGFIELDSIGN.com

AUTHORIZED SIGNATURE:

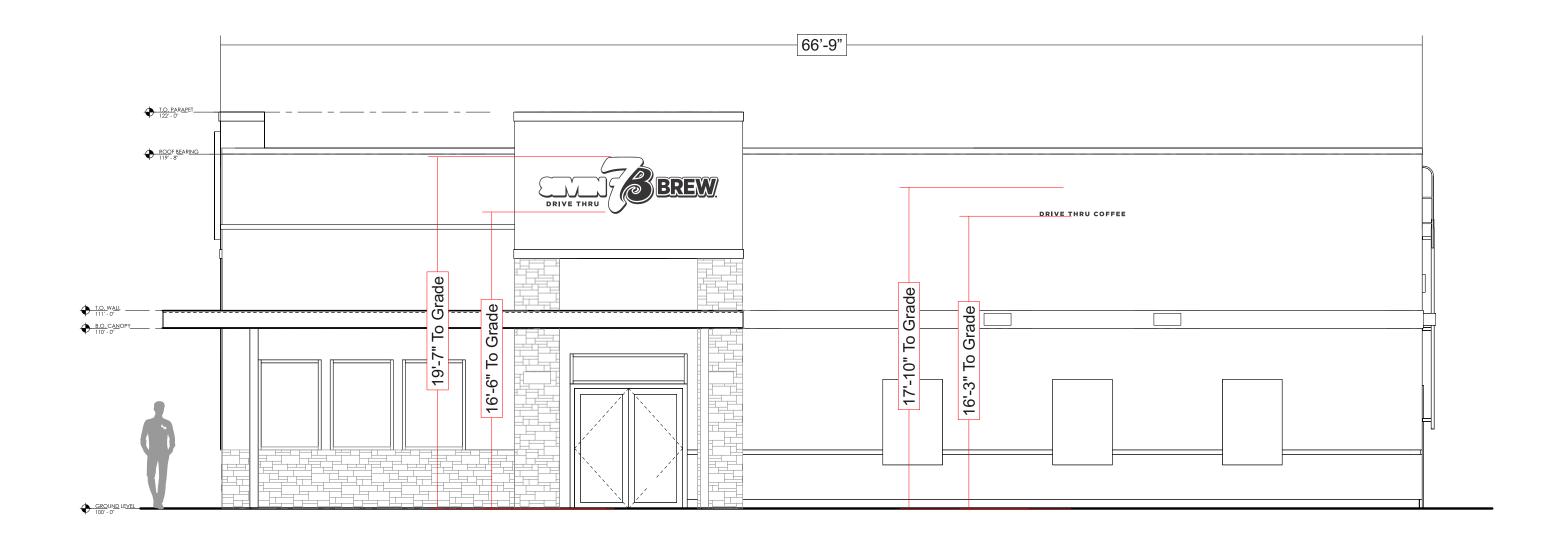
Elevation Sq. Ft. 883.7

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SCALE: 3/16"=1'



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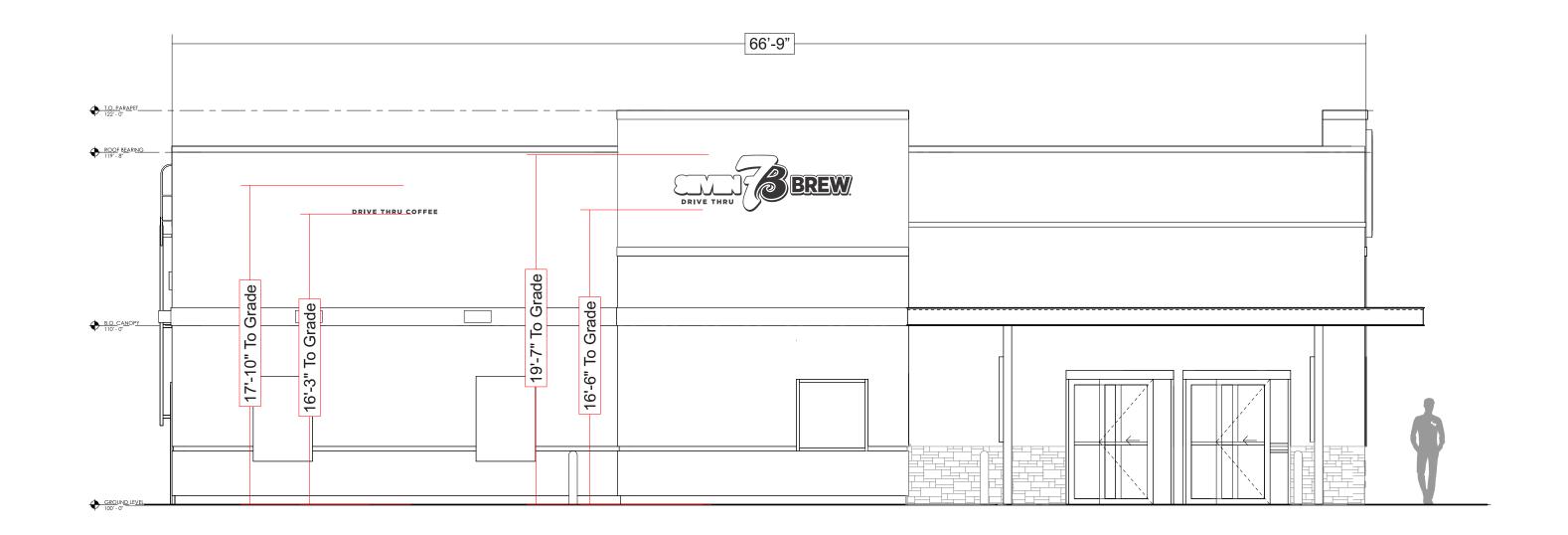
ELEVATION

SPRINGFIELD**SIGN**.com

SALES:Shaun CrawfordImage: Solution SolutionCLIENT:7Brew#ARTIST:Joshua KroegerCREATED: 4/29/24LOCATION:2006 N Reynolds Dr Bryant, AR 72022

EXTERIOR

SCALE: 3/16"=1'



AUTHORIZED SIGNATURE:

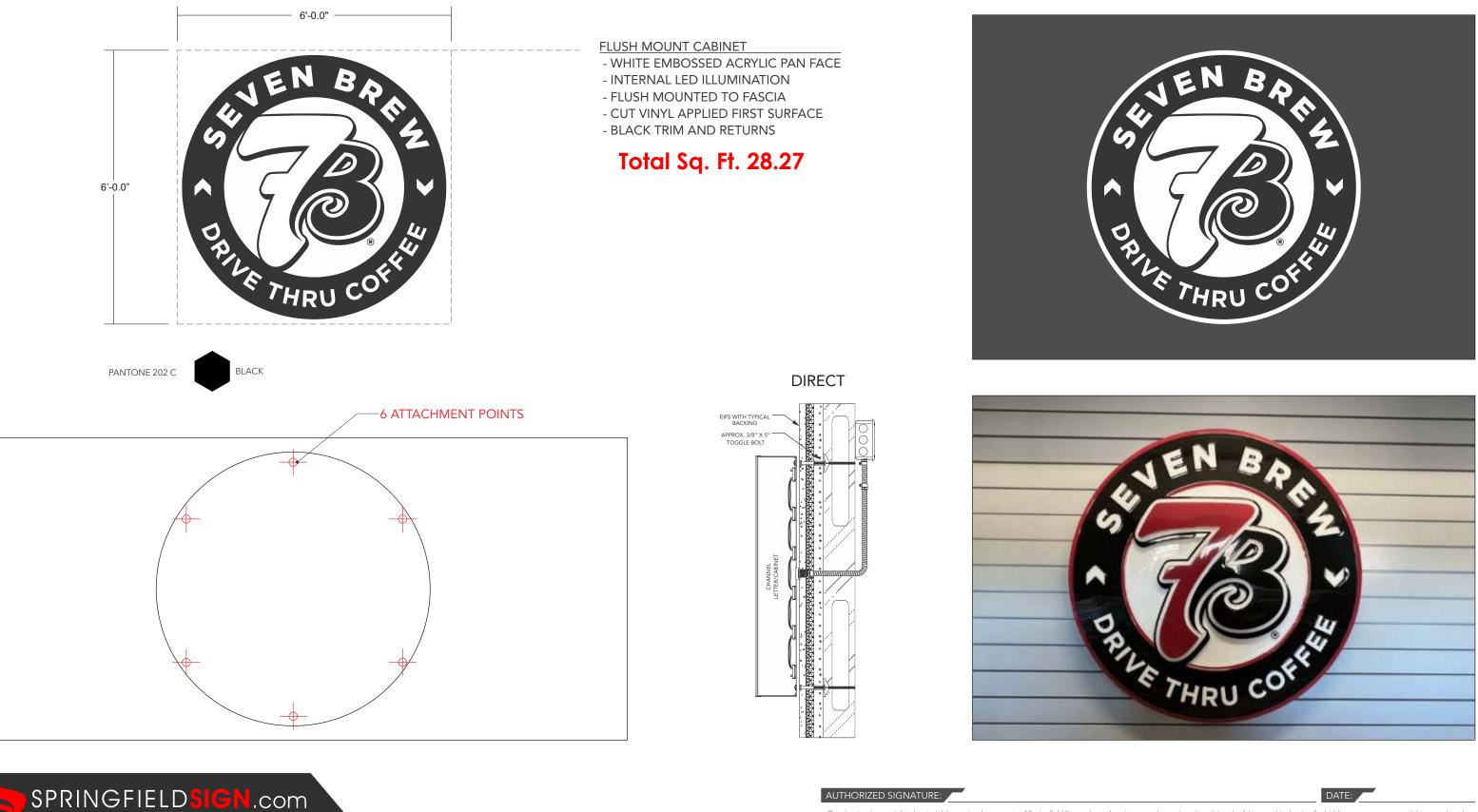
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Elevation Sq. Ft. 1468.5







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DATE:



WALL SIGN

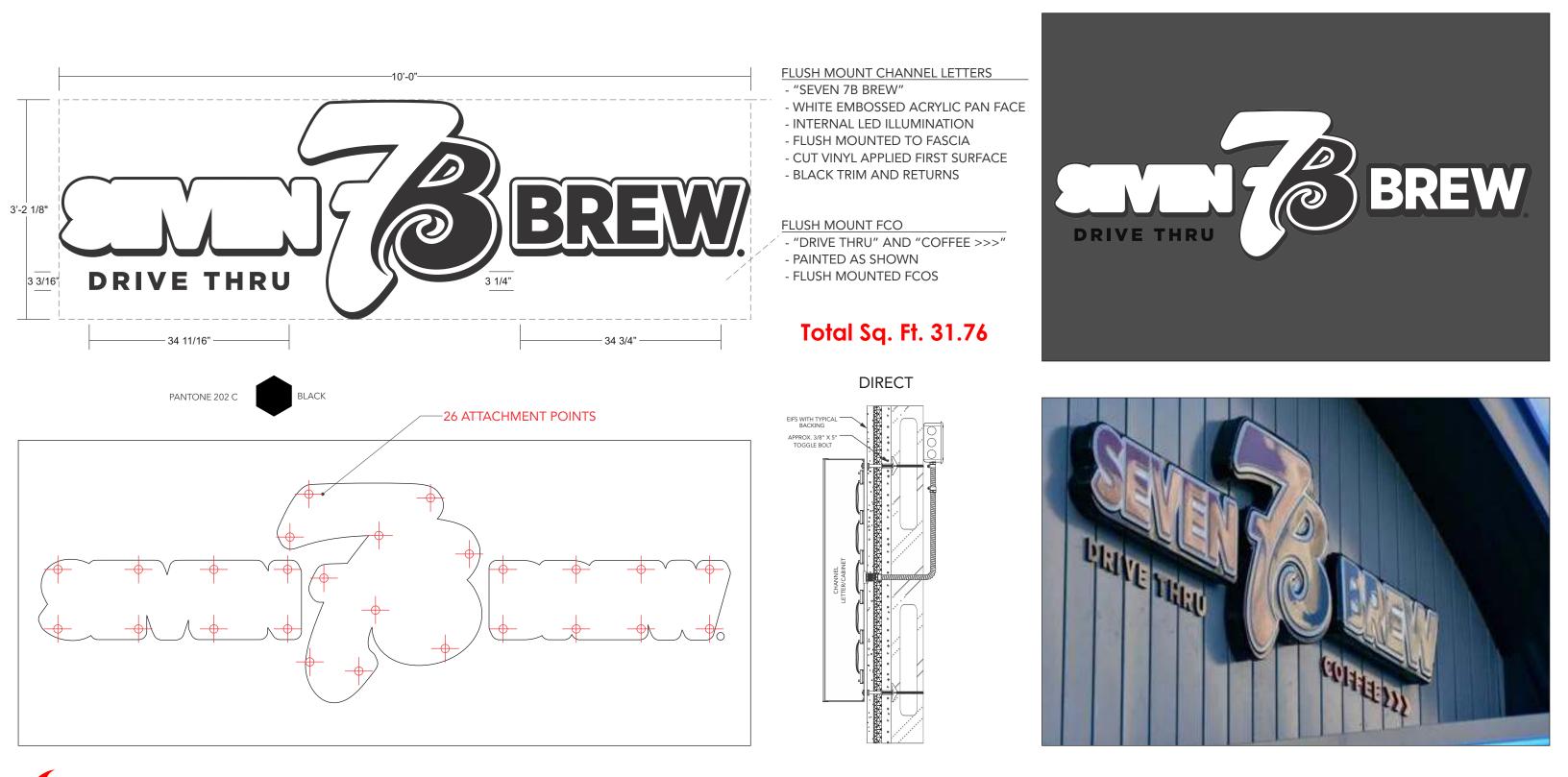


SPRINGFIELDSIGN.com

 SALES:
 Shaun Crawford
 Image: Solution of the solu

EXTERIOR

SCALE: 3/4"=1'





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DATE:



WALL SIGN



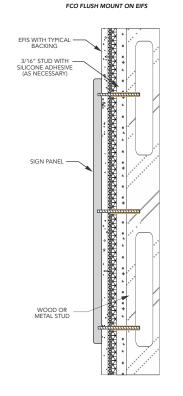
 800.845.9927 SALES: Shaun Crawford CLIENT: 7Brew CREATED: 4/29/24 ARTIST: Joshua Kroeger LOCATION: 2006 N Reynolds Dr Bryant, AR 72022

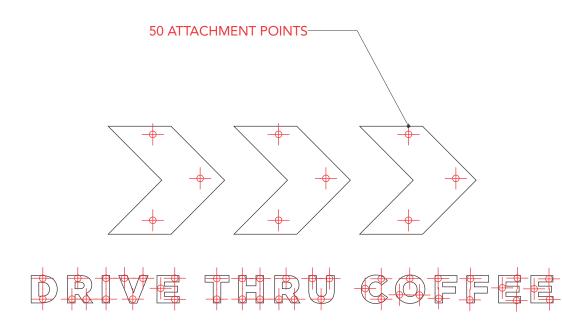
EXTERIOR

SCALE: 1-1/2"=1'









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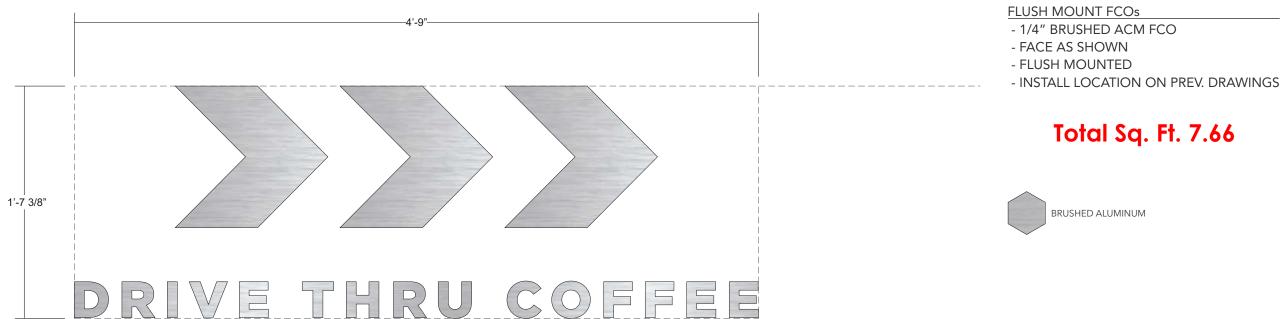


WALL SIGN

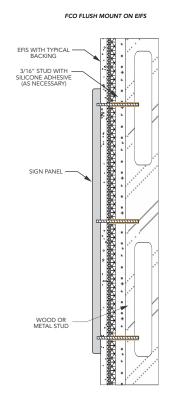


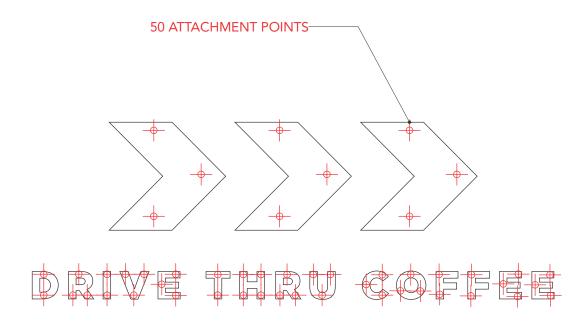


SCALE: 1-1/2"=1'









AUTHORIZED SIGNATURE:



WALL SIGN

Total Sq. Ft. 7.66

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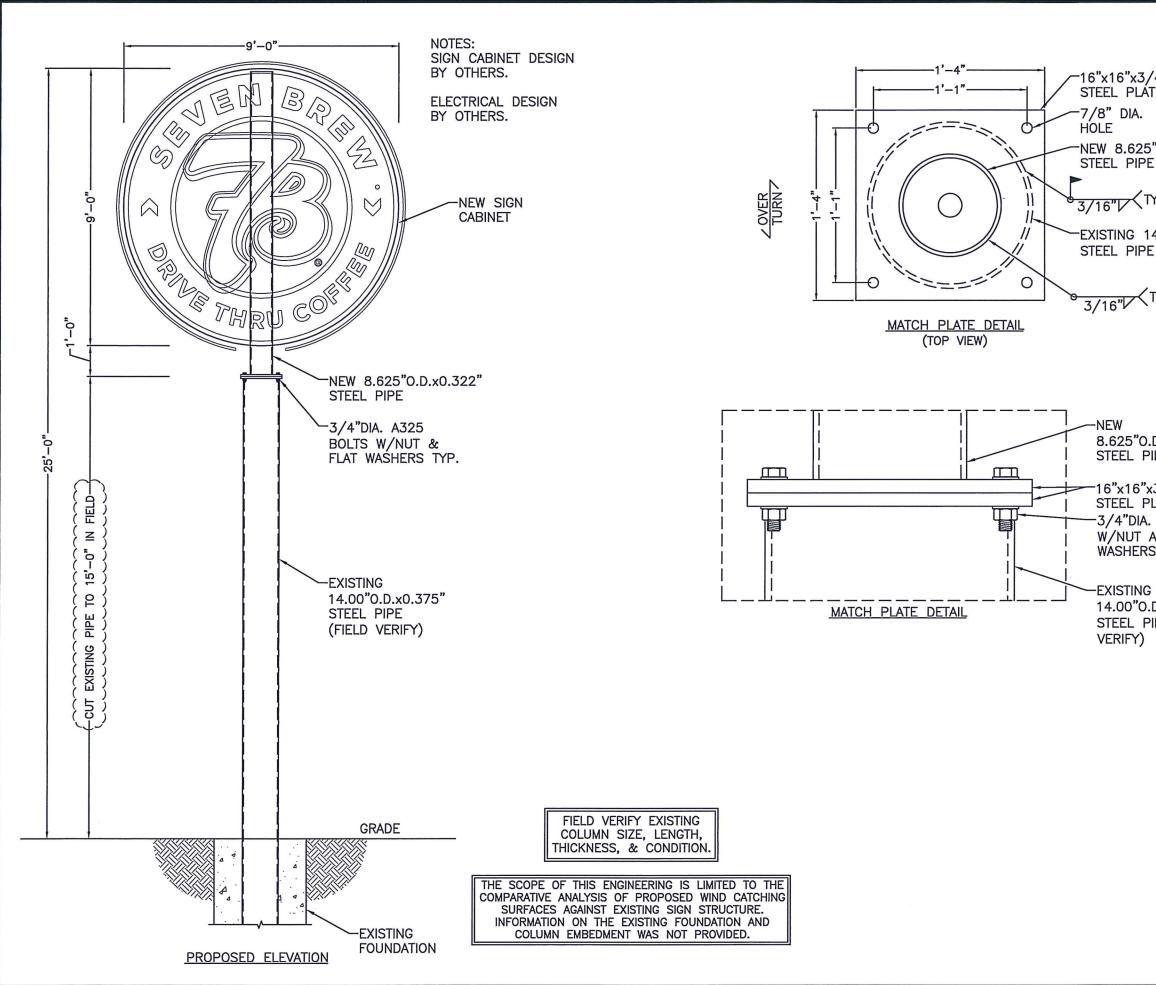
PYLON



D/F ALUMINUM FLEX FACE PYLON CABINET
 Black PAINTED CABINET AND SIDE TRIM
 BLUE FAUX NEON AROUND OUTSIDE OF CABINET
 INTERNAL LED ILLUMINATION

BLEED FACE FLEX FACES - ARTWORK CREATED WITH TRANSLUCENT VINYL

POLE STRUCTURE - REUSE EXISTING POLE (14" X 237")



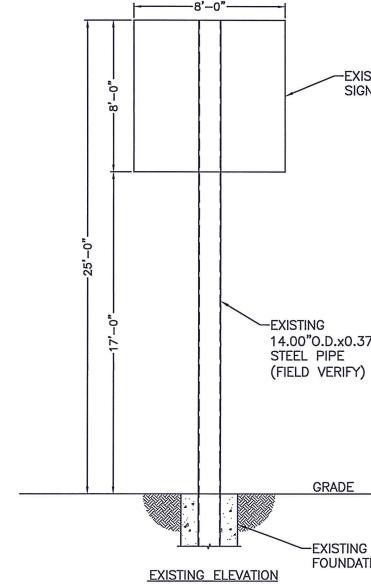
	INSTALLATION ADDRESS:
/4" ATE	7 BREW COFFEE 2202 N. REYNOLDS RD. BRYANT, AR 72022
5"0.D.x.322" ?E TYP.	CLIENT: SPRINGFIELD SIGN
14.00"0.D.x0.375" PE (FIELD VERIFY)	4825 E KEARNEY ST SPRINGFIELD, MO 65803 417.862.2454 FAX: 417.862.1887
(TYP.	Image: Contract of the set
).D.x.322" PIPE	SEAL & SIGNATORE.
X3/4" PLATE A. A325 BOLT AND FLAT RS, TYP.	
G D.D.x0.375" PIPE (FIELD	ARKANSAS ARKANSAS *** REGISTERED PROFESSIONAL ENGINEER *** No. 10828 N. KASHMU 9 5/24
	IMAD KASHIF, P.E. 135 South David Lane & Knoardile, Tennessee 37922 Phone: (885) 539-4001
	Project Number: Drawing Number: 24-0250R B1861518
	SHT. OF DATE: BY: 1 2 9/5/24 GHK

EXISTING

	PROJECT #	24-0250R		OWNER:	7 BREW COR	FEE	
	September 5, 2024				2202 N. REY	NOLDS RD.	
	DRAWING #	B1861518			BRYANT, A	R72022	
	WINDLOAD	20.65	PSF				
	WIND SPEED	105	MPH	CLIENT:	SPRINGFIEL	SIGN & NEON	1
	# COLUMNS	1	IBC 2021 Amended		4825 E. KEA	RNEY ST.	
	DESIGNER	GHK			SPRINGFIEL	D, MO	· ·····
		• • • • • • • • • • • • • • • • • • • •	SHAPE	CENTROID		TOTAL	
ITEM	HEIGHT	WIDTH	FACTOR	HEIGHT	AREA	FORCE	MOMENT
		8 000	4.000	4 000	C4 000	4 004	E 005
SIGN	8.000	8.000	1.000	4.000	64.000	1.321	5.285
COLUMN	17.000	1.167	0.700	8.500	13.883	1.608	30.185
OAH	25.000						

PROPOSED

		PROJECT #	24-0250R		OWNER:	7 BREW COF	FEE		
		September 5, 2024				2202 N. REY	NOLDS RD.		
		DRAWING #	B1861518			BRYANT, AF	R 72022		
		WIND LOAD	20.65	PSF					
		WIND SPEED	105	MPH	CLENT:	SPRINGFIELD	SIGN & NEOL	N	
		# COLUMNS	1	BC 2021 Amended		4825 E. KEARNEY ST.			
		DESIGNER	GHK			SPRINGFIELD	D, MO		
				SHAPE	CENTROID		TOTAL		
	ILEN	HEIGHT	WDTH	FACTOR	HEIGHT	AREA	FORCE	MOMENT	
	SIGN	9.000	9.000	0.785	4.500	63.617	1.313	5.910	
	COLUMN	1.000	0.719	0.700	0.500	0.503	1.324	7.229	
	COLUMN	15.000	1.167	0.700	7.500	12.250	1.577	28.983	
1	OAH	25.000							
		COLUMN CA	LCULATION	(CODES P=PIPE;O=OTHE		R;T=TUBE)			
							DESIGN		
		COLUMN	COLUMN	COLUMN	kx	MODULUS	REQUIRED	FLEXURAL	
	ITEM	WIDTH	DEPTH	WALL	COLUMN	COLUMN	MOMENT	STRENGTH	UNITY
=									
P	SIGN		8.625	0.300	68.1	20.80	5.910	36.33	0.163
P	COLUMN		8.625	0.300	68.1	20.80	7.229	36.33	0.199
P	COLUMIN		14.000	0.349	348.9	65.05	28.983	113.61	0.255
		BOL	CALCULAT	IONS			-		
				*					
			BOLT	BOLTS/	TENSION	BOLT	ALLOW.	ALLOWABLE	
	ITEM	MOMENT	SPACING	PLATE	BOLT	DIAM.	STRESS	TENSION	
	COLUMN	7.229	13.000	4.000	3.337	0.750	20.000	8.836	
			ECALCULAT						
	ПЕМ	TENSION	MOMENT	MOMENT	PLATE	PLATE	PLATE	MINIMUM	
		BOLT	ARM	PLATE	WDTH	DEPTH	THICK.	THICK.	
	COLUMN	3.337	4.875	16.266	9.125	16.000	0.750	0.629	



General Notes:

- 1. Design is based on a 105 mph, 3 second gust wind design per IBC 2021 Am Exposure C. Seismic Design Category D.
- All support members is assumed to be free from defects. Steel Pipe up to 24 presumed to meet ASTM A53 Grade B with a minimum yield strength of 35000
 Steel welds shall be made with E70xx low hydrogen electrodes by persons qua
- with AWS standards within the past two years. All structural bolts shall conform to ASTM A325, and be zinc coated unless not used with structural bolts, heavy hex nuts shall conform to ASTM A563, and we to ASTM F436. Pretension all high strength bolts using the Turn-of-Nut method 4.
- otherwise.
- 5. The scope of this engineer is limited to the comparative analysis of proposed surfaces against the existing sign structure. No information pertaining to the column embedment was made available. The proposed structure depicted on this drawing will produce 96% of the over
- 6. existing structure at grade. Based on this analysis, the existing foundation will structure with a greater factor of safety than it supports the existing structure Structural analysis for this sign is based on field measurements as reported b
- 7. Should field conditions differ from what is shown on this drawing, cease all w SPRINGFIELD SIGN immediately for direction. The scope of this engineer does observations.
- 8. Imad Kashif, P.E., will not be responsible for the safety on this job site before installation of this structure. It is the responsibility of the owners, contractors ensure that the installation and erection of this structure is performed using r full compliance with OSHA regulations.
- Any deviation from this design or from any part of this drawing, including the without prior written consent from Imad Kashif, P.E., voids this drawing in its 9.
- 10. The structure designed on this drawing is intended to be installed at the add not be used at any other location.

SN CABINET CLIENT: SPRINGFIELD_MO 65803 417.862.2464 FAX: 417.862.1887 ST5" ST5" ST5" ST5" ST5" State DATE DESCRIPTION ST5" ST5" ST5" ST5" ST5" State ST5" ST5" State ST5" State ST5" State		
STING N CABINET 2202 N. REYNOLDS RD. BRYANT, AR 72022 STING N CABINET CLENT: SPRINGFIELD SIG SPRINGFIELD S		INSTALLATION ADDRESS:
375" SPRINGFIELD, MO 65903 417.822.2454 FAX: 417.862.1887 375" A despen of join Medical on the despect of the other provest of MD RASE, PL Medican of a -/-/- A despen of join Medical on the despect of the other provest of MD RASE, PL Medican of a -/-/- A despen of join Medical on the despect of the other provest of MD RASE, PL Medican of a -/-/- A despen of join Medical on the despect of the other provest of MD RASE, PL Medican of brobbid within the within counce of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the despect of MD RASE, PL Medican of the Medical or the MEDICAL Constraints, The Medical or the Medical or the Medical or the Medical table of MD RASE, PL Medican of the Medical or the new respect of MD RASE, PL Medican of MD RASE, PL Medican of the Medical or the new respect of MD RASE, PL Medican of MD RASE, PL Medican of the Medical or the new respect of MD RASE, PL Medican of MD RASE, PL Medican of the Medical or the new respect of MD RASE, PL Medican of MD RASE, PL Medican of the Medican of MD RASE, PL Medican of the Medican of MD RASE, PL Medican of MD RASE, PL Medican of the Medican of MD RASE, PL Medican of MD RASE, PL Medican of the Medican of MD RASE, PL Medican of		2202 N. REYNOLDS RD. BRYANT, AR 72022
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mended, Category II, 24 inch O.D. is 00 psi. Judified in accordance moted otherwise. When washers shall conform nod unless noted d wind catching e existing foundation or enturning moment of the II support the new Ire. by SPRINGFIELD SIGN. work and contact not include onsite ore, during or after rs and installers to methods that are in Project Number: Interpret Number: In	375")	▲ -/-/- ▲ -/-/- ▲ -/-/- ▲ -/-/- ▲ -/-/- ■ -/ ■ -/ ■ -/ ■ -/ ■ -/ ■ -/ ■ -/ ■ -/ ■ -/ ■ -/ ■ -/ ■ -/
mended, Category II, 24 inch O.D. is 00 psi. Judified in accordance moted otherwise. When washers shall conform nod unless noted d wind catching e existing foundation or enturning moment of the II support the new Ire. by SPRINGFIELD SIGN. work and contact not include onsite ore, during or after rs and installers to methods that are in Project Number: Interpret Number: In		
REGISTERED PROFESSIONAL ENGINEER washers shall conform hod unless noted d wind catching e existing foundation or erturning moment of the Ill support the new re. by SPRINGFIELD SIGN. work and contact not include onsite ore, during or after rs and installers to methods that are in PROFESSIONAL ENGINEER * * * No. 10828 No.		ARKANSAS
e existing foundation or prturning moment of the ill support the new ire. by SPRINGFIELD SIGN. work and contact not include onsite ore, during or after rs and installers to methods that are in Project Number: IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	00 psi. Ialified in accordance noted otherwise. When washers shall conform	REGISTERED PROFESSIONAL ENGINEER No. 10828
Ill support the new ire. by SPRINGFIELD SIGN. work and contact not include onsite ore, during or after rs and installers to methods that are in Project Number: ITMAD KASHIF, P.J 135 South David Lane • Knowlin, Tennessee 37822 Phone: (855) 539-4001 Project Number:		9/5/24
not include onsite not include onsite re, during or after rs and installers to methods that are in Project Number: Drawing Number:	ll support the new re.	IMAD KACHIE DE
rs and installers to methods that are in Project Number: Drawing Number:	work and contact not include onsite	
e General Notes Project Number: Drawing Number:	rs and installers to	Protect Numbers
entirety. 24–0250R B1861518		24-0250R B1861518

2 9/5/24 GHK

2



SIGN PURCHASE AGREEMENT

€ 800.845.9927 ● springfieldsign.com

This agreement, made and entered into this _____ day of _____ (month), 20 ____ (year), by and between Springfield Sign & Graphics, INC. d/b/a Springfield Sign (herein after referred to as Seller), and Buyer (as outlined below and labeled as Buyer) witnesseth, that the Seller agrees to manufacture for Buyer the sign(s) and/or other sign products/services as outlined in a separate E2 document as follows, QUOTE Number: or other such unique document of description as follows:

BUYER:

COMPANY NAME D/R/ABILLING ADDRESS: CITY STATE JOB DETAILS: COMPANY NAME D/B/A BILLING ADDRESS CITY STATE

All wiring on the premises to the site of the signs- installed location, including the connection of the sign to such primary wiring source is to be the responsibility of the Buyer, at additional cost to the Buyer, at the direction of the Buyer, as designed by the Buyer or Buyer's agents and, as necessary, all other aspects and expenses, as required, to bring primary electrical wiring to the sign's location for energizing of such signs. All voltages to be 120 Volt at 60 Hertz unless otherwise specified. Any damages caused by the energizing circuit to the sign or sign products due to improper design (including but not limited to improper voltages), improper connection thereof or any other causes related to the energizing primary circuitry will be solely and completely at the Buyer's risk and expense. Any additional work, trouble shooting in the field, by phone, by internet or otherwise required on behalf of Seller will be bi/Jed in addition to Buver on a Time and Materials basis, at additional expense. All Permit fees/Engineering fees and labor/drawing costs for the acquisition thereof will be billed in addition to prices stated herein at additional expense unless specifically outlined in this document to be bi fled in another manner as described herein. Any required sales/use taxes are the responsibility of the Buyer, now and in the future as so levied by applicable governing authorities. All taxes are due and payable upon demand by Seller at or any time subsequent to the execution of this SIGN PURCHASE AGREEMENT. It is understood that taxes are in addition to the prices outlined in the SIGN PURCHASE AGREEMENT, E2 Quote or any other document outlining the signs, products or services for stated Job Location unless specifically and clearly outlined otherwise. It is expressly and undeniably inderstood by both Buyer (or Buyer's agents, subcontractors, salespersons, etc.) and Seller that no verbal agreement has been entered into. Both parties are to adhere to the terms and conditions of this SIGN PURCHASE AGREEMENT and related attachments as properly executed and initialed. Any governing entity outside the control of Seller, such as but not imited to, any applicable City/Municipalities, County Office/Agent, national codes (such as but not limited to NEC, BOCA, ETC.) with jurisdiction or control upon the product, labor manufacturing or installation) or any issues, procedures or otherwise related to the execution of the terms, signs, sign products, services or otherwise, foreseen or unforeseen, may affect the costs and timely delivery of such products/services herein NOTICE: THIS IS A LEGAL DOCUMENT WITH BINDING OBLIGATIONS READ BOTH SIDES OF THIS NSTRUMENT BEFORE SIGNING, AS THE TERMS OF THIS SIGN PURCHASE AGREEMENT ARE SET OUT THEREON. The specific terms for payment may vary based on product types or other reasons, but it is expressly understood that PAYMENT IN FULL as outlined by this Agreement is due and must be paid PRIOR TO INSTALLATION OF SIGN PRODUCT, PARTS OR SERVICES. All outstanding balances over 30 days due are subject to a 2% per month (collectively compounding) Late Fee.

DATE:

Buyer's	Guarantee	to	Build	 (initials)

Rejection of Buyer's Guarantee to Build _ (initials)

	BU	ſΕ
_		

By:

PRINTED NAME:

I/We have read this entire agreement and agree to defend and hold harmless Seller as stated herein. ACCEPTED:

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TITLE:

By: _

ACCEPTED

SELLER:

Mark Wessell, CEO Springfield Sign 4825 E Kearney St Springfield, MO 65803 1. DOCUMENT ATTACHMENT As allowed by this contract, other documents such as but not limited to E2 quotes, product specifications, manufacturer's specifications, etc. may be referenced in the area in the beginning of this Sign Purchase Agreement, These documents may have additional terms. conditions, pricing, restrictions, limitations or otherwise as describe by those written instruments, such as but not limited to LED (or other nest of electronic displays.

2. LIMITED WARRANTY Seller warrants all new materials and/or services delivered herein to be at time of completion of job and time of delivery, to Buyer, to be free from defects of material and/or workmanship. Seller agrees to repair or replace, solely at Seller discretion, any products or parts thereof, which are found defective in material or workmanship within 90 days from time of installation of sign or sign product. Seller's obligation with respect to such products or parts shall be STRICTLY LIMITED to replacement or repair and in NO event shall Seller be liable for consequential, incidental or special damages, or for transportation, installation, adjustment or any other expenses which may arise in connection with such products or parts, including but not limited to loss of business or loss of trade. THIS WARRANTY IS EXPRESSLY MADE IN LIEU OF ANY AND ALL OTHER WARRANTIES EXPRESS OR IMPLIED. INCLUDING THE WARRANTIES OF MERCHANTABILITY AND FITNESS FOR A PARTICULAR PURPOSE. THERE ARE NO OTHER WARRANTIES. Seller's obligations hereunder shall extend only to defects for which Buyer shall have given Seller writter notice thereof within ninety (90) days after date of delivery or installation, as applicable. Buyer is NOT authorized to make independent arrangements for warranty work. All warranty work on said signs, products, parts, services, as described herein, shall be arranged or subcontracted by Seller or be done by Seller's employees or representatives, solely at the discretion of the Seller. Ir the event that Buyer does not permit Seller to inspect product. access property or in any other way directly or indirectly inhibits the Seller to arrange for or conduct necessary repair work required under this Agreement, or Buyer makes independent arrangement for such repair work. Buyer garees that Buyer will be solely responsible for the costs of such repairs. In the event Buyer does not comply with the above, Seller hereby EXCLUDES ALL WARRANTIES, EXPRESS AND/OR IMPLIED, AND BUYER PURCHASES THE SIGN, SIGN PRODUCT AND/OR SERVICES "AS IS" and WITH ALL FAULTS, WAIVING ALL WARRANTIES HEREUNDER. Additional limitations include but are not limited to, acts of God, acts of nature, vandalism, acts of War or

rrorism and/or accidental damage 3. INSURANCE As long as any amount of monies is due and owing to Seller, Buyer shall insure sign, sign products or services, in an amount no less than monies due Seller, and NAME SELLER in the loss payable clause of such insurance policy, strictly for the benefit of the Seller Buyer further agrees to not limit the insured causes allowing for fire or any other casualty. Buyer shall furnish Seller with evidence of such certification of insurance, in writing from Buyer's insurance agent or agency, upon Seller's demand. Seller's certificate of insurance for liability/workers compensation shall be provided by Seller to buyer pon request from Buyer

4. TAXES Buyer shall be responsible for and pay all taxes including but not limited to Sales, Use, Personal Property or any other municipal, county, state or federal taxes that may be levied, imposed or assessed by law on the sign product, parts or services or improvements thereon, or uses of such. Buyer agrees to reimburse Seller for any amount for such taxes, that may be billed to and paid by Seller. Any interests or penalties associated with any taxes as outlined herein will also be due and owing to Seller if so paid by Seller. These taxes, as allowed by law, may or may not be assessed at time of initial sale or delivery of sign product, parts or services and may continue forward in time without end.

5. PERMITS/LICENSES Seller shall not be obligated to commence fabrication of sign product, parts or services until all necessary permits have been issued. If permits are denied after reasonable effort by both parties to secure same, then this Sign Purchase Agreement shall terminate without liability to either Buyer or Seller except that Buyer shall pay Seller for reasonable compensation for labor and costs expended until the time permits are denied. Buver shall be responsible for securing and maintaining in effect writter consent from the owner of record of the premises upon which sign product, parts or services is to be installed and for all other priv permissions, consents or licenses, including but not limited to, the use of registered trademarks or copyrights used on the sign product, parts or services, necessary for the manufacture, the installation nance and use of such. The only exception to this will be if the Buver's Guarantee to Build check box at the beginning of this Sign Purchase Agreement is checked. The terms and conditions for this briefly being stated herein, shall GUARANTEE FULL PAYMENT TO SELLER with no guarantee to Buyer that the sign product, parts or services will be utilized to any benefit of the Buyer. Buyer will be obligated to pay the full contract price, including installation and will have to make arrangements for receipt of, off loading of and storage of sign product, parts or services with no future claims for installation, service or maintenance of such from Seller. The Buver's Guarantee to Build is strictly offered to allow manufacturing of sign product, parts or services to proceed WITHOUT the proper permits obtained. It in no way obligates Seller to be adverse to the law for installation (without permits) of sign product, parts or service. 6. INSTALLATION OF SIGN PRODUCT Buyer agrees to and stipulates that Buyer has designated the location for the sign product, parts of services and subsequent installation of such and is responsible for all required materials, labor and any other associated expense, at Buyer's risk, for the necessary requirements for proper, obstruction free and/or lawful installation. Obstructions, obstacles or other encumbrances, includes but is not limited to building reinforcement building or site alterations, all obstacles as required for successful, safe, lawful installation including but not limited to overhead (powe lines, buildings, trees or other encumbrances), underground (such as utilities, easements, rocks, buried objects natural, man made or otherwise), landscaping, sidewalks, planters, asphalt, concrete or any other such improvements, construction crews other that Seller's or Seller's agents or Buyer's normal business traffic, Seller will not be responsible for any damages for such items during the normal installation process. Any return trips or delays or overtime charges incurred will be passed on to Buyer at Buyer's expense.

7. ASSIGNMENT This Agreement shall be binding and inure to the benefit of the parties hereto, their respective successors, executors administrators, assigns and legal representatives; provided, howeve that the interests of Buyer herein shall be assianed only with the expressed, written consent and approval of Seller. No transfer or assianment of this Aareement or any interest hereunder shall release 8. DEFAULT OR BREECH OF AGREEMENT The parties stipulate that the

sign product, parts or services is(are) not an article of general trade or utility but is uniquely designed and is to be constructed and/or nstalled at the request and for the sole and special purpose Buyer. The sign product, parts or services is of no value to Seller, and herefore, has no resale or other value to anyone other than Buyer this Agreement is not cancelable except with expressed written nission of the Seller. Buyer shall be deemed to have breached this Agreement by insolvency, default in payment amounts or chedules as set forth herein, abandonment of the sign product, parts or services or vacating the premises where such is located appointment of a receiver for Buyer's business, the filing of a voluntary or involuntary petition of bankruptcy with respect to Buyer, or any act or omission of Buyer in contravention to this Agreement. I addition to Buyer's other obligations hereunder, in the event Seller shall institute any action or lawsuit for the enforcement of the obligations of Buyer herein, Buyer shall pay and indemnify Seller for all costs of court, reasonable attorney's fees expended, interest expenses of 2% per month or as allowed by law whichever is more, collection fees, administration fees, and, pay Seller all amounts awarded by the court as a result of such proceedings. Buyer's preach of any provision in any other Sign Purchase Agreement or other instruments as put forth by Seller or Confirmation of Order with Seller shall also be deemed to be a breach hereunder, and Seller may suspend its performance and delivery under this and all other agreements with Buyer until Buyer provides Seller with adequate surance of performance within a reasonable time, not exceeding ten (10) days, after Seller has informed Buyer orally or in writing, of its

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specifically retains ownership and/or title of sign product, parts or services until Buyer has performed and fulfilled all terms and conditions required by Seller, herein, or as otherwise allowed by law to Seller's benefit. Buyer also, conveys to Seller the absolute right to access property to remove sign product or parts from said property, should any default arise on Buyer's behalf, and, to pay for all necessary costs for removal and possible subsequent re-installation of said product at expense in addition to that outlined herein, solely at Buyer's risk and expense. Buyer agrees and will defend same that Seller shall at all times have title to all original drawings, designs and specifications relating to the work hereunder, which were developed or created by or on behalf of Seller, and Seller hereby claims copyrights, where applicable, of all such drawings, designs and specifications. Payment of all or part of any amounts hereunde does not pass title to the "original drawings, designs, specifications" of said sign product, parts or services, although the same may be reproduced with the expressed written consent of Seller. Buyer shall, upon request of Seller, promptly return all such drawings, designs and specifications, and copies thereof, to Seller during all times which Buyer owes Seller any amounts hereunder. Buyer agrees that Seller has specific legal rights in the form of Copyrights or other instruments given by law to Protect and does hereby declare Seller's ownership of all drawings artwork and the like during and after the terms of the areement hereir

10. MANUFACTURING SPECIFICATIONS/INDUSTRY STANDARDS Buyer understands and agrees to allow Seller, solely at Seller's discretion to make modifications for and conforming to Seller's standard manufacturing practices. It is also understood by Buyer, that no color, shape, dimension or any other specific feature of said sign product, parts or services is augranteed absolute. As practical examples absolute color matches or dimensions are not guarantee and will be allowed reasonable differences within industry standards Buyer agrees that Seller may mark and label sign for legal, national code, electrical, manufacturina, advertisina or other requirements and purposes as is reasonably necessary to conduct day-to-day business as allowed or required in the industry and for Seller's own staction and benefit 12. SUBJECT TO PRIOR SALE Used sign products, new or used product

ed for resale from other manufacturers, such as but not limited to electronic displays may be subject to prior sale affecting price and or delivery times to Buyer. Seller will not be responsible for any onsequences related to such issues. This includes promotional or sales or any other type of proposal made to customer, Seller has no authority or control over other manufacturers and Seller employ's numerous sales agents all of whom may be promoting the same sale em, thus depleting any available inventory. 13. CHANGE ORDERS Any alteration, deviation and/or reasonably significant variance from the scope of work, construction or labor or otherwise from the sign(s) or products or services as outlined herein, including all extra costs, hereafter called Change Order, will be executed only upon written orders and may become an additional charge over and above the price stated herein. Any such Change Order requested by Buyer must be agreed to by Seller, in writing and solely at the discretion of the Seller. It is understood that any Change Order could necessitate the need for a price increase, an extension the time required to complete the work outlined herein or any other reasonable and necessary charges, terms, conditions, equipment or the like as so required to execute the Change Order

14. SUSPENSION OF MANUFACTURING Any request or event or otherwise required by Buyer to delay, suspend, cancel or otherwise impede the manufacture, delivery and/or installation of said sign product, parts or services, for any reason or occurrence out of th control of Seller, then Buver shall immediately pay the full purchase price or any amount remaining and due to Seller. Furthermore, upor such request, Buyer shall be solely responsible for storage charges and any increases in labor and/or material costs incurred by Seller in the manufacturing process. Buyer's failure to comply with this provision will be deemed and construed as an anticipatory breach of this Agreement. In the event Buyer complies with the foregoing, Seller will complete the manufacturing, delivery and/or installati within a reasonable period of time upon reasonable request of

15. SECURITY INTEREST Buyer grants Seller a SECURITY INTEREST in the sign product, parts and/or services until all obligations to Seller, hereunder are fully paid. Seller may file and record this Agreement as a financing statement under Chapter 400 of the Missouri Uniform

P. COPYRIGHTS, TITLE & CONVEYANCE OF OWNERSHIP Seller

Commercial Code Section 400.9-521, in addition to any other permitted standard or nonstandard forms. If Buyer shall fail to pay as agreed to herein. Seller (or Seller's agents or representatives) shall ve the right, and will be defended by Buyer, and is hereby authorized and empowered to take and resume possession of and move into Seller's possession, with or without process of law, the sian product, parts or services and all other property described in, wherever found, and remove and sell the same at either public or private sale, or by any other viable method, as deemed solely by Seller, at such time and place as Seller shall choose, and as allowed by law. Seller shall apply the proceeds of such sale as a credit upon the obligations of Buyer hereunder. In such event, Seller is entitled to recover all expenses of sale, including any reasonable attorney's fees necessary in handling the matter, without prejudice to Seller to the further enforcement of any balance of such obligation due Seller by Buyer, or expenses remaining due from sucl sale. In the event the proceeds of such sale exceed the balance of Buyer' obligation to Seller and the expenses of such sale, Seller shall forward any such excess to Buyer. Buyer shall not use said sign products as to lessen the value of Seller's SECURITY INTEREST or impair the operation of said sign product, and in the event the sign product is damaged through the intentional acts or willful negligence of Buyer, Buyer's customers, its agents or employees, contractors or third parties, or by wind, hail, earthquake, fire, war, tornado, hurricar flood, labor dispute, vandalism, acts of God or acts of nature, Buyer agrees to pay for the necessary expenses to restore said sign product, part or services in operable condition. After delivery and/c installation, whichever is contracted for, in the event the sign product is lost, stolen, destroyed, or otherwise impaired, Buyer shall main liable to Seller for all amounts hereunder, UNTIL BUYER'S OBLIGATIONS TO SELLER ARE FULLY SATISFIED HEREUNDER, THE PROPERTY DESCRIBED HEREIN WILL REMAIN PERSONAL PROPERTY OF SELLER WHETHER THE SAME IS ATTACHED IN ANY MANNER TO THE REALTY OR NOT. SAID PROPERTY SHALL NOT, BY REASON OF ATTACHMENT OR CONNECTION TO THE REALTY, BECOME OR BE DEEMED A FIXTURE OR APPURTENANT TO SUCH REALTY. No transfer ewal, extension, or assignment of the Agreement or of any interest hereunder, and no loss, damage or destruction shall release Buyer or any Guarantor from the obligations assumed hereunder. During all times in which Buyer is obligated for any amounts to Seller hereunder, Buyer shall keep said property free from all tax liens and other encumbrances, and any sum of money that may be paid by Seller to release any such liens or encumbrances shall be paid on demand by Buyer in addition to the obligations secured hereunde 16. WAIVER OF CONSUMER RIGHTS

17. DELIVERY AND PERFORMANCE Seller shall not be held responsible for, and the period of time required for completion of any project or maintenance or repairs, shall be tolled during any time when Seller is delayed or prevented from completing the obligations hereunder because of strikes, equipment breakage, fire, war, terrorism, labor disputes, commercial delays, acts of God/nature, regulations or restrictions of any government entity or public authority, or any accidents or forces, conditions, or circumstances beyond Seller's control, and Seller shall not be liable for any loss whatsoever suffered by Buyer, directly or indirectly, as a result of any such events or occurrences. Buyer agrees to examine and inspect all installations repairs, and maintenance, and within ten (10) days, notify Seller in riting of any complaints about work performed under thi Agreement. The failure of Buyer to give such written notice shall constitute acceptance of the work performed. The provisions of the paragraph shall not be limited by any provision in which time is nade of the essence. Notwithstanding anything in this Agreement to the contrary, if at any time prior to completion of this Agreement Seller's prospects for payment are, in Seller's sole discretion, impaired Seller may require payment in advance before permitting delivery o any installation or services hereunder, and may demand Buyer's immediate performance of Buyer's obligations hereunder. If equested by Seller, Buyer shall furnish evidence, satisfactory to Seller prior to commencement of Seller's work hereunder or at any time ereafter, that sufficient funds are available and committed to pay the full amount owing by Buyer under this Agreen

18. STATE OF JURISDICTION/SEVERABILITY/MISCELLANEOUS All esentatives of Seller are stipulated and specified in this Agreement. No modifications hereof shall be valid unless made ir writing AND agreed to, AND signed by both Seller and Buyer. No waiver by either party hereto shall be a waiver of any subsequent breach of or failure to perform the same or any other term, condition, or obligation hereof. It is agreed by both parties hereto that venue of any action arising under the Agreement shall be in Greene County, Missouri and the laws of the State of Missouri shall govern this Agreement. Should any part of this Agreement contravene public policy or laws of the jurisdiction in which it is sought to enforce the same, then such part shall be considered null and void and have no force and effect, and the balance of the rms and conditions of this Agreement shall remain valid and in full force and effect. Buyer expressly grants Seller the right to use photographs, drawings or other replicas of the sign product specified herein in its brochures, pamphlets, displays, sales documents or other advertising or promotional media in the ordinary course of business of Seller. Seller may place on the sign product its ame, telephone number and location of such information, as shall be determined by Seller and solely by the Seller. Buyer agrees that Buyer is purchasing said sign product for business or commercial purposes or use and not for personal, family or household use or poses. In regard to payment of any amount due hereunder, time is of the essence.



City of Bryant, Arkansas Community Development 210 SW 3rd Street Bryant, AR 72022 501-943-0943

SIGN PERMIT APPLICATION

Applicants are advised to read the Sign Ordinance prior to completing and signing this form. The Sign Ordinance is available at <u>www.cityofbryant.com</u> under the Planning and Community Development tab.

Date: 9/6/2024

Note: Electrical Permits may be Required, Please contact the Community Development Office for more information.

Sign Co. or Sign Owner Name_Alicia Walton - Springfield Sign Address 4825 E. Kearney St

City, State, Zip Springfield MO 65803

_{Phone} 417-862-2454

Email Address aliciaw@springfieldsign.com

Property Owner

Name Michael Lannon Address 529 N Prince Lane City, State, Zip Springfield MO 65803 Phone 417-860-4714 Email Address michael.lannon@cmcmod.com

GENERAL INFORMATION

Name of Business Seven Brew

Address/Location of sign_2006 N Reynolds Dr Bryant AR

Zoning Classification C-2

Please use following page to provide details on the signs requesting approval. Along with information provided on this application, a **Site Plan showing placement of sign(s) and any existing sign(s) on the property is <u>required</u> to be submitted. Renderings of the sign(s) showing the correct dimensions is also** <u>required</u> to be submitted with the application. A thirty-five dollar (\$35) per sign payment will be collected at the time of permit issuance. According to the Sign Ordinance a fee for and sign variance or special sign permit request shall be one hundred dollars (\$100). Additional documentation may be required by Sign Administrator.

READ CAREFULLY BEFORE SIGNING

Alicia Walton_____, do hereby certify that all information contained within this application is true and correct. I fully understand that the terms of the Sign Ordinance supersede the Sign Administrator's approval and that all signs must fully comply with all terms of the Sign Ordinance regardless of approval. I further certify that the proposed sign is authorized by the owner of the property and that I am authorized by the property owner to make this application. I understand that no sign may be placed in public right of way. I understand that I must comply with all Building and Electrical Codes and that it is my responsibility to obtain all necessary permits.

SIGN	Type (Façade, Pole, Monument, other)	Dimensions (Height, Length, Width)	Sqft (Measured in whole as	Height of Sign (Measured from lot surface)		Column for Admin Certifying
			rectangle)			Approval
				Top of Sign	Bottom of	
					Sign	
A	Front Wall	6' x 6'	28.27	16' 10.2"	10'10.2"	
В	Left side wall	3' 2 1/8 x 10'	31.76	20' 3.8"	12' 4.5"	
C	Right side wall	3' 2 1/8 x 10'	31.76	20' 3.8"	12' 4.5"	
E	Front panel	1' 7 3/8 x 4'9"	7.66	2' 5.375"	10'	
F	Side panel x 2	1' 7 3/8 x 4'9"	7.66	9' 11.3	8'4.5"	
G	Pole sign	9' 4" x 9' 6"	63.62	25'	17	

Use table below to enter information regarding each sign for approval. Please use each letter to reference each sign rendering.