ARKANSAS STORAGE CENTER BRYANT, AR DRAINAGE REPORT

FOR City of Bryant, Saline County, AR

October 2023

Owner & Developer: STUART FINLEY Address: P.O Box 10, Bryant, AR. 72089

By:



PROJECT TITLE

I-30 SELF STORAGE

PROJECT PROPERTY OWNER

STUART FINLEY

PROJECT LOCATION

25300 I-30 North, Bryant, AR

PROJECT DESCRIPTION

The proposed self-storage facility development is located on High-way I-30 in the city of Bryant, Arkansas. The total development area is 24.31 acres.

DRAINAGE ANALYSIS

On Site Drainage- Rational method was used to determine the existing and proposed flows from proposed site. Detailed drainage calculations considering the future expected development have been conducted. Summary of the calculations are below:

- Pre-development area: 28.91 acres.
- Post-development area: 28.91 acres.
- Pre-development runoff coefficient: 0.47.
- Post-development runoff coefficient: 0.88.
- Time of Concentration for Pre-development Area: 16.05 min
- Time of Concentration for Post-development Area: 8.03 min
- Pond has a bottom area of 1.67 acres with bottom elevation of 349.00'
- One 18" RCP with 0.5% slope is proposed for outflow culvert.

Peak flows for Pre and post development phase of onsite area have been tabulated below-

		Post-Development	Post-Development
	Pre-Development	without Detention	with Detention
	Peak Flow (cfs)	Peak Flow (cfs)	Peak Flow (cfs)
2-Year	53.08	131.14	2.99
5-Year	58.66	147.91	3.498
10-Year	69.15	166.14	4.020
25-Year	79.33	189.21	4.600
50-Year	90.45	213.91	5.051
100-Year	96.16	226.82	5.157
тос	16.05 min	8.03 min	

CONCLUSION

The onsite drainage calculation for pre and post condition has been provided.

Watershed Model Schematic



Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	53.08	1	16	50,961				Pre-development
2	Rational	131.14	1	8	62,945				Post-Development
3	Reservoir	2.990	1	16	57,823	2	349.84	61,739	Pond
3	Reservoir	2.990	1	16	57,823	2	349.84	61,739	Pond
22		Storage	Drainge	Report or		eriod: 2 Vo) par	Wednosday	(10 / 18 / 2023

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Pre-development

Hydrograph type	= Rational	Peak discharge	= 53.08 cfs
Storm frequency	= 2 yrs	Time to peak	= 16 min
Time interval	= 1 min	Hyd. volume	= 50,961 cuft
Drainage area	= 28.910 ac	Runoff coeff.	= 0.47
Intensity	= 3.907 in/hr	Tc by User	= 16.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1



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Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	58.66	1	16	56,310				Pre-development
2	Rational	147.91	1	8	70,997				Post-Development
3	Reservoir	3.498	1	16	65,800	2	349.95	69,554	Pond
3	Reservoir	3.498		16	65,800	2	349.95	69,554	Pond
22-	⊥ 0800 I-30 Self	Storage	Drainge	Report.gr	wReturn P	eriod: 5 Ye	l ar	Wednesday	/, 10 / 18 / 2023

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Hyd. No. 1

Pre-development

Hydrograph type	= Rational	Peak discharge	= 58.66 cfs
Storm frequency	= 5 yrs	Time to peak	= 16 min
Time interval	= 1 min	Hyd. volume	= 56,310 cuft
Drainage area	= 28.910 ac	Runoff coeff.	= 0.47
Intensity	= 4.317 in/hr	Tc by User	= 16.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1



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Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	69.15	1	16	66,385				Pre-development
2	Rational	166.14	1	8	79,748				Post-Development
3	Reservoir	4.020	1	16	74,479	2	350.06	78,053	Pond
22-	 0800 I-30 Self	Storage	Drainge	Report or	wReturn P	eriod: 10 Y	l ′ear	Wednesday	/. 10 / 18 / 2023

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Pre-development

Hydrograph type	= Rational	Peak discharge	= 69.15 cfs
Storm frequency	= 10 yrs	Time to peak	= 16 min
Time interval	= 1 min	Hyd. volume	= 66,385 cuft
Drainage area	= 28.910 ac	Runoff coeff.	= 0.47
Intensity	= 5.089 in/hr	Tc by User	= 16.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1
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Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	79.33	1	16	76,152				Pre-development
2	Rational	189.21	1	8	90,822				Post-Development
3	Reservoir	4.600	1	16	85,472	2	350.21	88,823	Pond
3	Reservoir	4.600		16	85,472	2	350.21	88,823	Pond
22-	0800 I-30 Self	Storage	Drainge	Report.gp	wReturn P	eriod: 25 Y	ear	Wednesday	/, 10 / 18 / 2023

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Pre-development

Hydrograph type	= Rational	Peak discharge	= 79.33 cfs
Storm frequency	= 25 yrs	Time to peak	= 16 min
Time interval	= 1 min	Hyd. volume	= 76,152 cuft
Drainage area	= 28.910 ac	Runoff coeff.	= 0.47
Intensity	= 5.838 in/hr	Tc by User	= 16.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1



Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	90.45	1	16	86,827				Pre-development
2	Rational	213.91	1	8	102,677				Post-Development
3	Reservoir	5.051	1	16	97,246	2	350.36	100,388	Pond
22-	0800 I-30 Self	Storage	Drainge	Report.gp	wReturn P	eriod: 50 Y	ear	Wednesday	v, 10 / 18 / 2023

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Pre-development

= Rational	Peak discharge	= 90.45 cfs
= 50 yrs	Time to peak	= 16 min
= 1 min	Hyd. volume	= 86,827 cuft
= 28.910 ac	Runoff coeff.	= 0.47
= 6.656 in/hr	Tc by User	= 16.00 min
= Bryant 50.IDF	Asc/Rec limb fact	= 1/1
	 Rational 50 yrs 1 min 28.910 ac 6.656 in/hr Bryant 50.IDF 	= RationalPeak discharge= 50 yrsTime to peak= 1 minHyd. volume= 28.910 acRunoff coeff.= 6.656 in/hrTc by User= Bryant 50.IDFAsc/Rec limb fact



Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	96.16	1	16	92,318				Pre-development
2	Rational	226.82	1	8	108,874				Post-Development
3	Reservoir	5.157	1	16	103,403	2	350.44	106,461	Pond
22-	0800 I-30 Self	Storage	Drainge	Report.gp	wReturn P	eriod: 100	Year	Wednesday	v, 10 / 18 / 2023

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Pre-development

Hydrograph type	= Rational	Peak discharge	= 96.16 cfs
Storm frequency	= 100 yrs	Time to peak	= 16 min
Time interval	= 1 min	Hyd. volume	= 92,318 cuft
Drainage area	= 28.910 ac	Runoff coeff.	= 0.47
Intensity	= 7.077 in/hr	Tc by User	= 16.00 min
IDF Curve	= Bryant 50.IDF	Asc/Rec limb fact	= 1/1
Intensity IDF Curve	= 7.077 in/hr = Bryant 50.IDF	Tc by User Asc/Rec limb fact	= 16.00 min = 1/1



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Wednesday, 10 / 18 / 2023

Hyd. No. 1		Hyd. No. 3	
Pre-development		Pond	
Hydrograph type Peak discharge Time to peak Hyd. Volume	= Rational = 53.08 cfs = 16 min = 50,961 cuft	Hydrograph type Peak discharge Time to peak Hyd. Volume	= Reservoir = 2.99 cfs = 16 min = 57,823 cuft



Hyd. No. 1		Hyd. No. 3	
Pre-development		Pond	
Hydrograph type Peak discharge Time to peak Hyd. Volume	= Rational = 58.66 cfs = 16 min = 56,310 cuft	Hydrograph type Peak discharge Time to peak Hyd. Volume	= Reservoir = 3.50 cfs = 16 min = 65,800 cuft



Hyd. No. 1		Hyd. No. 3	
Pre-development		Pond	
Hydrograph type Peak discharge Time to peak Hyd. Volume	= Rational = 69.15 cfs = 16 min = 66,385 cuft	Hydrograph type Peak discharge Time to peak Hyd. Volume	= Reservoir = 4.02 cfs = 16 min = 74,479 cuft



Hyd. No. 1		Hyd. No. 3	
Pre-development		Pond	
Hydrograph type Peak discharge Time to peak Hyd. Volume	= Rational = 79.33 cfs = 16 min = 76,152 cuft	Hydrograph type Peak discharge Time to peak Hyd. Volume	= Reservoir = 4.60 cfs = 16 min = 85,472 cuft



Hyd. No. 1		Hyd. No. 3	
Pre-development		Pond	
Hydrograph type Peak discharge Time to peak Hyd. Volume	= Rational = 90.45 cfs = 16 min = 86,827 cuft	Hydrograph type Peak discharge Time to peak Hyd. Volume	= Reservoir = 5.05 cfs = 16 min = 97,246 cuft



Hyd. No. 1		Hyd. No. 3	
Pre-development		Pond	
Hydrograph type Peak discharge Time to peak Hyd. Volume	 Rational 96.16 cfs 16 min 92,318 cuft 	Hydrograph type Peak discharge Time to peak Hyd. Volume	= Reservoir = 5.16 cfs = 16 min = 103,403 cuft



Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

Pond No. 1 - <New Pond>

Pond Data

Trapezoid -Bottom L x W = 412.0 x 175.0 ft, Side slope = 2.00:1, Bottom elev. = 349.00 ft, Depth = 5.00 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	349.00	72,100	0	0
0.50	349.50	73,278	36,344	36,344
1.00	350.00	74,464	36,935	73,279
1.50	350.50	75,658	37,530	110,810
2.00	351.00	76,860	38,129	148,939
2.50	351.50	78,070	38,732	187,671
3.00	352.00	79,288	39,339	227,010
3.50	352.50	80,514	39,950	266,960
4.00	353.00	81,748	40,565	307,525
4.50	353.50	82,990	41,184	348,710
5.00	354.00	84,240	41,807	390,517

Culvert / Orifice Structures

[B] [A] [C] [A] [C] [PrfRsr] [B] [D] = 18.00 0.00 0.00 0.00 Inactive Inactive 0.00 Crest Len (ft) = 5.00 Rise (in) Span (in) = 18.00 18.00 0.00 0.00 Crest El. (ft) = 355.00 0.00 0.00 0.00 No. Barrels = 1 1 0 0 Weir Coeff. = 3.33 3.33 3.33 3.33 347.00 0.00 0.00 Invert El. (ft) = 349.00 Weir Type = Rect ------------= 70.00 30.00 0.00 0.00 Multi-Stage = No No No Length (ft) No = 0.50 0.50 0.00 n/a Slope (%) = .013 N-Value .013 .013 n/a Orifice Coeff. = 0.60 0.60 0.60 0.60 Exfil.(in/hr) = 0.000 (by Wet area) = n/a No No No TW Elev. (ft) = 0.00 Multi-Stage

Weir Structures

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



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DETENTION POND MAINTENANCE PLAN

Background

There will be one retention pond in this project. The retention pond is located at the North-East of the subject property. It is designed to temporarily detain stormwater to meet water quantity criteria before discharging off the property.

Routine Maintenance

The property owners association will maintain the drainage easements. Routine maintenance will include but not be limited to: -Mowing of the bank slopes and area around the pond on a monthly basis during the growing season and as needed during the cooler months.

-The outlet pipe from the pond and other areas will be inspected monthly for debris which could inhibit the proper flow of discharge. Any debris will be removed immediately and disposed of or placed in a location to prevent future maintenance and prevent future erosion. to not cause impact up or downstream of the structure.

-Trash will be removed from around the pond to prevent entering the pond. Generally, the site should be kept free of loose trash which could be carried off site by wind or rain.

-Inspect the pond and outlet pipe for non-routine maintenance need.

Periodic or Non-Routine Maintenance items may include but not be limited to:

-Any other maintenance or repairs which would minimize other maintenance to the pond or outfall structures.



The routine inspection of the pond area and discharge pipe will identify needed repairs and non-routine maintenance. These

-Re-growth of trees on or around the pond bank. These should be cut and removed from the pond area.

-Sediment from the site may accumulate in the pond bottom and reduce the pond to below design volume requirements. The pond should be excavated if the pond bottom elevation reached a level that allows excessive aquatic growth or reduces the pond efficiency such, that the sediments are passing the discharge structure and release off site.

-Stabilization or re-grading of side slopes may be required periodically or after excessive rain events. Any disturbance of slopes should be reseeded or may require installation of erosion control materials until seeding can reestablish adequate grasses to



